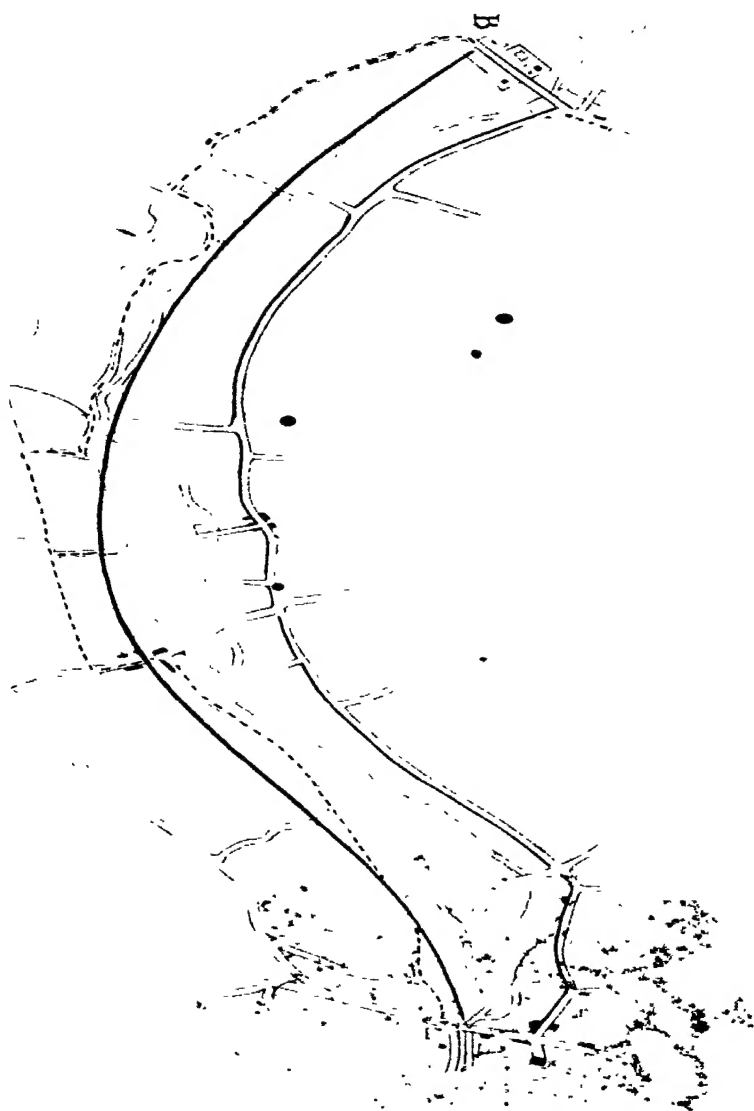




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A TREATISE  
ON  
THE PRINCIPLES AND PRACTICE  
OF  
LEVELLING.

SHOWING ITS APPLICATION TO PURPOSES OF  
RAILWAY ENGINEERING AND THE CONSTRUCTION OF ROADS, &c.

BY  
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CIVIL ENGINEER.

*SIXTH EDITION, REVISED AND CORRECTED.*

WITH THE ADDITION OF  
MR. LAW'S  
PRACTICAL EXAMPLES FOR SETTING OUT RAILWAY CURVES  
AND  
MR. TRAUTWINE'S  
FIELD PRACTICE OF LAYING OUT CIRCULAR CURVES.

*WITH PLATES AND WOODCUTS.*



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AND PARLIAMENT STREET

## ADVERTISEMENT.

THE FOLLOWING PAGES were written at the request of the Publisher,\* in consequence of the very numerous applications he had received for a book upon this subject. In doing this, it was suggested that, in addition to explaining the method of taking levels in the field, and afterwards transferring them to paper in the form of a section, I should add an example of their application to practical purposes: I have accordingly inserted an example of road-work, wherein the necessary calculations of earth-work are shown, and worked out in full, both by the Prismoidal Formula, and the shorter process by the use of the Tables of Mr. Macneill†; and, as in a manner connected with the subject, it was also suggested that I should add some particulars upon the choice of a line of direction through a country for a road or railroad, preparatory to taking levels. In conclusion, I have given an abstract of the late Mr. Telford's rules for making and repairing roads, as contained in full in the valuable work of 'Sir Henry Parnell on Roads.'

F. W. S.

\* The late Mr. Weale.

† Now Sir John Macneill.

NAWAB SALAR JUNG BAHADUR.

## ADVERTISEMENT TO THE SIXTH EDITION.

THIS WORK having been out of print for some time, and numerous enquiries having been made for it, the present Publishers have decided to reprint it. They have not done this, however, without submitting it to a searching revision, which has led to the detection and correction of several important errors. It is believed that this has added considerably to the value of the book.

. . .

*September, 1875.*

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A TREATISE  
ON  
LEVELLING.



PART I. .

ON THE PRINCIPLES OF LEVELLING.

LEVELLING is the art of tracing a line at the surface of the earth which shall cut the directions of gravity everywhere at right angles. If the earth were an extended plane, all lines representing the direction of gravity at every point on its surface would be parallel to each other; but, in consequence of its figure being that of a sphere or globe,\* they everywhere converge to a point within the sphere which is equi-distant from all parts of its surface; or, in other words, the direction of gravity invariably tends towards the centre of the earth, and may be considered as represented by a plumb-line when hanging

\* The figure of the earth is not exactly that of a sphere, but of an oblate spheroid flattened at the poles; the length of the equatorial diameter being 7924 miles, and that of the polar diameter 7898 miles. For our present purpose, it is sufficiently correct to consider it as a sphere.



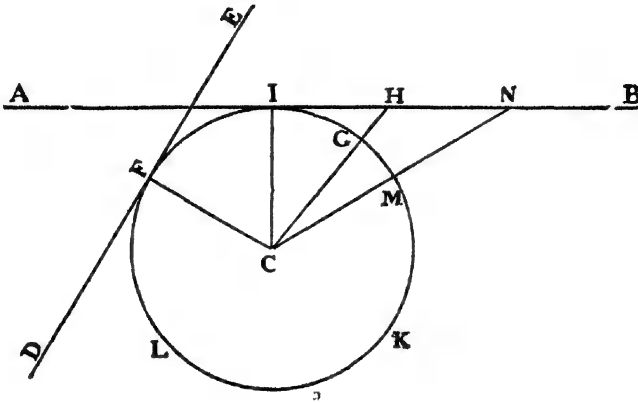
freely, and suspended beyond the sphere of attraction of the surrounding objects.



In the above diagram let the *straight* line A B represent the surface of the earth, upon the supposition of its being an extended plane, the direction of gravity at the points A, I, and B, would be represented by the lines A C, I D, and B E, all parallel to each other, and at right angles to the horizontal line A B. Now if the surface was undulatory, as shown by the curved line A B, and it was required to make a section representing it; an instrument capable of tracing out a line parallel to the horizontal line A B (as a spirit level), might be set up anywhere on the surface, as at I, and staves being placed or held along the line, as at a, b, c, d, &c., the different heights above the ground where such staves were intersected by the line so traced out, would at once show the relative level of all those points, with regard to the horizontal line, as a datum or standard of comparison.

But as the earth is a globe, its circumference must be

circular, as I K L in the annexed figure ; the straight line A B will therefore not represent the surface of the



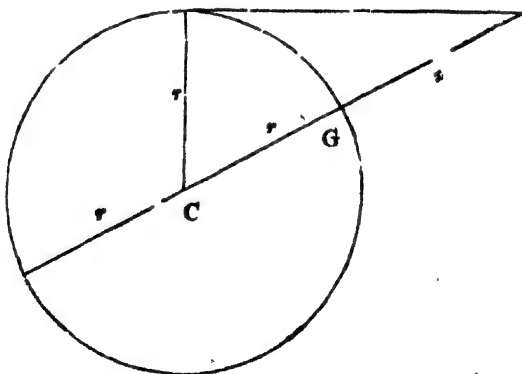
earth, but the sensible horizon of an observer stationed at the point I, to which point it is a tangent, being at right angles to the radius of the circle (or semi-diameter of the earth), I C. A line which is parallel to the sensible horizon of the observer, is the line traced out by our spirit-levels ; it is parallel to a tangent to the earth's surface at that point only where the instrument is set up,—thus A B is a tangent at I, and D E a tangent at F ; such being the fact, the difference of level between any two points cannot be determined by simple reference to a horizontal line, since every point on the surface of the globe (however near to each other) has a distinct horizon of its own.

If the earth were everywhere surrounded by a fluid at rest, or that its surface was smooth, regular, and uniform, every point thereon would be equally distant from the centre ; but in consequence of the undulating form of the surface, places and objects are differently situated, some further from, and others nearer to, the centre of the earth, and consequently at different levels.

The operation of levelling may therefore be defined as the art of finding how much higher or lower any one point is than another, or, more properly, the difference of their distances from the centre of the earth.

Referring to our last figure, we have seen that the line  $A B$  is a true horizontal or level line at the point  $I$ , but being produced in the direction  $A$  or  $B$ , rises above the earth's surface; and although it may appear to be level as seen from  $I$ , yet it is above the true level (which is represented by the circumference of the circle) at every other point, and continues to diverge from it the further it is produced; at  $G$ , the apparent line of level, as the horizontal line  $A B$  is called, is above the true level, by the distance  $G H$ , and at  $M$  by the distance  $M N$ , *the difference being equal to the excess of the secant of the arc of distance above the radius of the earth.*

The difference,  $G H$ , or  $M N$  (see last figure), between the true and apparent level may be thus found: put  $t$  in the adjoining diagram for the tangent  $I H$ ,  $r$  for



the radius  $C I$  of the earth, and  $x$  for  $G H$ , the excess of the secant of the arc of distance above the radius;  $I H$  being considered as equal to  $I G$ ; then

$$\begin{aligned}
 (r+x)^2 &= r^2 + t^2 \\
 r^2 + 2rx + x^2 &= r^2 + t^2 \\
 \text{and } 2rx + x^2 &= t^2 \\
 \text{or } (2r+x)x &= t^2
 \end{aligned}$$

But because the diameter of the earth  $2r$  is so great with respect to the quantity  $(x)$  sought, at all distances to which a common levelling operation usually extends, that  $2r+x$  without sensible error may be replaced by  $2r$ , we then have

$$\begin{aligned}
 2rx &= t^2 \\
 \text{and } x &= \frac{t^2}{2r}
 \end{aligned}$$

Or in words: *The difference  $(x)$  between the true and apparent level is equal to the square of the distance  $(t^2)$  divided by the diameter of the earth  $(2r)$ , and consequently is always proportional to the square of the distance.*

The mean diameter of the earth is 7916 miles, and the excess of the apparent above the true level for one mile  $\frac{t^2}{2r} = \frac{1}{7916}$  of a mile, or 8.004 inches. At two miles, it is four times that quantity, or 32.016 inches; at three miles, it is nine times that quantity, or 72.036 inches; and so on increasing in proportion to the square of the distance. If we reject the decimal .004, and assume the difference between the true and apparent level for one mile to be exactly eight inches, or two-thirds of a foot, there arises the following convenient form for computing the correction of level due to the curvature of the earth, for distances given in miles, which may easily be remembered:

$$\text{correction} = \frac{2 D^2}{3}$$

D being the distance in miles. Or in words: *Two-thirds of the square of the distance in miles will be the amount of the correction in feet.*

*Example.*

From a point on the Folkestone road, the top of the keep of Dover Castle was observed to coincide with the horizontal wire of a levelling telescope when adjusted for observation, and therefore was apparently on the same level; the distance (D) from the instrument to the Castle was four miles and a half: consequently,

$$\begin{aligned} D^2 &= 20\cdot25 \\ 2 D^2 &= 40\cdot50 \\ \frac{2 D^2}{3} &= 13\cdot5 \text{ feet, the correction required.} \end{aligned}$$

From this it appears, that the keep of Dover Castle was 13·5 feet higher than the centre of the telescope on the Folkestone road; but on account of the curvature of the earth, it was apparently depressed to the same level.

But the effect of the earth's curvature is modified by another cause, arising from optical deception; namely, Refraction. An object is never seen by us in its true position, but in the direction of the ray of light which conveys the impression or image of the object to our senses. Now the particles of light, in traversing the atmosphere, are, by the force of superior attraction, refracted or bent continually towards the perpendicular, as they penetrate the lower or denser strata; and consequently they describe a curved track, of which the last portion, or its tangent, indicates the apparent elevated situation of a remote point. This trajectory, suffering almost a regular inflexure, may be considered as very

nearly an arc of a circle, which has for its radius seven times the radius of our globe; in consequence of which, the distance at which an object can be seen by the aid of refraction, is to the distance at which it could be seen without that aid, nearly as 14 to 13, the refraction augmenting the distance at which an object can be seen by about a thirteenth of itself. Hence, to correct the error occasioned by refraction, it will only be requisite to diminish the effects of the earth's curvature, or height of the apparent above the true level, by one-seventh of itself. Thus for our example of Dover Castle,  $\frac{1}{7}$  of 13·5, or  $\frac{13\cdot5}{7} = 1\cdot93$  feet nearly, to be subtracted from 13·5, which leaves 11·57 feet for the height of Dover Castle above the level of a certain point on the Folkestone road.

The following Tables show the reduction of the apparent to the true level, both for the curvature of the earth only, and also for the combined effects of curvature and refraction. The first gives the corrections corresponding to distances expressed in miles, and the for distances in chains.

# A TREATISE

*of the Difference of the Apparent and True Level for  
Distances in Miles.*

Distance in Miles.	CORRECTION.			
	Curvature.		Curvature and Refraction.	
	feet.	inches.	feet.	inches.
$\frac{1}{4}$	0	0.5	0	0.4
$\frac{1}{2}$	0	2.0	0	1.7
$\frac{3}{4}$	0	4.5	0	3.9
1	0	8.0	0	6.9
2	2	8.0	2	3.4
3	6	0.0	5	1.7
4	10	8.1	9	1.8
5	16	8.1	14	3.5
6	24	0.1	20	7.0
7	32	8.2	28	0.2
8	42	8.3	36	7.1
9	54	0.3	46	3.7
10	66	8.4	57	2.1
11	80	8.5	69	2.1
12	96	0.6	82	3.9
13	112	8.6	96	7.4
14	130	8.8	112	0.7
15	150	0.9	128	7.6
16	170	9.0	147	2.3
17	192	9.2	165	2.7
18	216	1.3	185	2.8
19	240	9.4	206	4.7
20	266	9.6	228	8.2

*Table of the Difference of the Apparent and True Level for Distances in Chains.*

Distance in Chains.	CORRECTION.	
	Curvature in decimals of feet.	Curvature and Refraction in decimals of feet.
1	·000104	·000089
2	·000417	·000358
3	·000938	·000804
4	·001668	·001430
5	·002605	·002233
6	·003752	·003216
7	·005107	·004378
8	·006670	·005717
9	·008442	·007236
10	·010422	·008933
11	·012610	·010809
12	·015007	·012863
13	·017613	·015097
14	·020427	·017509
15	·023450	·020100
16	·026680	·022869
17	·030120	·025817
18	·033767	·028943
19	·037623	·032248
20	·041687	·035732
21	·045960	·039394
22	·050442	·043236
23	·055132	·047259
24	·060031	·051455
25	·065137	·055832
26	·070452	·060388
27	·075975	·065121
28	·081708	·070036
29	·087648	·075127
30	·093798	·080399



The correction for distances greater than those given in the latter Table may be computed by the following rule, the same by which the Table itself was computed :

Rule.—*To the arithmetical complement of the logarithm of the diameter of the earth, or 2.3788603, add double the logarithm of the distance in feet, the sum will be the logarithm of the correction for curvature in feet and decimals ; from which, if one-seventh of itself be subtracted, the result will be the combined correction for curvature and refraction.*

The practice of levelling is one of the most delicate operations that fall within the province of a surveyor, requiring the utmost possible circumspection to avoid the numerous sources of error to which he is liable. More especially, as it is seldom possible for him, after levelling over a long tract of country, to conjecture in what portion of the work his error lies, if he should then find that he had been so unfortunate as to commit any, and, not unfrequently in such cases, sufficient time cannot be spared to go over the ground again ; as, for instance, when a section is required within a very limited time to produce before a parliamentary committee, either to support or oppose any measure submitted to their consideration. We have witnessed an instance where such a committee, during their inquiry into the merits of a certain proposed line of railroad, had brought before them a *rival contemplated line with pretensions to great superiority* ; but it had been so hastily surveyed, that the learned counsel who had the supporting of the measure, acknowledged, in his opening address, that a trifling error at some unknown part of the line had been detected, which did not exceed fifty

feet. We hardly need add, that the rival line was rejected.

The importance of extreme accuracy may also be felt, when it is known that from the section, the engineer has to make his calculations of the quantity of earthwork, in cuttings and embankments, necessary to carry into execution the intended measure, whether of a canal, a railway, or turnpike road, and of course the accuracy of the estimated expense is involved in it; and further, the fitness of the ground itself for such works is determined from the section; that is, whether the inclinations, which the undulations of the ground admit of being introduced, are suitable for the purpose either of a railway or turnpike road. And if the object be the formation of a canal, the section must show what extent of lockage will be required; not only affording a key to the expense, but also the possibility of its execution. We do not throw out these suggestions to alarm the mind of the young beginner, by bringing before him a fearful responsibility, but that he may understand the ultimate object of his labours, and to induce him, by carefulness and attention, to merit that confidence which is sure to be reposed in those who are known to possess such habits.

#### LEVELLING INSTRUMENTS.

It is essential to the good execution of work, that the surveyor should possess instruments most proper for the purpose, and of the best construction. Upon the subject of instruments, we shall generally refer the reader to a cheap work, entitled, "A Treatise on the

principal Mathematical Instruments employed in Surveying, Levelling, and Astronomy, explaining their construction, adjustments, and use;" where the various kinds of spirit-levels, and levelling staves, together with the method of performing their several adjustments, &c., are minutely detailed, and represented by engravings;\* and as the work alluded to contains also a similar account of the most important instruments used in surveying and astronomy, and has had an extensive sale, we presume it to be in the hands of most beginners in the profession; we shall, however, give some particulars in this place, and annex a description of the cause of, and a remedy for, the *parallax* between the wires of a levelling telescope, and the levelling staves, which is the cause of much annoyance to observers.

#### . SPIRIT-LEVELS.

*The Y level*, so called from the supports in which the telescope rests, resembling in shape the letter Y, is the oldest construction of the spirit-level now in use: its adjustments are convenient to be performed, but, on the other hand, this kind of instrument seldom retains its adjustments perfect for any length of time; besides, there are conditions in its construction which are assumed to be perfect, but which practical men know to present difficulties in the manufacture. The use of this instrument is now very much superseded by those of modern construction.

*Troughton's Improved Level*.—This instrument has

\* Also a Work published by Mr. Weale, on Drawing Instruments, with Instructions for Field Work, in 12mo., price 3s. 6d.

been a very general favourite among engineers for a length of time: its construction renders its adjustments much more permanent than those of the Y level, and it is altogether a more stable instrument. The telescope, which, in the former instrument, is capable of reversion on its supports, for the adjustment of the line of collimation, is, in Troughton's construction, firmly fixed in its place, as is also the glass tube of the spirit bubble. The verification and correction of the adjustments are performed very differently, and may at first appear more complex and difficult than those of the other; yet when a person has once mastered and become familiar with his instrument, these apparent difficulties vanish.

*The Dumpy Level.*—This modification of the spirit-level has but recently been introduced by William Gravatt, Esq., and bids fair to become the favourite instrument among civil engineers. In its general figure it does not differ very essentially from the level last spoken of, but it possesses many decided advantages. The aperture of the object glass is much larger for the same length of telescope; consequently more rays of light are admitted to the eye, producing the advantages of greater distinctness. We lately tried a *fourteen-inch* level, constructed upon Mr. Gravatt's principle, and found that we could distinctly read the levelling-staff at twenty chains (a quarter of a mile) distant, which was the utmost we could do with a *twenty-inch* level upon the old construction: we have, therefore, the advantage of a more portable instrument, fourteen inches in length, capable of performing the same work as a more cumbersome one of twenty inches. Besides this advantage, the instrument in question is more complete in its details.

It possesses a cross level, placed at right angles to the principal level, which affords very great facility in setting up the instrument, and adjusting for observation, as will be hereafter described: it likewise has a reflecting mirror, mounted with a hinge joint, and capable of being placed on the principal level tube, and adjusted, to show the observer if the instrument shifts from its horizontality whilst he is noting the observation: it also possesses other important though minor additions, all of which, in fact, could be applied by the maker to the other kind of instruments, if ordered, and for the particulars of which we refer to the work before alluded to.

From the large aperture and short focal length of the telescope, the instrument has altogether a dumpy appearance, and hence it is generally known by the cognomen of "Gravatt's Dumpy Level:" usually of nine or fourteen inches. We have seen some beautiful specimens of this kind of levelling instrument constructed for I. K. Brunel, Esq.

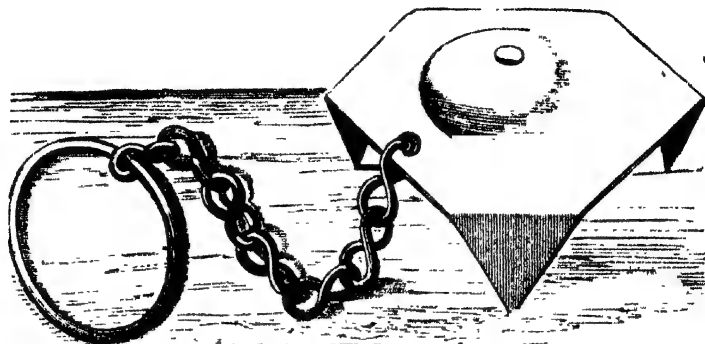
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#### LEVELLING STAVES.

In the Treatise on Mathematical Instruments, will be found a description of the different kinds of levelling staves in use. The former construction, even as improved by Troughton, was decidedly defective in practice, inasmuch as the staff had to be read off by the assistant, who had then to communicate the result to the observer; or, if he was not sufficiently intelligent to be intrusted with so responsible a duty, he was obliged, after the observation was made, to carry the staff to the observer, or wait for him to come and read off the height of the

vane, and register it in his field-book. This occasioned great loss of time and uncertainty in the results, for the vane on the staff might possibly be shifted in the meantime. We remember an instance of an ignorant attendant holding the staff upside down, which at once introduced an error of several feet in the result. To obviate this, a new staff has been contrived, originally, we believe, by Mr. Gravatt, and subsequently by Mr. Hennett, Mr. Bramah, Mr. Sopwith, &c., each varying the mechanical arrangements, but all agreeing in retaining the main advantage, viz. a sufficiently distinct graduated face for the observer to read off the quantities himself through the telescope of his instrument: the sliding vane is therefore dispensed with, and the only dependance to be placed on the staff-holder is, that he may hold it perpendicularly. To assist him in this, a small plummet is suspended in a groove cut out in the side of the staff, by which its verticality can be determined in one direction, and the observer himself can detect if it be held aslant in the other direction, as may be understood from the diagram at page 21, which represents the staff *e* as it appears in the field of the telescope, which shows objects inverted. If the staff be held perpendicularly, it will appear between and equally distant from each of the two vertical wires *c d* fixed in the telescope; consequently, if it be held aslant, it will cross the wires obliquely, and any want of verticality in the staff will be immediately detected, and the observer must signal to the staff-man accordingly. The advantages from the use of the modern staves, over those of the old construction, are so great, especially in saving of time, that we have no doubt of their general adoption.

## THE IRON TRIPOD.



Another instrument of simple construction is represented in the above figure: its use is to rest the staff upon when held at any station. By this means the staff is sure to be kept on the same spot, and at the same height from the ground, while the observer is reading the staves both at the back and forward station on each side of the spirit-level: it is at present not generally used, but we consider it of more importance than is usually attached to it. It consists of a triangular piece of sheet iron, of about one-tenth of an inch in thickness, having the corners turned down to form the feet of the tripod, which are to be pressed into the ground by the foot of the staff-holder; a rounded piece of iron is riveted on the upper surface, to present a clean spot to rest the staff upon when held at the station; the chain with the attached ring is for the convenience of the staff-holder in lifting it from the ground, and carrying it from station to station.

## THE MEASURING CHAIN.

In levelling operations it is in most cases necessary to note the relative distances of the staves from each

other, from the spirit-level, or from some given point or place, otherwise no section of the ground levelled over can be made. For this purpose a measuring tape may be employed where the distances are short, but in most cases the means employed is a chain; the one commonly used is 4 poles in length, called Gunter's chain, which is divided into 100 links of 7·92 inches each. In many cases, however, this will not be found so convenient as the use of a chain with links of 1 foot in length; but there is a practical inconvenience attending these long links where the ground is rough and uneven, as the links are likely to get bent in being drawn through the hedges and rough places: whenever this occurs the chain is reduced in length, and, unless discovered and rectified, a considerable error in distance will very soon result. When we have had occasion to use such a chain over rough ground we have had the links made 6 inches long, and although it occasioned more trouble in noting and registering the distances, yet the liability of the links to become bent was greatly diminished. No measurements are required in taking what are called running or check levels, the object of which is merely to test the accuracy of a section previously made, by finding the difference of level between certain points on the section, to see if the results are identical with the former determination; which is the same thing as ascertaining the whole difference of level between distant places. Neither are any measurements required to produce a section if you possess a correct map or plan of the district or line, for if the level points are noted on the said plan, their relative distances can be taken therefrom by its scale; this, however, can only be considered



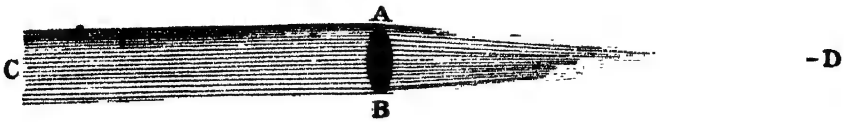
as an approximate operation as far as the horizontal measures are concerned. In this way, however, many extensive trial sections for long lines of railway have been made by means of the Ordnance maps, and will, if properly done, determine the general features of the country sufficiently for the engineer to choose the best route for a minute and detailed survey, which would cost too much time and money to undertake in the first instance where there exists any doubt between two or more routes as to which it would be most judicious to adopt.

#### ON INSTRUMENTAL PARALLAX.

The foregoing is an account of the instruments necessary for the purposes of levelling; but before closing this part of our subject, we think it may be useful to add some particulars respecting instrumental parallax, which we have occasionally found to be the source of much annoyance to the surveyor. This has invariably arisen from ignorance of the principles of the telescope, and hence, not knowing how the parallax arises, the means of removing it have not been understood; we shall endeavour to explain, in a popular manner, both the cause and the remedy.

The rays of light which proceed from surrounding objects, and which, by entering our eyes, convey to us the sense of vision, move in perfectly straight lines, unless turned from their rectilineal course by the intervention of a refracting or reflecting medium, and whatever portion of such rays as can enter our eyes may (without sensible error) be considered as moving not only in *straight*, but *parallel* lines; the more remote the

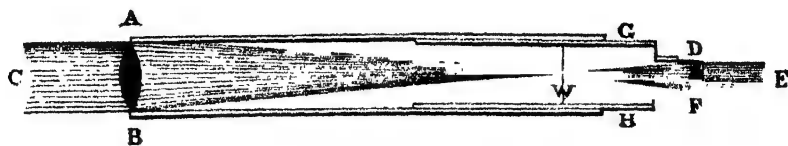
object is, the more nearly this will be the case. In the adjoining diagram, let A B represent the section of a



lens (or object glass of a telescope); let the parallel lines on the left represent the rays of light coming from some distant object in that direction; the instant they impinge upon the glass, and in passing through it, they suffer refraction—that is, they are bent out of their former rectilinear path—and on leaving the lens at the opposite side, they converge to a certain point D, which is the focus of the object glass (in this point *all* the rays passing through a *perfectly formed* glass meet, and it is situated on the line C D, the direction of the ray which passes through the centre of the glass, the only one that continues its former course, and is called the axis of the lens); the concentration of the rays form an image of the distant object in the focal point D, “and if a piece of ground glass, transparent paper, or a plate of glass having one surface covered with a dried film of skimmed milk, be held up at D, a person looking at it from a few inches behind would see a perfect image of the distant object formed on the ground glass; and by steadily keeping the eye in the same position, the ground glass may be removed, and the image will appear in the same spot suspended in the air.”

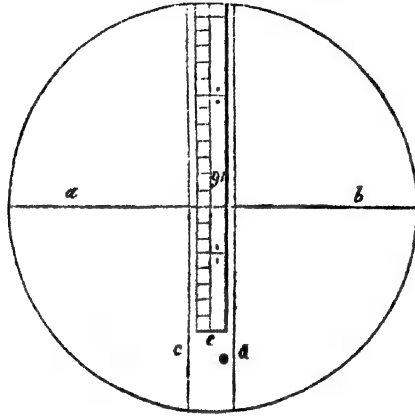
Now let us imagine the lens applied to the construction of a telescope, and the adjoining diagram to represent a section of it; the image of a levelling staff held at a distance, in the direction of C, would be formed at the

point W, the focus of the object glass; let D F represent the eye-glass, which is fixed in a sliding tube, and together called the *eye-piece*. The eye-piece may be



considered as a microscope, with which the observer magnifies the image of the object formed at W; to do this, it will readily appear to the reader that its distance from the image at W must be such as to cause its focal point to coincide therewith, making that point the common focus of the two glasses: for the purpose of effecting this, the eye-piece is made to slide either in or outwards, and the observer can tell when it is at the proper distance, for he will then obtain a perfectly distinct view of the object. The axis of the two glasses forms a continued straight line C E, which in a telescope is technically termed the optical axis of the instrument, or line of collimation; this imaginary line is, in levelling telescopes, the zero, from whence the readings on the staff are taken. It is therefore necessary to represent it by something tangible, that shall at the same time not interfere with the rays of light passing through the telescope to the eye; this is done by fixing across the interior of the telescope very fine wires, or threads from a spider's web, so that their intersection may not only coincide with the axis C E, but cross it precisely at W, the common focus of the two glasses, where the image of the staff (or distant object) is formed, and therefore the wires and the staff will appear to an observer as one object, or, at least, equally distant from him. The

following diagram shows the appearance of the wires and the staff as seen through an inverting telescope; where  $ab$  represents the horizontal wire,  $c$  and  $d$  two



wires placed at right angles to it, and separated so as to admit, at usual distances, the staff  $e$  to appear between them, by which the observer can always tell if the staff-man holds it erect in a lateral direction, as before explained. The staff is represented as seen at the moment of completing an observation; the horizontal cross wire coinciding with the division  $\cdot 20$  above 16 feet, the staff being read downwards in consequence of its apparent inversion; the reading therefore, of such an observation, to be entered in the field-book, would be 16·20 feet.

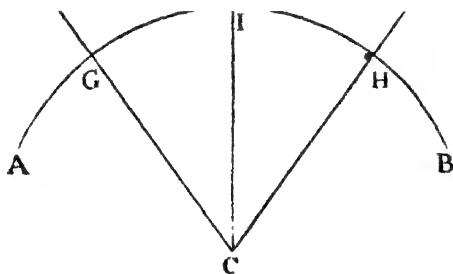
The adjustment of the line of collimation consists in making the centre of the horizontal wire (or intersection of the wires in instruments intended for measuring angles) coincide with the optical axis of the telescope; this, when once accomplished, will, with care, keep correct for a long time, but the placing it in the common focus of the two glasses requires attention at every observation. For detailed instructions upon the former,

we refer to the treatise on Mathematical Instruments, &c.; but as the latter forms part of every observation, and is the source of the perplexing parallax, we shall speak of it in this place.

The cross wires are fixed to a plate, called a diaphragm, attached by screws to the slide G H, which also carries the slide D F of the eye-piece. The point W, or focus of the object glass, does not remain constant for terrestrial objects, but varies with every change in the distance of the staff; if it be brought closer to the instrument, the image, or focal point, will recede further from the glass, and *vice versâ*; therefore, the wires and the focus of the eye-piece must be brought to coincide with that of the object glass by their respective slides; and first, the eye-piece should be moved in its slide till its focus coincides with the wires in the tube G H; when this is accomplished, the observer will see the wires perfectly sharp and well-defined; next, motion must be given to the slide G H, by turning a milled head attached to the telescope, which gives motion to the slide by rack work; this will carry both the wires and the focus of the already adjusted eye-piece to coincide with the focus of the object glass, on whatever part of the optical axis of the instrument it may be situated. When this is done, the adjustment of the telescope for observation will be complete, and its proof consists in the observer having at the same time a clear and well-defined image both of the staff and the cross wires, which will be the case if they seem to be *attached* to each other,—or, in other words, appear equally distant from him; and the moving about of the observer's eye does not detect any apparent displacement of the staff, with respect to the

wires. Such a displacement, or relative motion, is what is meant by *parallax*; and when it exists, it must be got rid of by a repetition of the adjustment of the glasses as above described, till the motion of the eye will no longer detect the least apparent movement, or passing and repassing of the wires and the staff: till this is done, no correct observation can be made.

From what has been advanced on the subject of the corrections for curvature and refraction, it may be necessary, before entering upon any practical examples, to remark, that such corrections are very seldom applied in practice, the observer, by the arrangements of his operations doing away in a great degree their injurious effects, which we will endeavour to explain.



Suppose it were required to find the difference of level between any two points G and H in the preceding figure; let A B represent a portion of the earth's surface, let C represent the centre, and CG, CI, and CH the radii of the earth. Now a spirit-level being set up and adjusted at I, an observer looking through the telescope would see objects in the direction of the horizontal line DE only, and a staff held upright at H would be read off in the point E on the horizontal line; but this point is

higher than the true level by the distance  $HE$ , which is the correction for curvature due to the distance  $IH$  (see page 5); and if that quantity be subtracted from the reading of the staff, the remainder will show the difference of level between the points  $I$  and  $H$ . If the same process be gone through by holding a staff at  $G$ , then the difference of level between  $G$  and  $I$  will also be ascertained, which being compared with the former difference, will show how much higher one of the points  $G$  or  $H$  is above the other; but it must be evident, that if  $G$  and  $H$  be equally distant from  $I$ , the horizontal line  $DE$ , being a tangent to the surface at the middle point  $I$ , must cut the staff at  $D$  on the same level with the point  $E$ ;—that is,  $CD$  is equal to  $CE$ , therefore  $D$  and  $E$  are level points, being equi-distant from the centre of the earth; and if the reading of one staff above the ground is greater than the reading of the other, the difference will at once show the variation of level between the points where the staves were held, viz.  $G$  and  $H$ ; the effect of curvature is thus removed by *simply placing the instrument midway between the station staves*. The effects of the atmospheric refraction will likewise be done away with in the same process, because it will affect both observations alike, unless under peculiar circumstances of the weather, &c., over which the observer has no control.

The above method of finding differences of level, by placing the instrument as near as possible midway between the two staves, and noting their readings, is the one adopted in practice; but as it can scarcely ever happen, on account of the extent of the work, that one placing of the instrument will complete it, a succession

of similar operations must be performed, as shown in the annexed engraving.



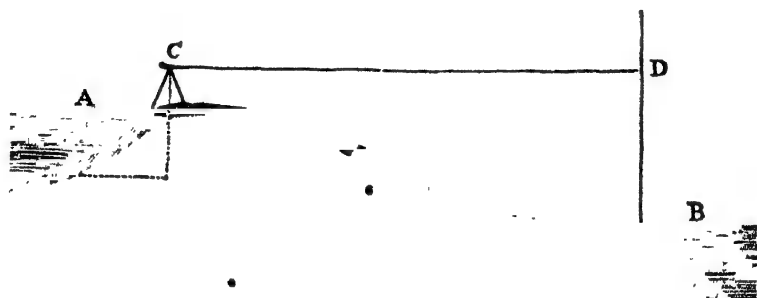
Suppose it were required to find the difference of level between the points A and G; a staff is erected at A, the instrument is set up at B, another staff at C, at the same distance from B that B is from A. The readings of the two staves are then noted; the horizontal lines connecting the staves with the instrument represent the visual ray or line of sight. The instrument is then conveyed to D, and the staff which stood at A is now removed to E, the staff C retaining its former position, and from being the forward staff at the last observation, it is now the back staff: the readings of the two staves are again noted, and the instrument removed to F, and the staff C to the point G; the staff at E retaining the same position, now becomes in its turn the back staff, and so on to the end of the work, which may thus be extended many miles: the difference of any two of the readings will show the difference of level between the places of the back and forward staff; and the difference between the sum of the back sights and the sum of the forward sights will give the difference of level between the extreme points: thus,

	Back sights. ft. dec.	Fore sights. ft. dec.
A and C . . . . .	10·46	11·20
C „ E . . . . .	11·33	8·00
E „ G . . . . .	7·42	7·91
Sums . . . . .	29·21	27·11
	27·11	
Difference of level . . . . .	2·10	



showing that the point G is 2 feet and  $\frac{1}{10}$  higher than the point A.

The foregoing process is called compound levelling. The following is an example of simple levelling, being performed at one operation, and therefore subject to the correction for curvature and refraction to obtain a correct result.



Suppose it were required to drain a pond and marsh A, by making a cut to a stream at B, a distance of thirty chains: let a level be set up at C, and directed to a staff held upright at the edge of the water at B. The horizontal line CD represents the line of sight which would cut the staff at D, the reading being 17·44 feet; the height of the instrument above the ground was 4 feet, and the depth of the pond 10 feet; therefore the difference of level between the bottom of the pond and the surface of the stream was as follows:

Reading of the staff . . . . .	ft. dec. 17·44
Height of instrument . . . . .	4·00
Depth of pond . . . . .	10·00
Curvature and refraction for 30 chains (see Tables, pages 8 and 9) . . . .	0·09
	<hr/> 14·09
Difference of level . . . . .	<hr/> 3·35

## PART II.

## THE PRACTICE OF LEVELLING.

## ON RUNNING OR CHECK LEVELS.

To present, in the clearest possible manner, the practical application of the principles of levelling, we propose describing some operations in detail. We shall, therefore, commence with a case of a simple kind, which will prepare the way for more complicated examples. When a section of a line of country has been completed (for any purposes whatever), it is in most cases necessary to check its accuracy by repetition; but in doing this, it is seldom requisite to level over precisely the same line of ground, unless there is cause to suspect its general correctness, but to follow the most convenient and nearest route, and at intervals to level to some known points on the exact line of section, which will give *their* differences of level: the points thus selected are generally what are called bench marks, and are nothing more than marks or notches cut upon gate-posts, stumps of trees, mile or boundary stones, or any similarly immoveable objects, contiguous to the line of section, and at frequent intervals. These bench marks are made by the person who takes the section in the first instance, and are sometimes previously determined upon. When the section is complete, their relative heights with regard to the base line or datum of the

section become known ; consequently, they may be considered as so many zero or fixed points on the line, easily recognisable, from whence any portion of the work may be levelled over again; or branch lines of level may be conducted in any direction, and the levels of such branches be comparable with those of the main line.

When, in checking the principal levels, by proceeding in the most convenient direction from bench mark to bench mark, it is found that the differences of level prove identical with those on the section, or within the limits of probable error, it may be presumed that all the intermediate heights are likewise correct: it is, however, just possible that equal errors of an opposite kind may have been committed, when, the *sum* of each being of the same magnitude, a balance of errors would cause the extreme points to be right, whilst the intermediate levels would be incorrect ; but the probability is so much against such an occurrence, that we believe, unless there be some particular reasons for so doing, the whole exact line of a section is seldom levelled a second time for the purpose of checking the former results only.

From what has been remarked, it will appear evident that in taking running or check levels, there is no necessity for the use of the chain, or the compass attached to the instrument, the distances and bearings having all been determined at the time the principal levels were taken.

The example we are about to give of this kind of operation is represented in the engraving, Plate I., which shows both the ground plan and the section. The

strong black line on the plan is that of the section to be checked, and proceeds from a bridge in the town of A, in a circuitous direction along a valley, and nearly parallel to the course of a river, to a bench mark in the town of B: this originally formed a portion of a more extensive survey. We have selected this portion of the line as explanatory of our present subject; the route taken in proving the work is represented by the dotted line, and was confined to the public roads, that being considered the most convenient, because it would altogether exclude the necessity of passing through private property, as the surveyor would most likely have been ordered off, a great feeling of opposition existing among the owners and occupiers of the said lands; and further, the public road crossed the line several times, by which a number of intermediate points could be checked. Before giving the particulars of this example, we shall explain in detail the method of conducting the necessary observations.

In the first instance the staff-holder must place his staff on the bench mark from whence the levels are to commence. (In the case of our example the staff was first placed on a peculiarly shaped stone on the crown of the bridge at A, which could easily be recognised from description at any future time, if ever it should be necessary to refer to this spot again: it therefore answered as a bench mark.) The surveyor must next set up his spirit-level in the most suitable spot which presents itself, from whence he can have an uninterrupted view, not only of the staff at the back station, but also for a considerable distance in the direction he wishes to carry his levels. The station selected should not in any case

exceed four or five chains, and if it be only half that quantity, there will be less likelihood of error; for when long sights (as they are usually termed) are taken, unless both the back and forward stations are equally distant from the instrument, errors will gradually creep in upon the results, which, in a long series of levels, are liable, by their accumulation, to become of serious consequence. The proper station being determined upon,\* and the tripod legs of the instrument spread out and thrust into the ground sufficiently to ensure its stability, the observer must adjust his level for observation in the following order:—First, he must draw out the eye-piece of the telescope till he sees the cross wires perfectly well defined; then, directing it to the staff, he must turn the milled-headed screw, on the side of the telescope, till he can likewise distinguish with the utmost possible clearness the smallest graduations on the staff: that these two adjustments be very carefully and completely performed, is of more consequence than is generally supposed, for upon them depends the existence or non-existence of parallax. If any parallax is detected, it must be removed, or the observations will be incorrect: its existence may be detected by the observer moving his *eye* about at the same time that he is looking through the telescope at the staff; and if he sees that the cross wires do not appear to have the least motion with regard to the divisions with which they are coincident, then no parallax will exist; but if any motion appears to take place between the wires and the staff, it is a proof that

\* It must be borne in mind, when we thus minutely detail what may appear to the practical man as naturally obvious, that we are writing for the information of those who have never had any practice whatever.

one or both of the foregoing adjustments have been imperfectly made.

To remedy this inconvenience the eye-piece should first be moved to try and improve the distinct appearance of the cross wires. The observer will be greatly assisted in this operation if he holds a sheet of white paper before the object glass, which, at the same time that it prevents other objects from attracting his attention, presents a clean white disk, or ground, for the wires to be seen upon; and when he is satisfied that they are as sharp and well defined as possible, he must repeat the movement of the milled head by the side of the telescope till he is equally satisfied of the distinct appearance of the graduations on the staff; then let him again move his eye about before the eye-glass to see if any parallax still exists, and if so, he ought to repeat the above simple operation until it is removed. We have known the parallax of a telescope to be a source of great annoyance to persons in the profession, which has led us to be thus minute upon what to some would appear very simple. We have for the like reason given an explanation of its nature, &c. at page 18.

The turning the milled head to obtain distinct vision of the staff, in the old construction of instruments, communicated motion to the object glass; but in those of recent contrivance, it moves the whole of the eye end of the telescope, and with it the cross wires. In either case, the distance between the object glass and the wires is increased to a proper extent; the modern contrivance appears to be the most approved. The adjustment of the eye-piece for distinct vision when once made, is not likely to require alteration the whole day, unless it be

accidentally deranged; but that of obtaining distinct vision of the distant staff (together with the one we shall next describe) must be performed at every station, as it varies with the distance of the staff, as explained at page 22.

Having made the above adjustments perfect, bring the spirit-bubble into the centre of its glass tube, which position it must retain unmoved in every direction of the instrument; or in other words, the bubble must indicate a true level during the time the telescope is turned completely round horizontally on its staff head: this is accomplished by bringing the telescope successively over each pair of the parallel plate screws, and giving them motion, screwing up one while unscrewing the other to a corresponding extent; but if the telescope is supplied with a cross level, as in that contrived by Mr. Gravatt, the two bubbles, being at right angles to each other, will at once show which pair of screws require turning, in order to produce an indication of level in both bubbles. In the Treatise on Mathematical Instruments there is given an ample explanation of the adjustment of levels in all their details: upon such subjects we shall once for all refer to that work.

Having adjusted the level for observation, it must be directed to the back staff, of which a clear view must be had; then note with all possible exactness the foot, and decimal fraction of a foot, with which the central part of the horizontal wire appears to be coincident, which enter in the proper column of the field or observation book. This column should be headed "Back Sight," or "Back Station," as in the example given at page 38. As soon as it is registered, look to see that the spirit-

bubble has not removed from its central position, and then repeat the observation, to ensure that no mistake had been made in noting it: this should be invariably done, to guard against errors.

The back observation being made, turn the telescope round in the forward direction, and obtain a distinct view of the staff, by turning the milled head at the side of the telescope; then look at the spirit-bubble, and if it has at all changed its position, by receding towards either end of its tube, bring it back to the centre by the parallel plate screws, as before described (this can be done so readily, and without moving the telescope, when a cross level is attached, and having likewise other advantages, that we recommend its universal application to spirit-levels); then, by looking through the telescope, observe what division on the staff is intersected by the cross wire, and enter the reading in the proper column of the field-book, which should be headed "Fore Sight," or "Fore Station." Having entered it, look to see that the bubble is still correct, and then verify the observation by noting it again, which will complete the first levels.\*

It may be worth remarking that, in setting the level up, the pointed legs should be pushed into the ground sufficiently to ensure the stability of the instrument, and likewise that the observer should move himself about the instrument, whilst taking the levels, as little as possible, taking care not to strike the legs with his feet. Caution in these matters is required, for sometimes the least

\* When taking levels for the formation of a section, it is sometimes necessary to note the bearing of the compass needle, and to measure distances, as will be explained hereafter.



## A TREATISE

movement of the person will derange the levels of the instrument, particularly on loose or elastic ground ;—to do away the inconvenience arising from this source, a reflector has been contrived to fix on the top of the telescope tube, by which the observer can see both the staff and the reflected image of the spirit-bubble at the same time, and then he can make his observation at the instant he sees the bubble in its proper position. The foregoing description of the method of taking levels is general, and applies equally to every kind of levelling operation, with whatever additional matters may require attending to, when taking levels for the formation of a section, &c., which we shall hereafter describe.

The first levels being completed, the surveyor must take up his instrument, and, passing the man who holds the forward staff, proceed to some convenient spot to set up the instrument a second time, which, as before remarked, should not be more than four or five chains distant; the other man, also, who held the staff at the back station, must likewise take up a new station still further onwards in the required direction, and as nearly as possible at the same distance from the instrument as the instrument is from the staff, which has now become the back station; it being in every case necessary, to ensure correct work, that the instrument should occupy very nearly the middle point between the staves, for reasons which will be understood by those who have perused the former part of this book. Having set the instrument up, adjust it for observation as before—viz. see that the cross wires are distinct; turn the milled head by the side of the telescope till the graduation on

lastly, set the spirit-bubble level in every direction of the telescope by the parallel plate screws; which done, note the reading on the back staff, and enter it in the book; then examine the bubble, and again read the staff to ensure accuracy; then turn the telescope about, and do the same for the forward station, which will complete the second level. As the third and fourth, and all the following levels, are conducted in precisely the same manner, it will be unnecessary to repeat the instructions again.

The man holding the back staff should be instructed never to move it in the least from its position till the forward observation is completed, which he can always tell by seeing the surveyor carry his level onwards. It is sometimes the practice to use one staff only, and after taking the back observation, to cause the assistant to go on and take up a position suitable for a forward station; but besides the loss of time attendant upon such a process, if the instrument should in the interval get moved by accident, those two observations will be incorrect, unless the back sight be taken again, and this cannot be done unless the precise spot before occupied by the staff can be identified, which is sometimes uncertain. When this is the case, no alternative is left but to go back and renew the work at the last bench mark, or known station; and if none such exist, the whole operation will probably have to be gone over again, where great accuracy is required.

The iron tripod, described at page 16, should in all cases be placed on the ground by the staff-holder, to rest the staff upon, as it ensures to the observer the certainty of the staff keeping exactly the same spot when the face

movement of the person will derange the levels of the instrument, particularly on loose or elastic ground ;—to do away the inconvenience arising from this source, a reflector has been contrived to fix on the top of the telescope tube, by which the observer can see both the staff and the reflected image of the spirit-bubble at the same time, and then he can make his observation at the instant he sees the bubble in its proper position. The foregoing description of the method of taking levels is general, and applies equally to every kind of levelling operation, with whatever additional matters may require attending to, when taking levels for the formation of a section, &c., which we shall hereafter describe.

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lastly, set the spirit-bubble level in every direction of the telescope by the parallel plate screws; which done, note the reading on the back staff, and enter it in the book; then examine the bubble, and again read the staff to ensure accuracy; then turn the telescope about, and do the same for the forward station, which will complete the second level. As the third and fourth, and all the following levels, are conducted in precisely the same manner, it will be unnecessary to repeat the instructions again.

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The iron tripod, described at page 16, should in all cases be placed on the ground by the staff-holder, to rest the staff upon, as it ensures to the observer the certainty of the staff keeping exactly the same spot when the face

of it is presented to him in the two directions, forward and backward. The staff-holder should likewise be instructed to hold the staff perfectly upright, which he can himself determine, in one direction, by a little plumb-weight suspended in a groove in the staff; and as the observer can tell if he holds it upright in a lateral direction (as explained at page 15), he should frequently look to see if he signals for him to move the upper end of the staff to the right or left, taking care not to disturb its position on the iron tripod.

We have been supposing the use of the newly introduced staves, as we do not expect that those of the former construction will hold their ground against them, they having the advantage of providing to the observer the means of noting the reading of the staff himself. If however, from habit or otherwise, the use of the staff with the sliding vane should be preferred, the foregoing instructions equally apply; the only difference in its use is, that the observer must signal to the staff-holder to move the vane up or down on the staff, till it appears bisected by the cross wires of his telescope; then the reading of the staff must be noted, and entered by the assistant in a temporary book carried by him for the purpose: or if he cannot be trusted to perform so important a part of the business, he must convey the staff to the observer, or wait for him to come and read it himself. It requires no comment to show the uncertainty, and loss of time, in this method of proceeding compared with the use of the newly-contrived staves.

Having explained the method of taking observations for checking levels, we must refer to our example. The

levels, as before stated, were taken along the public road shown by the dotted line, that being the most convenient route from the town of A to the town of B, avoiding the necessity of passing through private property; the strong black line on the plan shows where the original section was taken; the section itself is represented above the plan, and is drawn to two scales; the one giving the horizontal measure, is the same as that of the plan, viz. one inch to one mile; and the vertical scale,  $\frac{1}{4}$  inch to 100 feet: from this section it appears that the crown of the bridge at A is fourteen feet above the datum line D E of the section, and that the bench mark (a stone by the road side) at B is 111 feet above the same datum; therefore the difference of level between the two places is  $111 - 14 = 97$  feet. Now, by referring to our observation book, of which we have subjoined a copy, we make the difference of level to be 96·8 feet, differing from the original section no more than two-tenths of a foot, or 2·4 inches, a quantity that may be disregarded; the inference, therefore, to be drawn from such a coincidence in the two results is, that the whole of the section between the points in question is sufficiently correct.

*Copy of Field-Book, for running or check levels.*

Back Sights.	Fore Sights.	Remarks.
0.34	3.16	Back $\Theta$ on B. M. on the bridge at A.
5.86	5.61	
4.19	4.24	Forward $\Theta$ at corner of road leading to B.
5.44	1.20	
4.96	3.20	
4.73	1.32	At crossing of line.
6.10	2.00	
5.33	3.96	
5.91	1.83	
5.70	0.90	
6.02	1.21	Staff placed on post notched for B. M.
1.21	4.00	At crossing of line.
3.53	6.07	
3.96	5.34	
3.94	4.81	
3.98	6.08	
4.08	4.94	Upon line.
3.90	3.96	
4.84	2.42	
1.54	5.12	
4.69	4.97	
5.04	1.60	
2.24	3.86	Upon line.
7.25	1.89	
4.03	1.30	
9.54	0.19	
6.70	1.70	
9.40	4.06	
6.44	0.38	
11.00	0.46	
5.98	1.30	
11.12	1.78	
9.84	2.20	
0.18	0.32	Upon line.
4.72	0.10	
8.89	0.77	
10.02	0.92	
10.00	1.03	
8.58	1.19	
9.53	1.18	
230.75	102.57	Sums.
102.57		
128.18		Difference.

*Copy of Field-Book—continued.*

Back Sights.	Fore Sights.	Remarks.
128·18		Brought forward.
9·90	0·68	
9·04	0·35	
10·00	8·52	
3·00	11·55	
3·68	0·88	
7·21	8·75	
1·99	10·48	
0·65	10·00	
4·48	10·44	
1·47	10·30	
1·55	11·70	
2·45	9·88	
3·78	1·04	
6·64	2·65	Forward $\Theta$ on B. M. called B.
194·02	97·22	Sums.
97·22		
96·80	· · · · ·	Difference = diff. of level between A and B.

The back sights being greater in amount than the forward sights, it is evident that the bench mark at B was higher than the bench mark at A by the difference of the two sums.

## LEVELS FOR THE FORMATION OF A SECTION.

Next to the running levels, the most simple case that can occur is, to take the levels of a line of country where the ground plan is already made, and the exact line of section determined upon, and in some instances picketed out. It is then only necessary, in addition to what is required for running levels, that the distance between the levelling staves, or the whole distance at every station from the starting point, be measured. The instrument should be placed, as usual, as near as



can be at an equal distance from each staff; but it is not essential that it be placed in the exact line between them, unless it should happen to prove the most advantageous position. Plate II. represents an example of this kind of work, the survey of the land having been completed, and the plan of the fields, &c. drawn: the strong black line A B was the direction determined upon as the most suitable for a portion of an intended line of railroad, and the section was accordingly taken; a bench mark had been previously agreed upon at each extremity (A and B), from whence other surveyors could take up the levels, and carry them onwards in both directions.

First a staff was placed on the bench mark at A for a back station, and another staff was held up for a forward station, in the adjoining field, but exactly on the line as marked down on the plan, a copy of which the surveyor had in his possession; the instrument was then set up, as near as could be estimated, or the level of the ground would admit, at an equal distance from each staff, so as to be able to read them both; the adjustment of the instrument for observation, as described at page 30, was carefully attended to, and the reading of the staves noted. As soon as the observations were made, the distance from staff to staff was measured with a Gunter's chain, which completed the first level.

The measurement of the distances can be more conveniently performed, and with a great saving of time, by two additional assistants, who can be measuring, whilst the surveyor proceeds to direct the man who held the back staff in the last case, to take up a forward station precisely on the line as laid down on the plan. The staff which was the forward station in the last case now

becomes the back station, and the instrument must be set up so as to read both stations as before, and as nearly equi-distant from them as can be: by the time the instrument is adjusted, and both the staves read off, the assistants would have completed the measurement from the bench mark A to the first forward staff, and be ready to continue on to the second one: whilst this is doing, the instrument and back staff can be carried forward and set up, &c., as before: by a continued repetition of a similar process, the whole line A B was levelled.

The measuring assistant should report to the surveyor the total distance of each forward staff from the bench mark at A as soon as it is determined, or, if thought more convenient, he may keep a book to enter the distances in, which should be ruled in two columns, one for his distances, and the other for references to them, as *a*, *b*, *c*, &c., or the numbers 1, 2, 3, &c., placed opposite; and if the observer makes similar notes in his book to each pair of sights, there can arise no mistake in placing the correct distances opposite the corresponding levels, when the measurer makes his return.

The following is a copy of the field-book of the example given in Plate II.; showing the manner of keeping it, and also the method adopted of reducing the levels to obtain the actual heights of each station, with regard to the starting point, for the purpose of drawing the section; which we shall then explain.

## LEVELLING FIELD-BOOK.

Dis- tances.	Rise.	Back Sight.	Fore Sight.	Fall.	Reduced Level.	Remarks.
519	5.83	13.71	7.88	—	+ 5.83	
1315	—	9.40	16.30	6.90	— 1.07	
1542	—	3.87	11.71	7.84	— 8.91	
1850	—	2.63	12.41	9.78	—18.69	
2358	13.67	14.62	0.95	—	— 5.02	
2698	15.55	17.00	1.45	—	+10.53	
3357	—	10.66	15.40	4.74	+ 5.79	
3758	—	2.87	17.00	14.13	— 8.34	
3976	—	3.40	10.32	6.92	—15.26	
5077	3.49	5.73	2.24	—	—11.77	
5904	15.69	16.54	0.85	—	+ 3.92	
6124	15.19	16.08	0.89	—	+19.11	
6437	13.83	14.56	0.73	—	+32.94	
7467	—	10.36	14.06	3.70	+29.24	
8369	8.48	9.84	1.36	—	+37.72	
9303	2.80	9.80	7.00	—	+40.52	
—	—	2.30	10.96	8.66	+31.86	Centre of road at 215 links.
9679	—	10.96	14.46	3.50	+28.36	
9936	—	2.08	15.05	12.97	+15.39	
10164	—	1.75	16.58	14.83	+ 0.56	
10576	—	1.84	17.10	15.26	—14.70	
11423	—	0.00	7.43	7.43	—22.13	Forward $\Theta$ at corner of Wood
13066	1.88	5.38	3.50	—	—20.25	
14954	4.00	8.50	4.50	—	—16.25	
15650	3.94	5.30	1.36	—	—12.31	
17345	0.80	10.20	9.40	—	—11.51	
19135	6.46	6.86	0.40	—	— 5.05	
19359	7.04	11.00	3.96	—	+ 1.99	
19631	8.27	11.80	3.53	—	+10.26	
19841	7.85	10.53	2.68	—	+18.11	Forward $\Theta$ at edge of Wood.
20561	6.84	8.22	1.38	—	+24.95	
21671	6.56	8.76	2.20	—	+31.51	
—	—	14.00	14.50	0.50	+31.01	Road at 450 links.
22710	10.18	14.50	4.32	—	+41.19	
23221	8.14	9.14	1.00	—	+49.33	B above A.
Sums.	166.49	304.19	254.86	117.16		
	117.16	254.86				
	49.33	49.33				

The first column contains the measured distances from the starting point to every forward station expressed in links of Gunter's chain. The two central columns, headed "Back Sight" and "Fore Sight," contain the readings of the two staves at the back and fore observations respectively. The *difference* of such readings is placed in one of the two side columns headed "Rise" or "Fall," according as the ground at the forward station is higher or lower than that at the back station. If it be highest (or the ground rises, as it is called), then the forward reading will be the smaller of the two: but if it be the lowest (or the ground falls), then the forward reading will be the greater of the two: thus, in our first reading, the back observation is 13·71, and the forward observation 7·88, their difference = 5·83 feet, which is the difference of level between the two points; and as the forward reading was the smaller of the two, it is clear that the ground was rising at that place, and, therefore, the difference of the readings, viz. 5·83, is placed in the column of Rises. In the next three successive pair of sights, the forward readings are the greatest, indicating a continued descent of the surface line, and the differences of those readings are inserted in the column of Falls, viz. 6·90, 7·84, and 9·78. At the next following sight, the forward reading is again the smallest, therefore the difference 13·67 is placed in the column headed "Rise," and so on of the rest. No mistake can arise by placing the subtraction in the wrong column, as in every instance it must be placed in the column adjoining the larger quantity; thus if the fore sight is greater than the back sight, the resulting quantity must be placed in the column of falls,

which is adjoining to that containing the reading of the fore sight, and *vice versâ*.

The adjoining column, headed "Reduced Levels," contains the absolute heights of each forward station above the datum line of the section, or a horizontal line passing through the starting point or bench mark A: these quantities, which are technically called the reduced levels, are obtained by the constant addition and subtraction of the numbers contained in the columns of "Rise" and "Fall," the former being considered as positive, and the latter as negative quantities; thus, assuming the level of the starting point A as the datum, we have the first forward station 5·83 feet higher than the datum, therefore in the column of reduced levels it is marked + (plus): next we have a fall or negative quantity of 6·90 feet, which must be subtracted; but as it is greater than 5·83, it shows that this station is below the datum line, by the difference between 5·83 and 6·90 = 1·07 feet, which is the depth of the second forward station *below* the datum line, and therefore is marked - (minus): the next is likewise a fall of 7·84, and as our last result was below the datum line, this additional negative quantity will take us still lower by its whole amount; it must, therefore, be added to 1·07, giving 8·91 feet for the depth of our third forward station below our datum, and it is therefore entered in the column of reduced levels with a minus sign. The next is also a fall of 9·78, which, applied as the last, gives 18·69 for the depth of the fourth forward station below the datum. The ground then rises again, and we have an ascent of 13·67 feet, which will bring us nearer to our datum; and as it diminishes our depth below the

datum line, it must be subtracted from the last result; thus,  $18.69 - 13.67 = 5.02$  feet for the depth of the fifth forward station below the datum; we have then a rise of 15.55, which will carry us above the datum by the amount of difference between it and 5.02, leaving 10.53 feet for the height of the sixth forward station above the datum line: the next is a fall of 4.74, which diminishes our height by that quantity, and therefore must be subtracted from 10.53, leaving 5.79 as the height of the seventh forward station above the datum.

In like manner every other pair of sights in our example was reduced, applying each difference of the back and forward readings with their proper signs, until, at the close of the work, the point B (the last forward station) was found to be 49.33 feet above the datum line, or level of the starting point A.

The reduction of levels becomes a simpler operation when the height of the bench mark (used as a starting point) above the intended datum line is known: thus (in our example), suppose the height of the bench mark A was 100 feet above the level of Trinity high-water mark at London Bridge, and that it was intended to assume the level of that mark as the datum line of our section; then 5.83 feet, the rise to the first forward station, must be added to 100, giving 105.83 for the height of the ground at the point *a* above datum; next, from 105.83 subtract the fall 6.90, which gives 98.93 for the height of the point *b* above datum; then from 98.93 subtract 7.84, which gives 91.09 for the height of *c* above datum; and in like manner, by adding the quantities of rise, and subtracting those of the falls, the whole line of levels may be reduced to the line assumed as the datum.

As a proof of the accuracy of the arithmetical operation, the columns of back and fore sights should be added up, and the lesser sum subtracted from the former; the result of the agreement with that by the reduced levels is a proof of accuracy. Likewise another proof may be obtained by adding up the contents of the columns of "Rise" and "Fall;" and if upon taking the lesser sum from the greater, the remainder represents the same quantity as obtained by both the other operations, there can be no doubt of the correctness of the reductions of the levels, as in our example. By the reduced levels, the height of B above A is 49·33 feet. The sum of the back readings is 87·95, and that of the forward readings 38·62; their difference also gives 49·33 for the height of B above A; and, lastly, the sum of the rises is 54·88, and that of the falls is 5·55, the difference giving, as before, 49·33 feet.

It is, perhaps, to be recommended, that the observer should reduce his levels as he proceeds in the field, as it will occupy but very little time, and can be frequently done while the staff-man is taking a new position; besides, the observer will frequently be able to detect by the eye if he is committing any glaring error, as, for instance, inserting a number in the column of Rises, when it ought to occupy a place in that of the Falls, the surface of the ground at once reminding him that he is going down hill instead of ascending.

If the foregoing method of reducing levels be found difficult or troublesome, on account of the introduction of plus and minus signs, they can be dispensed with, as well as the columns of "Rise" and "Fall," by proceeding in the following manner. Assuming the starting

point to be any even number of feet high; or, what is the same thing, assume a datum line any even number of feet below the starting point, as 100 or 1000, taking care that your choice falls upon a number greater than the number of the whole fall you are likely to experience in the operation; then from this assumed height *subtract* the reading of the forward staff, and to the remainder *add* the reading of the back staff; the result will be the height of the first forward station above the assumed datum line; then from this height subtract the next forward reading, and to the remainder add the reading of the back staff; the result will be the height of the second forward station above the assumed datum, and so on throughout the whole levelling operation. The difference between any two of the readings will be the difference of level between the corresponding points on the ground.

By way of illustration, we will reduce part of the foregoing example after this manner, and the student can adopt whichever method he may consider the best.



Back Sight.	Fore Sight.	Reduced Levels.	Remarks.
13·71	7·88	100·00 7·88	Assumed datum.
		92·12 13·71	
9·40	16·30	105·83 16·30	{ Height of 1st forward station above assumed datum.
		89·59 9·40	
3·87	11·71	98·93 11·71	Height of 2nd do. above do.
		87·22 3·87	
2·63	12·41	91·09 12·41	„ 3rd do. „ do.
		78·68 2·63	
14·62	0·95	81·31 0·95	„ 4th do. „ do.
		80·36 14·62	
17·00	1·45	94·98 1·45	„ 5th do. „ do.
		93·53 17·00	
10·66	15·40	110·53 15·40	„ 6th do. „ do.
		95·13	
2·87	17·00	105·79 17·00	„ 7th do. „ do.
		88·79 2·87	
		91·66	„ 8th do. „ do.

The above will, we trust, be found sufficient to make ourselves understood upon the subject of reducing levels. If, after adopting the latter mode, it should be required to reduce them to the level of the starting point as a datum, nothing more is required than to take the difference between the height thus found and that of the

assumed datum; thus, in our example, subtracting 100 (the assumed datum) from the height of the first forward station, gives 5·83 for its height above the starting point: next, from 100 subtract  $98\cdot93 = 1\cdot07$ , making the second forward station that quantity below the level of the starting point, and so of the rest. But it may be done much easier after the section is made to the assumed datum, by drawing a line parallel thereto through the point A, or any other that may be determined on; thus the section may be at once adapted to any required datum line.

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#### TO DRAW THE SECTION.

The levels being reduced, the surface line may be represented in the form of a section, as shown above the plan in Plate II. The vertical and horizontal scales of a section are seldom the same, which produces a caricatured representation; the vertical scale being so much greater than the horizontal, shows the depths of cutting and embankment required in the execution of road, railway, or canal works, with greater clearness than if both scales were equal. The plans and sections of projected works deposited with the Clerks of the Peace of counties, and in the Private Bill Office, to obtain the sanction of the legislature, are mostly drawn to scales of four inches to one mile horizontal, and one hundred feet to one inch vertical: we have adopted these scales in our example, Plate II.

To make the section of our present example, first draw the horizontal line C D as the datum to which our levels were reduced, assume any point A as the starting point, then set off the measured distance from A to the

first forward station  $a = 519$  links (see levelling field-book, page 42), at this point erect a perpendicular, and mark on it the height 5.83 of the first forward station, and connect the point A with this mark, and the result will show the surface line of the ground in that interval : next, from the same starting point A set off the point  $b$ , the second forward station, with the distance of 1315 links, as given in the levelling-book ; but as this point is a minus quantity (see reduced level, page 42), that is, below the datum line, let fall a perpendicular, and set off on it 1.07 feet, which connect by a line with the former level, and the surface line from A to  $b$  will then be represented ; then with the distance 1542 set off the point  $c$ , and on a perpendicular let fall therefrom, set off 8.91, which connect as before, and the section will be complete from A to  $c$ . In like manner, proceed with the rest of the reduced levels at the points  $d, e, f$ , &c., till the whole section is drawn.

Although, for the sake of clearness of description, we have desired the person plotting the section to draw the perpendicular, and thereon define the level point of the surface as he proceeds with setting off the horizontal distances step by step, yet in practice he will find it most expeditious in the first instance to place the chamfered edge of his ivory scale for the distances along the datum line, and at once to prick off the whole of the distances (or any convenient portion of them) successively as the numbers appear in the field-book ; then draw all the perpendiculars by means of a parallel ruler, or by a T square if the paper is properly fixed on a drawing table ; and, lastly, from the vertical scale prick off all the perpendiculars and connect those points, and the section will be made.

The distances given in the proper column of the field-book are supposed to be horizontal distances, and, in measuring them, care should be taken that they are as nearly such as possible (or they must afterwards be reduced thereto), otherwise the section will be longer than it ought to be. For the purpose of assisting the surveyor in making the necessary reduction from the hypotenusal to the horizontal measure, when laying down his section, we annex the following Table, showing the reduction to be made upon each chain's length, for the following quantities of rise, as shown by the reading of the staves:—

Rise in feet for one chain.	Reduction upon one chain in links and decimals.
1	0·01
2	0·04
3	0·11
4	0·19
5	0·29
6	0·44
7	0·56
8	0·74
9	0·94
10	1·16
11	1·40
12	1·76
13	2·01
14	2·24
15	2·61
16	2·99
17	3·39
18	3·76
19	4·23
20	4·64

The section can be referred to any other datum than the one by which it was produced; as, for instance, let it be required to refer the section, Plate II., to a datum line 100 feet below the point A; all that is required to

be done is, to draw a line  $EF$  parallel to  $CD$ , at 100 feet below it; then, by drawing perpendiculars from the surface line to this new datum, as shown by the dotted lines, the transfer will be complete, as the height of any point can be measured by the scale of the section. We need not go through a further explanation of this subject, as an inspection of our engraved example will explain whatever further may be required.

#### WORKING SECTION.

For the purposes of carrying into execution any work, the section should be much more minute than is requisite for general purposes; it is then called a working section. The following are the field notes taken for such a section, the line having first been carefully set out and a stake driven into the ground at the extremity of each chain's length: these stakes were about 18 inches long and 2 inches square (and were furnished by a country wheelwright at the price of ten-pence per dozen); every tenth stake was circular, and somewhat larger, and had an iron ring round its top, and together with every fifth stake had their tops painted white, the more easily to identify them; they were all numbered (or considered to be numbered) from one end of the line to the other. Plate III. shows the section of the ground and railway at the extreme end of the line where the numbers terminate at 1103 chains or  $13\frac{3}{4}$  miles and 3 chains: we would recommend the student to plot this section from the notes several times, and to various scales, that he may not only better understand the subject, but also for the sake of practice, it being an

actual example from the working section of a line of railway now completed and opened to the public.

• FIELD NOTES—WORKING SECTION.

Rise.	Back Sight.	Fore Sight.	Fall.	Distance.	Reduced Levels.	Remarks.
feet.	feet.	feet.	feet.	Links.	feet.	
					270.72	Brought forward (from last page of NOTES).
	4.47	4.53	0.06	103300	270.66	
	4.53	9.22	4.69	103400	265.97	
4.15	9.22	5.07		103500	270.12	
4.83	5.07	0.24		103600	274.95	
4.49	6.36	1.87		103700	279.44	
4.67	6.14	1.47		103800	284.11	
4.52	6.62	2.10		103900	288.63	{ Side of clapping post of field gate in occupation road.
	2.10	2.24	0.14	103916	288.49	
0.95	10.42	9.47			289.44	{ Lower hanging hook of gate. Centre of occupation road.
	9.47	13.22	3.75	103944	285.69	
0.07	13.22	13.15		103956	285.76	Edge of road.
4.40	13.15	8.75		103966	290.16	Top of bank.
4.27	8.75	4.48		103976	294.43	Do. do.
0.16	4.48	4.32		104000	294.59	
	2.44	8.84	6.40		288.19	B.M. southside of line.
6.01	8.84	2.83		104100	294.20	
	0.74	2.18	1.44	104200	292.76	
	2.18	5.35	3.17	104300	289.59	
	6.77	7.28	0.51	104400	289.08	
0.03	7.28	7.25		104490	289.11	Edge of ditch.
	7.25	8.36	1.11	104492	298.00	Bottom of ditch.
4.79	8.36	3.57		104500	292.79	Stump, top of bank.
0.62	3.37	2.75		104600	293.41	
1.32	2.75	1.43		104700	294.73	
	1.10	2.25	1.15	104800	293.58	
	2.25	8.88	6.63	104900	286.95	Enter alder plantation.
	5.65	9.53	3.88	104920	283.07	
	9.53	11.50	1.97	105000	281.10	
0.33	5.85	5.52		105021	281.43	
	5.52	12.01	6.49	105100	274.94	
	12.01	12.87	0.86	105148	274.08	
2.10	12.87	10.77		105190	276.18	
2.18	10.77	8.59		105200	278.36	{ Foot of bank, which rises perpendicularly 1 foot.
7.19	8.59	1.40		105300	285.55	

## FIELD NOTES—WORKING SECTION.

Rise.	Back Sight.	Fore Sight.	Fall.	Distance.	Reduced Levels.	Remarks.
					285.55	Brought forward.
3.80	8.22	4.42		105400	289.35	
1.45	4.42	2.97		105500	290.80	
	2.97	3.39	0.42	105600	290.38	
	3.39	5.51	2.12	105700	288.26	
	5.51	7.67	2.16	105800	286.10	
	5.41	6.68	1.27	105827	284.83	Edge of ditch.
	6.68	8.56	1.88	105832	282.95	Bottom of ditch.
2.48	8.56	6.08	6.30	105837	285.43	Top of bank.
	6.08	12.38	4.34	105854	279.13	Foot of bank.
	12.38	16.72	0.62		274.79	
	2.04	2.66	3.82	105900	274.17	
	2.66	6.48	2.38	105940	270.35	Edge of ditch.
	6.48	8.86		105944	267.97	Bottom of ditch.
2.86	8.86	6.00	1.58	105952	270.83	Top of bank.
	6.00	7.58	3.16	105960	269.25	Foot of bank.
	7.58	10.74	4.91	106000	266.09	
	3.33	8.24	0.91	106095	261.18	Top of bank.
	8.24	9.15	4.19	106100	260.27	Stump side of bank.
	9.15	13.34		106105	256.08	Bottom of ditch.
1.69	13.34	11.65	1.15	106110	257.77	Edge of ditch.
	11.65	12.80	0.51	106200	256.62	
	3.62	4.13		106300	256.11	
0.75	4.13	3.38		106349	256.86	Foot of bank.
2.88	3.38	0.50	3.85	106359	259.74	Top of bank.
	0.50	4.35		106368	255.89	Bottom of side drain.
0.27	4.35	4.08	0.23	106386	256.16	Centre of parish road.
	4.08	4.31		106405	255.93	Foot of bank.
3.74	4.31	0.57	2.45	106415	259.67	Top of bank.
	0.57	3.02	0.41		257.22	
	0.99	1.40	1.43	106430	256.81	Foot of bank.
	1.40	2.83	1.58	106500	255.38	
	2.83	4.41	0.07	106600	253.80	
	4.41	4.48		106700	253.73	
0.46	7.80	7.34		106800	254.19	{ (Crosses foot-path at 106831.)
2.93	7.34	4.41		106900	257.12	
3.67	4.41	0.74		107000	260.79	
4.20	10.63	6.43		107100	264.99	
5.06	6.43	1.37		107200	270.05	
5.79	10.76	4.97		107300	275.84	
3.85	4.97	1.12	0.05	107400	279.69	
	5.42	5.47		107500	279.64	
0.91	5.47	4.56	0.44	107600	280.55	
	4.56	5.00	0.56	107637	280.11	Edge of ditch.
	5.00	5.56		107640	279.55	Bottom of ditch.

## FIELD NOTES—WORKING SECTION.

Rise.	Back Sight.	Fore Sight.	Fall.	Distance.	Reduced Levels.	Remarks.
					279.55	Brought forward.
3.16	5.56	2.40	0.56	107647	282.71	Top of bank.
	2.40	2.96		107654	282.15	Foot of bank.
1.58	2.96	1.38		107700	283.73	
8.84	9.45	0.61		107800	292.57	
5.11	8.44	3.33		107853	297.68	Enter plantation.
2.91	3.33	0.42	1.50	107857	300.59	B. M. on timber stub.
	12.78	14.28		107882	299.09	
4.27	14.28	10.01		107900	303.36	
8.75	10.01	1.26		107947	312.11	
10.42	14.49	4.07		108000	322.53	
1.21	4.07	2.86	0.49	108008	323.74	
	2.86	3.35		108024	323.25	
2.98	3.35	0.37		108047	326.23	
15.33	16.35	1.02		108098	341.56	{ Top of bank, edge of plantation.
	1.02	1.32	0.30	108100	341.26	
2.74	8.66	5.92		108200	344.00	
0.78	5.92	5.14		108300	344.78	
	5.14	8.43	3.29	108400	341.49	
	1.05	4.50	3.45	108500	338.04	
	4.50	4.94	0.44	108520	337.60	
	4.94	6.83	1.89	108530	335.71	Edge of bank.
	6.83	12.54	5.71	108540	330.00	Foot of bank.
	12.54	16.82	4.28	108600	325.72	
	1.11	9.04	7.93	108700	317.79	
	1.18	9.09	7.91	108800	309.88	
	1.57	9.70	8.13	108900	301.75	
	1.28	9.58	8.30	109000	293.45	
	1.44	9.41	7.97	109100	285.48	
	1.34	9.14	7.80	109200	277.68	
	1.15	8.12	6.97	109300	270.71	
	3.04	4.43	1.39	109386	269.32	Edge of ditch.
	4.43	6.22	1.79	109390	267.53	Bottom of ditch.
1.06	6.22	5.16		109400	268.59	Stamp, top of bank.
	5.16	11.10	5.94	109405	262.65	Foot of bank.
1.81	11.10	9.29		109500	264.46	
3.10	8.87	5.77		109600	267.56	At post and rail fence.
1.17	4.63	3.46		109700	268.73	Edge of slope.
	3.46	7.06	3.60	109800	265.13	
	1.96	4.60	2.64	109900	262.49	Foot of slope.
	4.60	4.60		110000	262.49	
0.93	4.60	3.67		110149	263.42	
	4.58	5.03	0.45	110377	262.97	Stamp, end of curve.
0.63	5.03	4.40			263.60	On rails at end of curve.
	4.40	4.58	0.18		263.42	B. M. foot of post.
3.53	4.58	1.05			266.95	Top of said post.



In taking levels for a minute section where the observations must be very numerous, and consequently the back and fore sights not very far from each other, the observer will frequently be able to make a number of observations at each setting up of the level at one side of his line, so that his instrument may be about equally distant from his back and fore observations. Due attention to this will save much time and labour, and experience will enable the surveyor at a glance to see where he can set up his level at every remove forward with the greatest advantage. Upon looking down our field notes above, it will be seen it seldom occurred that only one back and one fore sight was obtained at a setting up of the level, and this only took place where the ground was very steep: by the first setting up of the instrument four forward sights were observed, and of course as many back ones; thus the first back sight was 4·47, the corresponding fore sight 4·53; this latter number was also placed as the back sight for the next observation, which was 9·22; this number was in like manner placed as the back sight for the next forward observation, 5·07, which also became the back sight for the last forward observation we could obtain at that setting up of the instrument, namely, 0·24: it should here be remarked that there was a necessity to place each forward reading as a back observation to the next forward reading, otherwise the difference of level between each point of observation would not have been obtained without more arithmetical work; the numbers otherwise only show the difference of level between each and the first point of observation; besides, by this arrangement, the whole section is continuous, however numerous the

intermediate observations may be, and having the distances opposite, the whole can be plotted off with facility. The columns of "Rise" and "Fall" need no observation after what has already been said upon this subject. The column of Distances denotes the continuous measurements from the commencement, Gunter's chain being the unit employed. Our notes commence at the 1033rd chain, and terminate with the end of the work, 1103 chains and 77 links, which we consider an ample extract for the purposes of the student. The column headed "Reduced Levels" contains the height of each point of observation above the datum line, which in this case was Trinity high-water mark, London Bridge: these numbers are obtained by adding the "rises" and subtracting the "falls" from the preceding reduced level, which in our notes commence with 270.72 feet.

#### THE SECTION.—SEE PLATE III.

The datum line must be drawn, every chain should then be pricked off and the perpendiculars erected; the chains or stakes should then be numbered beneath the datum line, to prevent mistakes, and just above the datum line the height of the surface at each stake should also be inserted; then the said heights can be pricked off upon the perpendiculars respectively, and the intermediate heights plotted from the field notes without fear of error, which otherwise, without great care, would be likely to occur in consequence of so many points falling near to each other, unless the scale be very large: the horizontal scale of the example is 1 inch to 5 chains, and the vertical scale 1 inch to 25 feet. Having

drawn the undulating line of the surface through these points upon the perpendiculars, the gradients or intended line of railway may next be laid down; the extreme left-hand point was given, being the level of the rails at the point of junction with another line. The railway is represented by two parallel lines, the upper one being the upper surface of the rails, and the lower one the bottom of the ballasting or formation level, being 2.25 lower than the surface of the rails: for a short distance the line is level, then it rises at the rate of 20 feet per mile, for the two-fold object of diminishing the great cutting and of getting sufficiently high over the road at stake 1064, to allow (with the lowering the surface of the said road a small quantity) of sufficient headway for the public carriages to pass under the railway: from this point the line falls at the rate of 20 feet per mile for a considerable distance, the object being to get as low down as possible further to the eastward, where there was to be a considerable embankment, and by these means such embankment was reduced in dimensions; and furthermore, the earth from the cutting to the right of the road was to be taken eastward to form the said embankment, and therefore the down-hill gradient was favourable for carrying on the work as well as for the drainage of the cutting. Part of the earth from the large cutting was also to be taken to the eastward; the ascending gradient, up to the bridge, was unfavourable for this purpose, however, so far as the bringing out the bottom of the cutting, the upper part being brought down by means of inclined planes: the ascending gradient was unavoidable in this case, but by judiciously working the excavation, little

inconvenience and extra expense attended it. Each change of gradients is denoted by a strong vertical line from the datum to the point of change, and the height marked thereon. The quantity of earth-work to form the cuttings and embankments with different slopes should be written upon them, as shown in our example; also over the line of figures denoting the height of the surface above the datum should be placed the depth of the cutting from the surface to formation level at the same point, or the height of the embankment, as the case may be: these heights and depths are those from which the calculations of the quantities are to be made, and therefore must be strictly correct; they should not be taken from the section by the scale, but should be obtained by calculation: the former method being liable to error. The calculation may be thus performed. Let it be required to find the depth of cutting at stake No. 1083, where the height of the surface above datum is 344·78 feet; at stake No. 1064, the height of formation level above datum is 269·20, from which point the gradient descends at the rate of 20 feet per mile, or 0·25 feet per chain, towards No. 1083; the distance from 1064 to 1083 is 19 chains, which multiplied by 0·25, gives 4·75 for the fall of the railway in the interval between the two points; consequently the height of the railway above datum at No. 1083 is  $269\cdot20$ , minus  $4\cdot75=264\cdot45$ ; this sum, subtracted from the whole height of the surface, gives  $344\cdot78-264\cdot45=80\cdot33$ , for the depth of the cutting at that point, and so of all the remaining numbers. After giving the above particulars nothing need be added upon this subject. It may be worth observing, that in laying down the

gradients care should be had so to dispose them as to produce the minimum quantity of work in the execution, and that the cuttings should equalize the embankments, or, if anything otherwise, they should be a little in excess, to allow for subsidence or slips in the embankments. The facilities for working the excavations and carrying the earth to bank should also be considered; a down-hill gradient in that direction is most suitable, provided it can be obtained without interfering with other and often more important considerations: the drainage of the works during the formation and after the line is completed should also be considered at the time of determining the gradients. We have inserted (Table I. at the end of the work) a very extensive and useful Table of Gradients, which is sufficiently self-explanatory as not to require further notice.

When a surveyor is required to level through a country in a perfectly straight line, and has not the advantage of its being picketed or poled out, his only means to keep a rectilineal course is by ascertaining, as accurately as possible, the magnetic bearing of one extremity from the other, and work in that direction by means of a compass. We once had business of this kind, and determined the bearing of our intended line from the map of the Ordnance survey (allowing for the variation of the needle), and after pursuing the route thus determined, we were surprised and delighted at finding how exactly we came to our required point, convincing us (if a proof had been required) how justly the public confidence has been placed in our national survey.

It is seldom the case in practice that the instrument can be placed precisely equi-distant from the back and

forward staves, on account of the inequalities of the ground, &c. It would appear, therefore, to be necessary, to make our results perfectly correct, to apply to each observation the correction for curvature and refraction, as explained in the early pages of our book; this, however, we believe, is seldom done unless in particular cases, where the utmost possible accuracy is necessary, on account of the smallness of such correction, as may be seen by referring to our Table, page 9, where the correction for eleven chains is shown to amount to no more than  $\frac{1}{100}$  of a foot; and as the difference in the distances of the instrument from the back and fore staves can in no case equal that sum, it is evident that such correction may be safely disregarded in practice.

Several machines have been constructed or designed for the purpose of describing a section of any ground passed over by the instrument, which at the same time would register the distance passed over, as well as the undulations: perhaps the best of this kind was the one designed and constructed by George Edwards, Esq., Civil Engineer, of Lowestoft, which is fully described and illustrated in the forty-fourth volume of the "Transactions of the Society of Arts," page 123, to which we refer. The use of such machines, however, must, from the nature of the work to be performed, be of a very limited character.

We have now described the leading principles and practice of levelling as employed in engineering operations; and although our observations may appear to be confined to its applicability to railroad purposes, yet the intelligent student will find no difficulty in applying to practice the same principles to every other branch of the

profession where levelling operations may be required. We might indeed have multiplied instances and examples which would in reality have had no other effect than to swell our volume, as it must have been, to a great extent, but simply a repetition of the details already given.

Before closing this subject we cannot refrain from stating, that it has long been our opinion that if a register could be kept by some public body (as the Institution of Civil Engineers) of the height of particular spots throughout the kingdom, above some given datum as Trinity high-water mark, London Bridge, or any other that might be agreed upon, such a record would be invaluable both in a particular and national point of view: to the engineer and geologist it would be most important, and the whole register could be prepared from time to time at a trifling cost, if each engineer and surveyor would but contribute to the common stock by sending to head-quarters the level of any particular spots as he, in the course of his professional engagements, may have opportunity of determining them. We consider that no time is likely to be so favourable for the purpose as the present, as nearly the whole country has been levelled over for railway purposes within the last few years; and no doubt the field notes of the greater part are still in existence, from which a great many such standard levels could be extracted by the parties who took the levels, and which in a few years it will be impossible to eliminate. By way of showing more fully our meaning, we have extracted from our own levelling books a few such standard levels, and arranged them after the manner we have above alluded to.

COUNTY OF KENT.

	Height in feet above Trinity high-water mark, London Bridge.
Upper edge of tablet over door of No. 1 Martello Tower, near Folkestone . . . . .	256·4
Top of first milestone on the road from Folkestone turnpike to Dover . . . . .	402·9
Top of second milestone, do. . . . .	510·7
Surface of ground at Folkestone turnpike gate . . . . .	534·6
Dock wall at Dover, opposite Railway Office . . . . .	7·4

COUNTY OF SURREY.

Surface of ground at New Chapel turnpike gate . . .	188·0
Waste board of Godstone Ponds, back of White Hart Inn .	319·2
Top of twentieth milestone (from Westminster Bridge) on the road from Godstone to East Grinstead . . .	287·6
River Medway (tributary stream) meadows, west side of turnpike road, at Blundley Heath . . . . .	151·2
Broadham Green, near Oxted, foot of pointing post . .	268·2

COUNTY OF SUSSEX.

Honey-pot Lane, South Chailley Common . . . .	122·5
Gullage Farm, source of the Medway, near the barn . .	324·6
Waste weir canal (east side of Lindfield) . . . .	82·1
Summit of South Downs at Plumpton Plains . . . .	682·5
„ „ at Mount Harry . . . .	573·3
Turnpike road, Brighton to Lewes, near the barracks .	85·1
Cross roads, at Turner's Hill turnpike gate . . . .	535·2

## LEVELLING WITH THE THEODOLITE.

The application of the theodolite to the practice of levelling is an operation of great simplicity. We must suppose the reader to be already acquainted with the construction and method of measuring angles with that



valuable instrument ; and those who have no such knowledge, we refer to the *Treatise on Mathematical Drawing Instruments* spoken of, where every particular respecting it may be found. The ordinary 5-inch theodolite, of the best construction, is the one we recommend to the use of the surveyor, it being sufficiently accurate for most purposes that fall within his province, and is convenient to use on account of its portability. A larger theodolite is seldom employed, except on surveys of great extent upon trigonometrical principles, as those of the United Kingdom under the direction of the Board of Ordnance, where theodolites of 3 feet diameter have been employed to obtain the requisite degree of accuracy.

To use the theodolite in the common purposes of levelling, it is only necessary to set the instrument up at every spot on the line of country to be levelled, where the inclination changes, without regard to the minor inequalities of the surface, taking care that the adjustments have been carefully examined and rectified, as explained in the book above alluded to, especially those adjustments which set the line of collimation, and the spirit-level attached to the telescope, parallel to each other. Then set the instrument level by means of the parallel plate screws, and direct an assistant to go forward with a staff, having a vane, or cross piece, fixed to it exactly at the same height from the ground as the centre of the axis of the telescope. Having gone to the forward station, the assistant must hold the staff upright, whilst the observer measures the vertical angle, which an imaginary line connecting the instrument and staff makes with the horizon; the instrument and staff should

then change places, or, to save time, another staff should take the place of the instrument, and the instrument be removed to the former staff, and from thence the same angle should be taken back again, and the *mean* taken as the correct result.

The distance must then be measured, which will furnish all the data required to find the difference of level between the places of the instrument and staff; this, it will appear evident, is a matter of trigonometrical calculation,\* the measured distance being considered as the hypotenuse of a right-angled triangle, of which the perpendicular is the difference of level. It scarcely appears necessary to give the rule for the calculation, but for the sake of uniformity we shall do so.†

*Add together the logarithm of the measured distance, and the log. sine of the observed angle; the sum, rejecting 10 from the index, will be the log. of the difference of level, in feet or links, &c., the same as the distance was measured in.*

If the distance be measured with Gunter's chain, the result (in chains) can at once be obtained in feet, by simply adding to the above two logarithms the constant 1.8195439, which (10 being rejected from the index) will give the log. of the height in feet.

In this manner, by considering the surface of every principal undulation as the hypotenuse of a right-angled triangle, the operation of levelling may be carried on with great rapidity, but, it must be remarked, without pretensions to great accuracy; in fact, in that particular, the use of the spirit-level will never be superseded.

\* Capt. Frome's Work, in 8vo., published 1840.

† See Appendix I.

Another method of applying a theodolite to the purposes of levelling was introduced by Sir John Macneill. He caused to be constructed, by Messrs. Troughton and Simms, a more powerful instrument for the purpose. It was a combination of the level and the theodolite. He set it up at the foot of an inclination, and sent a man on with a staff as above described; and whilst the observer was looking through the telescope, another assistant walked along the line, holding up another staff at every rise and hollow of the intervening surface, and thereby the observer could note how much such rises and hollows were below the line of his vision. The distances from the instrument to the points where the staves were held up could then be measured, and the section drawn by simply ruling a line at the angle of elevation given by the instrument (or, more correctly, by computing the total elevation and setting that up as a perpendicular; and drawing the hypotenusal line thereto), and marking thereon the measured distances, and from such marks drawing perpendiculars of the various lengths indicated by the staff at its different positions: a line connecting the extremities of the perpendicular will represent the section of the surface line.

Instead of measuring the distances, Sir John Macneill had attached to the eye-end of the telescope a beautifully-made wire micrometer, similar to those applied to astronomical telescopes, by which he could tell with sufficient accuracy the distances required. This method of levelling, like the former by the theodolite, will give but a general approximation to the truth, depending in a great degree upon the quality of the instruments, and the care bestowed upon the operation.

## PART III.

COMPUTATION OF EARTH-WORK—ROAD-MAKING—  
THE CLINOMETER, ETC.

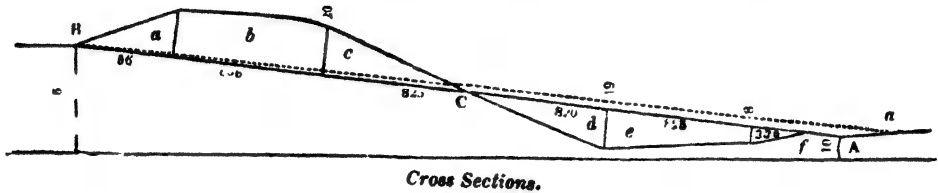
WE have now to show the manner of applying a section to practical purposes. If the object to be attained is the making of a railroad, it is essential that it be formed as nearly level, and as perfectly straight, as the surface of the ground will admit of; for the nearer it approximates thereto, the more profitably will it be worked when completed, as locomotive steam-engines perform the most work with the least expense when the resistance they have to overcome is uniform and invariable. The same remarks hold with respect to a turnpike road; but the inclinations on the latter may be made greater and more variable, being worked by animal power, which is capable of putting forth, on a sudden emergency, a greater exertion for a short time, which is not the case with elemental or mechanical power beyond limits much short of what an animal is capable of.

Sir Henry Parnell, in his valuable Treatise on Roads, recommends that a road should not be made steeper than 1 in 35; that is, for every 35 feet in length of road surface, the difference of level will be 1 foot, that being an inclination which presents no difficulty to fast driving either in ascending or descending. But on a line of

railroad to be traversed by locomotive engines, no rate of inclination, or gradient, as it is called,\* should exceed 20 feet in a mile, or 1 in 264. To draw the lines of proposed surface (or gradient) upon a section, which shall be the most suitable for the purposes intended, and at the same time to be the most economical in the execution, that is to say, to have the least possible quantity of earthwork in cuttings and embankments, requires judgment and experience: no definite rules can be given for this purpose, as no two sections present the same undulating surface. There is one material point we would suggest, and which should be carefully attended to; viz., that for every piece of cutting, there should be an equal, or *rather less*, quantity of embankment. We say *rather less* because every newly formed embankment experiences a settlement to a greater or less degree, and therefore more earth will be required to raise it to a proper level. The excess of the cuttings above the embankments should never be great, otherwise the surplus would have to be disposed of in mounds, termed spoil-banks. In no case whatever should the required embankments exceed in cubical contents the quantity of cuttings; for then a serious difficulty occurs—land has to be purchased for the purpose of digging earth to supply the deficiency, which is usually called side cutting.

\* Sir John Macneill, in his preface to his valuable translation of M. Navier's little work on the "Means of comparing the respective Advantages of different Lines of Railway," says, "I have rendered the word *peute* by *slope*, in preference to *inclination*, *inclined plane*, or *gradient*, considering the two former, though generally used, as improper expressions; and the latter, to say the least of it, as having so little to recommend it, that I hope it will have an extremely short existence in our nomenclature."

Suppose in the cut below the upper figure to represent the section of an old line of road, and that it were required, by cutting and embankment, to reduce it from its present hilly surface to one uniform rate of inclination from the point A to the point B. The lower



extremity A is 10 feet above the datum line of the section, and the higher point B 46 feet above the datum; consequently,  $46 - 10 = 36$  feet, the rise from A to B, and the distance 4356 feet, which, divided by the rise (36), will give 1 in 121 for the rate of inclination the road may be brought to.

Upon the section draw the straight line AB, which will show the extent of cutting and embanking to be made. The number of cubic yards of earth to be removed in the cutting between the points B and C, and the cubical contents, in yards, of embankment between C and A, may then be computed in the following manner:

Divide the quantities of cuttings and embankments, as shown upon the longitudinal section, into triangles and trapeziums, determined by the undulations of the surface lines, as shown in the above engraving, where, in the cuttings, *a* and *c* are triangles, *b* a trapezium; and in the embankments *d* and *f* are triangles, *e* a trapezium. The form of the excavation and embankment

is shown by the transverse or cross sections. Let the width of the roadway (or base of the cutting, and top of the embankment) be 50 feet, including the foot-path, &c., on each side; the slope of the cutting to be  $1\frac{1}{2}$  to 1, that is,  $1\frac{1}{2}$  horizontal to 1 perpendicular: consequently, where the depth is 20 feet, the width of the slope at the surface will be 30 feet; the slope of the embankment to be 2 to 1, that is, for 19 feet perpendicular, the base is to be 38 feet. With these data, the cubical quantities, as computed by the valuable Tables of Sir John Macneill,\* are as follows:

Excavation . . .	81517 yards
Embankment . . .	57081 „

24436 surplus cutting.

We have an excess of 24436 cubic yards of excavation, which is a quantity far too great. In order, therefore, to make the quantity of cutting and embankment more nearly balance each other, it would be necessary to continue the embankment beyond the point A, which would lengthen the inclination, as shown by the dotted line drawn from the point B to *a*; this dotted line would now represent the proposed surface of the road. By such means we diminish the quantity of cutting, and, at the same time, increase that of the embankments; and also by lengthening the inclination, we reduce its steepness. The alteration of the proposed surface line must be so made, that the cubical quantities of excavation and embankment are nearly equal;

\* "Tables for calculating the cubic quantity of earthwork in the cuttings and embankments of canals, railways, and turnpike roads." By Sir John Macneill, Civil Engineer, F.R.A.S., &c.

leaving, however, a preponderance in favour of the latter of about 10 per cent. to supply the deficiency occasioned by the consolidation and shrinking of the earth; and if any portion of the excess be then remaining, it may be disposed of in flattening the slopes of the embankments, when no more convenient mode presents itself.

The quantities of earthwork on a given section depend upon the arrangement and disposition of the *gradients*, or proposed surface lines; and there is no practical consideration of more consequence to the engineer, in laying out a proposed line of surface upon a section, especially if it be of any great extent (as the present projected lines of railway), than the most judicious distribution of the cuttings and embankments, which should not only be nearly equal to each other in quantity, but the circumstances must be considered under which the various embankments have to be supplied, it not being alone sufficient that for every hollow on the section there should be a corresponding protuberance, but that such protuberances be advantageously situated for filling the hollows; for otherwise the work assumes a character of difficulty, in consequence of the great additional expense of removing the earth to a considerable distance; and if, in addition, the material has to be conveyed up an ascent, it will be more tedious in the execution.

Knowing the value of practical examples in elementary books, we shall here give the calculations of the above results in full, both by the common method; viz., *The Prismoidal Formula*, and Sir John Macneill's Tables, by which the saving of labour by the use of the Tables will be made apparent.



*Prismoidal Formula.*—The area of each end added to four times the middle area, and the sum multiplied by the length divided by 6, will give the solid content. If the measures used in the calculation are yards, the result will be the content in cubic yards; but if they are feet, the result must be divided by 27, to obtain the corresponding number of yards.

CALCULATION OF THE TRIANGULAR PORTION *a*.

		Height 0	
		2) 18	
		<hr/>	
Height . . .	18	9	mean height.
Slope . . .	1.5	1.5	slope.
<hr/>		<hr/>	
9.0		4.5	
18		9	
<hr/>		<hr/>	
27.0		13.5	
50		50.0	
		{ base (bottom of cutting, or top of embankment).	
77		63.5	
Height . . .	18	9	mean height.
616		571.5	
77		4	
Area of greater end } 1386		2286.0	
		1386.0	
		4 times middle area.	
		area of greater end.	
<hr/>		<hr/>	
		3672	
		561	
		length.	
		<hr/>	
		3672	
		22032	
		18360	
6) 2059992		<hr/>	
3) 343332		cub. content in feet.	
9) 114444		<hr/>	
		12716	
		cub. content in yards.	

COMPUTATION OF *b*.

Height 18. Area, as before, 1386.

20 height.	18	} heights.
1.5 slope.	20	
<hr/>	<hr/>	
10.0	2) 38	
20	<hr/>	
<hr/>	19	mean height.
30.0	1.5	slope.
50 base.	<hr/>	
<hr/>	9.5	
80	19	
20 height.	<hr/>	
<hr/>	28.5	•
1600 { area between	50	base.
<i>b</i> and <i>c</i> .		
	<hr/>	
	19	mean height.
	<hr/>	
	7065	
	785	
	<hr/>	
	1491.5	middle area.
	4	
	<hr/>	
	5966.0	4 times middle area.
	1386	area of lesser end.
	1600	area of greater end.
	<hr/>	
	8952	
	858	length.
	<hr/>	
	71616	
	44760	
	71616	
	<hr/>	
	6) 7680816	
	<hr/>	
	3) 1280136	cub. content in feet.
	<hr/>	
	9) 426712	
	<hr/>	
	47412	cub. content in yards.

COMPUTATION OF  $c$ ,

Area 1600, as before.

$\begin{array}{r} 20 \\ 0 \end{array}$	$\left. \begin{array}{l} \\ \end{array} \right\}$	heights.
2) 20		
$\begin{array}{r} 10 \\ 1.5 \end{array}$		mean height. slope.
$\begin{array}{r} 50 \\ 10 \end{array}$		
$\begin{array}{r} 15.0 \\ 50 \end{array}$		base.
$\begin{array}{r} 65 \\ 10 \end{array}$		mean height.
$\begin{array}{r} 650 \\ 4 \end{array}$		middle area.
$\begin{array}{r} 2600 \\ .1600 \end{array}$		4 times middle area. area of greater end.
$\begin{array}{r} 4200 \\ 825 \end{array}$		length.
$\begin{array}{r} 21000 \\ 8400 \\ 33600 \end{array}$		
6) 3465000		
3) 577500		cub. content in feet.
9) 192500		
$\begin{array}{r} 21389 \end{array}$		cub. content in yards.

 $a = \text{cub. content} \quad . \quad . \quad . \quad . \quad . \quad 12716$  $b = \text{cub. content} \quad . \quad . \quad . \quad . \quad . \quad 47412$  $c = \text{cub. content} \quad . \quad . \quad . \quad . \quad . \quad 21389$ Total cuttings  $. \quad . \quad . \quad . \quad . \quad . \quad 81517 \text{ cub. yards.}$

COMPUTATION OF EMBANKMENT *d.*

19 height.	0 }	
2 slope,	19 }	
—	—	
38	2) 19	
50 base.	—	
—	9.5	mean height.
88	2	slope.
19 height.	—	
—	19.0	
792	50	base.
88	—	
—	69	
1672 area.	9.5	mean height.
	34.5	
	621	
	—	
	655.5	middle area.
	4	
	—	
	2622.0	4 times middle area.
	1672	area of greater end.
	4294	
	820	length.
	—	
	85880	
	34352	
	—	
	6) 3521080	
	3) 586847	cont. in cub. feet.
	—	
	9) 195616	
	—	
	21735	cont. in cub. yards.

COMPUTATION OF *e*.

Height 8. Area, as before, 1672.

8 height.	19	} heights.
2 slope.	8	
—	—	
16	2) 27	
50 base.	—	
—	13·5	mean height.
66	2	slope.
8 height.	—	
	27·0	
528 area.	50	base.
	77	
	13·5	mean height.
	—	
	385	
	231	
	77	
	—	
	1039·5	middle area.
	4	
	—	
	4158·0	4 times middle area.
	1672	area of greater end.
	528	area of lesser end.
	—	
	6358	
	825	length.
	—	
	31790	
	12716	
	50864	
	—	
6) 5245350		
	—	
3) 874225		cub. content in feet.
9) 291408		
	32379	cub. content in yards.

COMPUTATION OF  $f$ .

Area, as before, 528.

$\begin{array}{r} 8 \\ 0 \end{array}$	} heights.
2) 8	
4	mean height.
2	slope.
8	
50	base.
58	
4	mean height.
232	middle area.
4	
928	4 times middle area.
528	area of greater end.
1456	
330	length.
43680	
4368	
6) 480480	
3) 80080	cub. content in feet.
9) 26693	
2966	cub. content in yards.

 $d$  = cub. content . . . . 21735 $e$  = cub. content . . . . 32379 $f$  = cub. content . . . . 2966

Total embankment . . . . 57080 cubic yards.

*The same quantities computed by Sir John Macneill's  
Tables.*

THE CUTTINGS.

COMPUTATION OF <i>a</i> .		COMPUTATION OF <i>b</i> .	
Tabular No. . . . .	= 22·67	Tabular No. . . . .	= 55·26
Length . . . . .	561	Length . . . . .	858
	<hr/>		<hr/>
	2267		44208
	13602		27630
	11335		44208

Cont. of *a* in cub. yds. = 12717·87    Cont. of *b* in cub. yds. = 47413·08

COMPUTATION OF <i>c</i> .	
Tabular No. . . . .	= 25·92
Length . . . . .	825
	<hr/>
	12960
	5184
	20736

Cont. of *c* in cub. yards . = 21384·00

THE EMBANKMENTS.

COMPUTATION OF <i>d</i> .		COMPUTATION OF <i>e</i> .	
Tabular No. . . . .	= 3519	Tabular No. . . . .	= 5000
Base . . . . .	50	Base . . . . .	50
	<hr/>		<hr/>
	17·5950		25·0000
Tabular No. . . . .	+ 8·914	Tabular No. . . . .	+ 14·247
	<hr/>		<hr/>
	26·509		39·247
Length . . . . .	820	Length . . . . .	825
	<hr/>		<hr/>
	530180		196235
	212072		78494
			313976
Cub. content . . . . .	= 21737·380	Cub. content . . . . .	= 32378·775

COMPUTATION OF  $f$ .

Tabular No. . . . .	=	1481
Base . . . . .		50
		<hr/>
		74050
Tabular No. . . . .	+	1580
		<hr/>
		8985
Length . . . . .		330
		<hr/>
		269550
		26955
Cub. content . . . . .		2965050

## RESULTS BY THE TABLES.

CUTTINGS.	EMBANKMENTS.
$a = 12717.9$	$d = 21737.4$
$b = 47413.1$	$e = 32378.8$
$c = 21384.0$	$f = 2965.0$
<hr/>	<hr/>
81515.0	57081.2

By comparing these results with those obtained by the former process, it will be seen that the cubical quantity of cuttings differs but two yards, and that of the embankments but one yard. The computation by the Tables may be abbreviated by using but one place of decimals, which would be sufficiently accurate for practical purposes. Our object is to show the calculations, by the Tables, in their greatest extent, which even then produce a great saving of labour, and, of course, a much greater probability of accuracy, in consequence of the fewer figures employed than the former process.

It will be seen that the calculation of the embankments by the Tables is a longer process than that of the cuttings, the latter being done by simply multiplying a



number taken from the Tables (answering to the height or depth at each end) by the length; whilst, for the embankments, the tabular number is first multiplied by the base (or width of roadway), and to the product is added a second tabular number taken out at the same time as the first. The first series of Sir John Macneill's Tables contain the numbers corresponding to a base of 50, and a slope of  $1\frac{1}{2}$  to 1 (which is the slope of the cuttings in our example). But for a slope of 2 to 1, reference must be had to the second series of the same Tables, which are applicable to every width of base, and from slopes varying from  $\frac{1}{2}$  to 1, to 3 to 1. We have adopted this example to show the calculations both by the *particular* and *general* Tables, as the first and second series of the valuable work referred to may be called.

The following is an extract from Sir John Macneill's preface to his Tables:—"All practical engineers are well aware, by experience, of the inconveniences which arise from the length of time necessary for calculating the cubic quantity of earthwork in the cuttings and embankments of canals, railways, and turnpike roads, especially when the section is of considerable extent, and the ground very uneven. As calculations of this kind are frequently, on a short notice, required to be completed within a limited period, the consequence is, that errors are almost sure to be made, as a multiplicity of figures is necessary, though the calculations in themselves are so very simple.

"To save time in making these calculations, and ensure accuracy in the results, were the principal objects I had in view in constructing the following Tables; how

far I have succeeded, must be left to the decision of practical men, for whose use they were intended, and who are best able to judge of their utility.

“An advantage may arise from the use of these Tables, which I had not at first contemplated. By the common but erroneous method of calculation, the cuttings may appear to be equal to the embankments; yet when the work is carried into effect, a large quantity of earth may be required to make up the embankments, or there may be too much earth in the cuttings for the embankments, according to the shape or figure of the section, as will be shown hereafter. Such a circumstance as this cannot take place if the following Tables be used to ascertain the cubic quantities; for, as they are calculated from the prismoidal formula, they will give the true cubic quantity in any cutting or embankment; and consequently, if the cuttings be laid down on the section to balance the embankments, they will be found in practice to do so, when the work comes to be executed.

—“Contractors very frequently find that they have more earth to move than they had previously calculated upon from the section, and are, therefore, often great losers. This, in most cases, arises from erroneous calculations; for the common practice is, either to add the two extreme heights together, and to take half the sum for a mean height; or to take half the sum of the areas at each end for a mean area. Both these methods are erroneous; one makes the quantity too much—the other too little.”

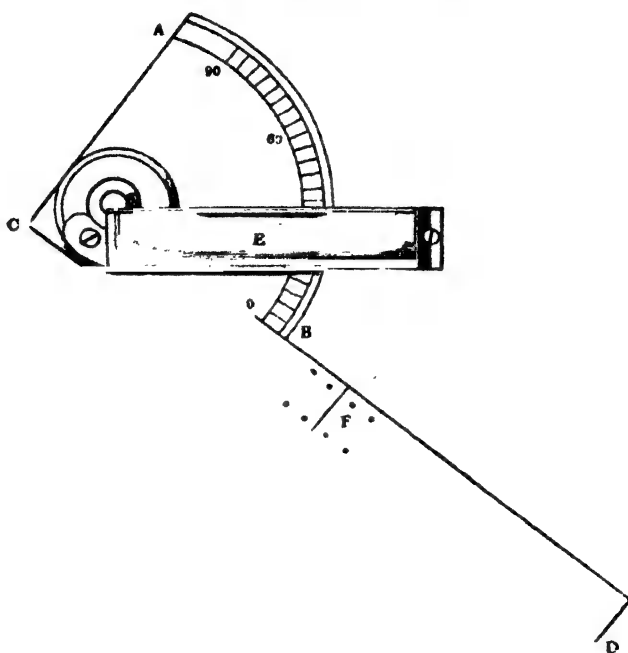
## SLOPES, ETC.

As connected with the subject of earth-work, we may insert in this place some particulars respecting the arrangement of slopes in cuttings and embankments. They are usually expressed in terms of the height or depth of cutting, as half to one, one to one, two to one, &c., signifying that for every foot perpendicular, the cutting shall batter half a foot, one foot, two feet, &c.

The slope adopted must depend upon the nature of the material worked upon. Solid rock may be left perpendicular, whilst loose friable material, or sand, will stand but at a very small angle with the horizon. The true criterion to judge of the proper slope to work to, is to observe, if convenient, what slope or angle the materials naturally assume when left to themselves. To determine this by measurement would be troublesome and tedious: but by the aid of a small instrument called a clinometer, the angle which any sloping surface makes with the horizon may be at once measured, and the ratio of the slope to the perpendicular, as one to one, &c., be readily deduced. As this very useful portable instrument is not generally known, we shall subjoin an engraving and description of it.

The following figure represents a clinometer, or, as it is called in some parts of the country, a *batter level*. It consists of a quadrant A B, of about two inches radius, attached to a flat bar C D, six inches long. The quadrant is graduated to degrees, from B towards A, and adjoining the divisions may be inserted, if required, the corresponding ratio of the slopes, one to one, &c.

An index bar, E, turns upon the centre of the quadrant, and carries a spirit-level by which the index may be set



truly horizontal by the hand ; and whatever angle is there denoted on the quadrant, will be that of the slope required. At F is a hinge joint, by which the bar C D may be folded up, and the instrument can then be deposited in a box of very small dimensions, and carried in the pocket without inconvenience.

To use this instrument, open the hinge-joint, and rest the edge of the bar C D on the face of the slope to be measured ; then gently move the index E round its centre until the attached spirit-bubble assumes a central position in its glass tube, and the angle, indicated by the index on the graduated arc, will at once measure the inclination. The ratio of the slope to the perpendicular is represented by the natural co-tangent of the angle thus measured ; but as the observer may not have

at hand a Table of natural co-tangents, &c., we have annexed a Table at once showing the slopes corresponding to the various angles of inclination. likely to be required.

It will appear evident, that the longer the bar C D is, the more accurate will the measure of the slope be; but there is no necessity for the instrument to be encumbered with a long bar, which would destroy its portability, because it can easily be attached, by tying, to the end of a long straight rod, which can be furnished by any neighbouring carpenter, and the real slope of an undulating inclined surface can then be accurately measured.

*Table of Slopes.*

Slope or Batter to 1 foot perpen- dicular.	Ratio of Slope to perpen- dicular.	Angle of Slope.		Slope or Batter to 1 foot perpen- dicular.	Ratio of Slope to perpen- dicular.	Angle of Slope.	
		With vertical.	With horizon.			With vertical.	With horizon.
ft. in.		° ' "	° ' "	ft. in.		° ' "	° ' "
0 1	$\frac{1}{48}$ to 1	1 12	88 48	1 0	1 to 1	45 0	45 0
0 2	$\frac{1}{24}$ " 1	2 23	87 37	1 3	$1\frac{1}{4}$ " 1	51 20	38 40
0 3	$\frac{1}{16}$ " 1	3 35	86 25	1 6	$1\frac{1}{2}$ " 1	56 19	33 41
0 4	$\frac{1}{12}$ " 1	4 46	85 14	1 9	$1\frac{3}{4}$ " 1	60 15	29 45
0 5	$\frac{1}{10}$ nearly 1	5 57	84 3	2 0	2 " 1	63 26	26 34
0 6	$\frac{1}{8}$ to 1	7 8	82 52	2 3	$2\frac{1}{4}$ " 1	66 2	23 58
0 7	$\frac{1}{6}$ nearly 1	8 18	81 42	2 6	$2\frac{1}{2}$ " 1	68 12	21 48
0 8	$\frac{1}{5}$ to 1	9 28	80 32	2 9	$2\frac{3}{4}$ " 1	70 1	19 59
0 9	$\frac{1}{4}$ nearly 1	11 46	78 14	3 0	3 " 1	71 34	18 26
0 10	$\frac{1}{3}$ to 1	14 2	75 58	3 6	$3\frac{1}{2}$ " 1	74 3	15 57
0 12	$\frac{1}{2}$ " 1	16 16	73 44	4 0	4 " 1	75 58	14 2
0 15	$\frac{2}{3}$ " 1	18 26	71 34	4 6	$4\frac{1}{2}$ " 1	77 28	12 32
0 20	$\frac{1}{2}$ " 1	20 34	69 26	5 0	5 " 1	78 41	11 19
0 25	$\frac{1}{2}$ " 1	22 37	67 23	5 6	$5\frac{1}{2}$ " 1	79 42	10 18
0 30	$\frac{1}{2}$ " 1	24 37	65 23	6 0	6 " 1	80 32	9 28
0 36	$\frac{1}{2}$ " 1	26 34	63 26	7 0	7 " 1	81 52	8 8
0 45	$\frac{1}{2}$ " 1	30 15	59 45	8 0	8 " 1	82 53	7 7
0 54	$\frac{1}{2}$ " 1	33 41	56 19	9 0	9 " 1	83 39	6 21
0 63	$\frac{1}{2}$ " 1	36 52	53 8	10 0	10 " 1	84 17	5 43
0 75	$\frac{1}{2}$ " 1	39 48	50 12	11 0	11 " 1	84 48	5 12
0 90	$\frac{1}{2}$ " 1	42 31	47 29	12 0	12 " 1	85 14	4 46

It is very important, in fixing upon the slopes for the sides of an excavation or embankment, to approximate very nearly to the inclination at which the ground would

naturally stand without slipping ; for if they be made greater than necessary, a large quantity of labour, and of the surface of the ground, will be uselessly devoted. The proper slope for each particular soil can only be determined by observation and experience. "An embankment that would stand perfectly firm, and bear the action of the weather, when formed of sand, gravel, or the débris of rocks, and other materials that do not retain water in their fissures, would not last one winter, if it chiefly consisted of clay. The same remark applies with equal force to cutting, where it is made through a stratum of clay." \* "A slope of 1 to 1, that is, a slope of  $45^\circ$ , is found sufficient for ordinary earth; for clay  $1\frac{1}{2}$  to 1, or a slope of  $33^\circ 41'$  with the horizon, may often be required, unless it can be mixed with open materials to prevent water collecting in the fissures produced by its shrinkage in dry water. In other cases, so steep a face may be left as  $\frac{3}{4}$  to 1, or even  $\frac{1}{2}$  to 1; and the slope that will be likely to stand may easily be judged of, by knowing the nature of the strata which will be cut through, and examining its state when exposed in the surrounding district."

At Boughton Hill, near Canterbury, there is a large cutting through London Clay, which, together with the embankment at the foot of the hill, formed of the same material, has been constantly giving way. The slopes of the embankment have been flattened from time to time, and now assume some appearance of consolidation; but the slopes of the cutting near the summit of the hill continue to slip down upon the roadway. From some cross-sections we were able to take a short time since, it

\* Tredgold on Railroads, first edition, page 117.

appears that the original slope of the cuttings was about 2 to 1, forming an angle with the horizon of  $26^{\circ} 34'$ ; but the natural slope assumed by the soft clay where it has slipped is about  $9^{\circ}$ , or a little more than  $6\frac{1}{4}$  to 1.

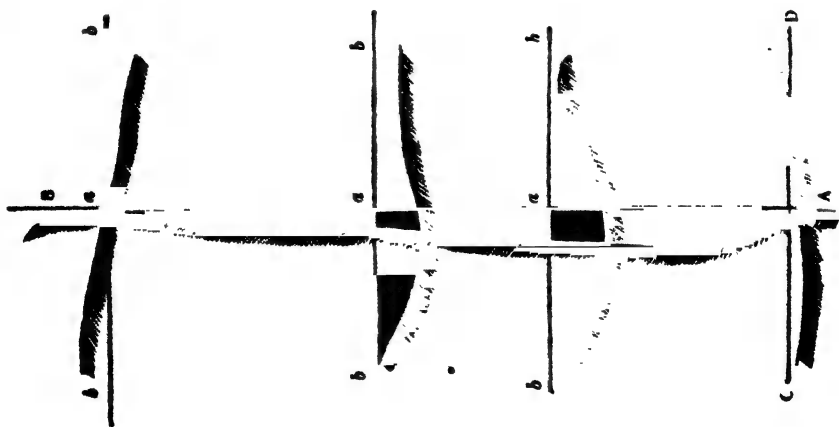
#### ON SELECTING A LINE OF COUNTRY FOR A ROAD OR RAILWAY.

The choice of a suitable line of country for the formation of a turnpike-road, a railroad, or a canal, preparatory to the levels being taken, requires both judgment and care; otherwise a fruitless expenditure of time in taking a number of trial-sections may be the result, if attended with no more serious and permanent inconvenience. A person undertaking such a work should previously devote a little time to obtain a knowledge of the country, its localities, its structure, and geological character: such knowledge will lead to the choice of several lines of direction, which appear to the eye as equally favourable; it then becomes necessary to make such preliminary surveys as will enable the engineer to adopt the one which, under all circumstances, is likely to prove the most advantageous.

At p. 113 (1st edition) of the late Mr. Tredgold's work upon Railroads, we find the following observations upon this subject: "In order to facilitate the choice of a line as it regards the surface of the country, the engineer may be reminded, that even in the disposal which nature has made of hills and valleys there is much system. Those things which to the first glance of the better-informed, and at all times to the ignorant, appear to be without order or arrangement, are the result of the uniform action of natural causes, and are, in reality,

capable of being traced and described with less difficulty than would be expected. Where a considerable tract of country is to be surveyed, the best index to its elevations and depressions is its streams and rivers; these indicate every change of inclination, and to the experienced eye, with considerable precision. It will also be observed, that each river has its system of valleys; and except in a few instances, where the draining is effected by the outburst of an open stratum, a district, whose bounding ridge is easily traced, is drained by its river and system of valleys.

“ Having formed a tolerable idea of the best direction for the road, the next step must be to make a more particular survey, with a view to fix nearly the precise line. We would recommend the principal engineer to have this done by rectangular lines, as infinitely superior to surveying by triangles, in giving him an exact knowledge of the surface of the country. Perhaps, with the assistance of a diagram, we shall be able to render the advantage of this method obvious.



“ Let A B be a portion of the intended line, and C D the breadth of the country to be included in the survey. At any suitable distances choose stations, *a, a, a*, their



distances apart depending on the changes of level, and let the principal line A B, and also the cross lines  $bb$ ,  $b b$ , &c., be accurately levelled, and then drawn, as shown in the figure, on the plan of the line of road. If the distance  $bb$  is required to be considerable, perhaps an additional line in the principal direction may be necessary. The etched lines show the form of the surface at the lines A B,  $bb$ ,  $bb$ , &c., on the plan; and the latter being sections at right angles to A B, there is no difficulty in seeing the extent of cutting, or of embankment, that may be avoided by varying the position of the principal line. In fact, a plan of this kind, to a person familiar with sections, is better than a model of the country."

The most advantageous direction for a line, either of roadway or railroad, intended to connect two places, is evidently that of a right line, both horizontally and vertically: if one extremity of the line is more elevated than another, the straight line connecting them will be an inclined plane, having one uniform rate of inclination; but if a uniform slope cannot be obtained in the direct line, it is necessary to deviate therefrom to obtain, as nearly as the circumstances of the country will admit, such an inclined plane, or at least to obtain continued progressive rises, avoiding as much as possible the introduction of useless ascents, that is, ascending where we must descend again, and *vice versâ*. When a line of road is encumbered with numerous and extensive useless ascents, the wasteful expenditure of power in the conveyance of goods is very great, as the number of feet actually ascended is increased many times more than is necessary, if each height, when once gained, were not lost again.

Sir Henry Parnell, in his valuable treatise on Roads, gives the following instances of this kind of road-making:—"As one instance, amongst others, of the serious injury which the public sustain by this system of roadmaking, the road between London and Barnet may be mentioned, on which the total number of perpendicular feet that a horse must now ascend is upwards of 1300, although Barnet is only 500 feet higher than London; and in going from Barnet to London, a horse must ascend 800 feet, although London is 500 feet lower than Barnet."

Another instance of this defect in road-engineering is observable in the line of the old road across the island of Anglesea, on which a horse was obliged to ascend and descend 1283 perpendicular feet more than was found necessary by Mr. Telford, when he laid out the present new line, as shown by the annexed Table:

	Height of summit above high water.	Total rise and fall.	Length.	
			Miles.	Yards.
Old road .	339	3540	24	428
New road .	193	2257	21	1596
Difference .	146	1283	2	592

In choosing the best direction for a line of roadway, the rate of inclination which can be obtained, with a moderate outlay in cuttings and embankments, is a consideration of greater importance than the mere maintaining of a direct line. For though the measured length of a circuitous route may be considerably greater than the length of a direct line, yet if the inclinations in

the former case are much more favourable than those in the latter, it must be evident that more may be gained in speed, with the same expenditure of power, than is lost by the increase of distance. Thus, if two roads rise, one at the rate of 1 in 15, and the other at the rate of 1 in 35, the same expenditure of power will move a weight through 15 feet of the one and 35 feet of the other, at the same rate.

Upon the subject of the maintenance of turnpike roads, we shall annex an abstract of the General Rules for Constructing and Repairing Roads, laid down by the late Mr. Telford, and which is so fully treated upon in the important work of Sir H. Parnell on Roads.

#### SHAPE, OR TRANSVERSE SECTION.

The roadway should be 30 feet broad; the centre should be 6 inches higher than the level of the sides, where the junction of the surface, with the sloping edge of the footpaths, or other defining bounds of the roadway, form the side channels; at 4 feet from the centre (on each side) the surface should be half an inch lower; at 9 feet, it should be 2 inches lower; and at 15 feet, its extreme edge, it should be 6 inches lower; this will give the form of a flat ellipse, which is well adapted for carrying off the water to the side channels, without making the cross section of the road too round, and allow the sun and wind to have a greater effect in evaporation, and keeping the road dry. In giving the surface one uniform curvature from side to side, the surveyor should use such a level as is described at page 96.

The footpaths should be 6 feet broad, and have an inclined surface of 1 inch in a yard towards the road; its surface should not be lower than the level of the centre of the road, and the edge should be sloped down (and covered with green sod) to meet the roadway, and form the side channel to carry off the water from the surface.

#### DRAINAGE.

All open main drains should be cut on the field side of the road fences, and should lead to the natural water-courses of the country; in general, they should be 3 feet deep below the bed of the road, 1 foot wide at bottom, and from 3 to 4 feet wide at top. Stone drains and culverts should also be made under the road, and continued to the open side drains, or ditches; side channels (before named) must be made on the road side, with openings of masonry into the cross drains, to prevent any water lying on the road, it being necessary, in order to preserve the surface of a road perfect, that it be kept completely dry. All land springs ought to be carried from the site of the road by under-draining.

#### FENCES.

“All road fences should be kept as low as possible, never being allowed to exceed 5 feet in height, in order that they may not intercept the sun and wind, and diminish their effects in producing evaporation;” and for the same reason no tree should be allowed to grow by the side of a road; for by keeping the roads wet, they occasion the rapid wear of the materials of which they are formed.

## ROAD MATERIALS.

The hardest description of stone should always be preferred, such as basalt, granite, quartz, &c. "The whinstones found in different parts of the United Kingdom, Guernsey granite, Mountsorrel and Hartshill stone of Leicestershire, and the pebbles of Shropshire, Staffordshire, and Warwickshire, are among the best of the stones now commonly in use. The schistus rocks, being of a slaty and argillaceous structure, will make smooth roads, but they are rapidly destroyed when wet by the pressure of the wheels, and occasion great expense in scraping, and the constantly laying on of new coatings. Limestone is defective in the same respect. Sandstone is generally much too weak for the surface of a road; it will never make a hard one. The hardest flints are nearly as good as the best limestone; but the softer kinds are quickly crushed by the wheels of carriages, and make heavy and dirty roads. Gravel, when it consists of pebbles of the harder sorts of stones, will make a good road; but when it consists of limestone, sandstone, flint, and other weak stones, it will not; for it wears so rapidly, that the crust of a road made with it always consists of a large portion of the earthy matter to which it is reduced, and prevents the gravel from becoming consolidated, and the road from attaining that perfect hardness it ought to possess." \* When the materials are stone, they should be broken to a size of a cubical form not exceeding  $2\frac{1}{2}$  inches in their largest dimensions,

\* Abridged from Sir H. Parnell on Roads, page 271.

and should be capable of passing through a ring of that diameter. When it consists of gravel, the pebbles which are from .1 to  $1\frac{1}{2}$  inch in size only should be used for the middle part of the road; all larger pebbles should be broken; the smaller stones may be used for the sides of the roads and the footpaths.

#### THE FOUNDATION AND DISPOSITION OF MATERIALS.

Before the foundation is laid, the surface on which it is to rest must be prepared, by making it level from side to side, and, if necessary, raising it so that the finished surface of the road may not be below the level of the adjoining fields. If the subsoil be wet and elastic, it must be rendered non-elastic by whatever means is best adapted to overcome the cause, as drainage, &c. The foundation should consist of a rough close-set pavement, of any kind of stones that can be most readily procured; those set in the middle of the road should be 7 inches in depth; at 9 feet from the centre, 5 inches; at 12 feet from the centre, 4 inches; and at 15 feet, 3 inches. They should be set with their broadest faces downwards, and lengthwise across the road; and no stone should be more than 5 inches broad on its face. "The irregularities of the upper part of the pavement should be broken off with the hammer, and all the interstices should be filled with stone chips, firmly wedged, or packed by hand with a light hammer; so that, when the pavement is finished, there may be a convexity of 4 inches in the breadth of 15 feet from the centre.

"The middle 18 feet of pavement should be coated

with hard broken stones, of the form and size described under the head 'Road Materials,' to the depth of 6 inches. Four of these 6 inches to be first put on, and worked in by carriages and horses; care being taken to rake in the ruts until the surface becomes firm and consolidated, after which the remaining 2 inches are to be put on.

“The paved spaces on each side of the 18 middle feet should be coated with broken stones, or well-cleansed strong gravel, up to the footpath, or other boundary of the road, so as to make the whole convexity of the road 6 inches from the centre to the sides of it; and the whole of the materials should be covered with a binding of an inch and a half in depth of good gravel, free from clay or earth.”

The footpaths should be made with a coating of strong gravel, or small broken stones, at least 6 inches deep. The annexed engraving exhibits a section of a road constructed according to the above rules.

#### REPAIRING ROADS.

Towards the latter end of the autumn of each year, a road should be put into a complete state of repair, to preserve it from being broken up during the following winter, between which time and the preceding spring, all repairs, by laying on of new materials, should be done. If thin coatings be laid on at a time, and when the ground is

wet, they will soon be worked into the surface without being crushed into powder.

All ruts and hollows should be filled up as soon as they appear. The side channels and drains should be continually kept clean, and free from obstruction; and all damage they may have sustained be made good as soon as discovered.

“A road should be scraped from time to time, so as never to have half an inch of mud upon it; the mud should not be scraped into, or allowed to remain in, the side channels, so as to stop the running of water in them.

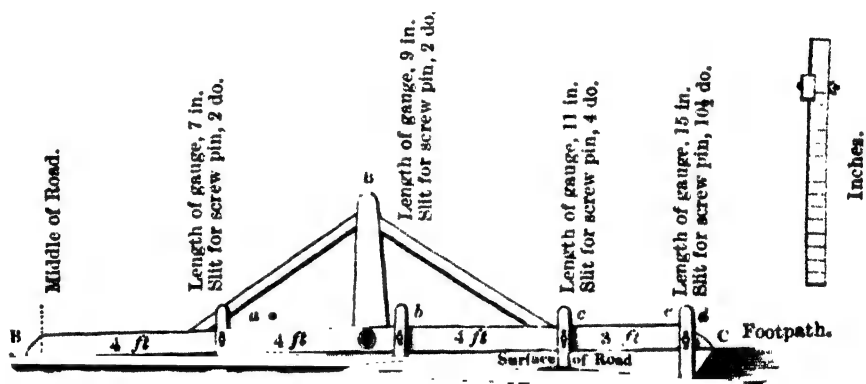
“The hedges should be kept constantly clipped and cut as low as possible, without rendering them unfit for confining cattle; and all projecting branches of the trees in the fences should be lopped.”

In the minutes of evidence given before a Select Committee of the House of Commons on the subject of Steam Carriages, we find the following paragraph as part of the evidence given by Sir John Macneill:

“Well-made roads, formed of clean, hard, broken stone, placed on a solid foundation, are very little affected by changes of atmosphere; weak roads, or those that are imperfectly formed of gravel, flint, or round pebbles, without a bottoming or foundation of stone pavement or concrete, are, on the contrary, much affected by changes of the weather. In the formation of such roads, and before they become bound or firm, a considerable portion of the subsoil mixes with the stone or gravel, in consequence of the necessity of putting the gravel on in thin layers: this mixture of earth or clay, in dry warm seasons, expands by the heat, and makes the road loose and open; the consequence is, that the



stones are thrown out, and many of them are crushed and ground into dust, producing considerable wear and diminution of the materials: in wet weather, also, the clay or earth, mixed with the stones, absorbs moisture, becomes soft, and allows the stones to move and rub against each other when acted upon by the feet of horses or wheels of carriages. This attrition of the stones against each other wears them out surprisingly fast, and produces large quantities of mud, which tend to keep the road damp, and by that means increase the injury."



The above engraving represents the level employed by road-surveyors in laying out new works. On the horizontal bar B C are placed four sliding gauges, *a*, *b*, *c*, *d*, which move in dovetailed grooves cut in the horizontal bar, and when adjusted to their proper depth below the bottom edge of the level, can be firmly fixed in their position by a thumb-screw. A section of this portion of the instrument, taken through the line at *e*, is given on the right, drawn to a larger scale; the remaining parts of the instrument require no explanation.

For laying out slopes, the clinometer, described at page 82, is the best instrument that can be used.

TABLE I.—*Showing the reduction upon each chain necessary to reduce hypotenusal to horizontal measure.*

Angle of ascent or descent.	Reduction in links.	Angle of ascent or descent.	Reduction in links.	Angle of ascent or descent.	Reduction in links.
0		0		0	
4 0	$\frac{1}{4}$	23 48	$8\frac{1}{2}$	33 55	17
5 44	$\frac{1}{2}$	24 30	9	34 25	$17\frac{1}{2}$
7 2	$\frac{3}{4}$	25 11	$9\frac{1}{2}$	34 55	18
8 7	1	25 51	10	35 25	$18\frac{1}{2}$
11 28	2	26 30	$10\frac{1}{4}$	35 54	19
12 50	$2\frac{1}{2}$	27 8	11	36 24	$19\frac{1}{2}$
14 5	3	27 45	$11\frac{1}{4}$	36 53	20
15 13	$3\frac{1}{2}$	28 22	12	37 21	$20\frac{1}{2}$
16 15	4	28 58	$12\frac{1}{2}$	37 49	21
17 15	$4\frac{1}{2}$	29 33	13	38 17	$21\frac{1}{2}$
18 12	5	30 8	$13\frac{1}{2}$	38 45	22
19 6	$5\frac{1}{4}$	30 41	14	39 12	$22\frac{1}{2}$
19 57	6	31 15	$14\frac{1}{2}$	39 39	23
20 47	$6\frac{1}{2}$	31 48	15	40 6	$23\frac{1}{2}$
21 34	7	32 20	$15\frac{1}{2}$	40 33	24
22 20	$7\frac{1}{2}$	32 52	16	40 58	$24\frac{1}{2}$
23 5	8	33 24	$16\frac{1}{4}$	41 25	25

TABLE II.—*Gradients or Inclined Planes.*

Ascent or descent.		Rate of inclination.	Angle of inclination.	Ascent or descent.		Rate of inclination.	Angle of inclination.
In 1 mile.	In 1 chain.			In 1 mile.	In 1 chain.		
feet.	ft. dec.	1 in	° ' "	feet.	ft. dec.	1 in	° ' "
1	0·013	5280·0	0 0 39	51	0·638	103·5	0 33 12
2	0·025	2640·0	0 1 18	52	0·650	101·5	0 33 51
3	0·038	1760·0	0 1 57	53	0·663	99·6	0 34 30
4	0·050	1320·0	0 2 36	54	0·675	97·8	0 35 10
5	0·063	1056·0	0 3 16	55	0·688	96·0	0 35 49
6	0·075	880·0	0 3 55	56	0·700	94·3	0 36 28
7	0·088	754·3	0 4 34	57	0·713	92·6	0 37 7
8	0·100	660·0	0 5 13	58	0·725	91·0	0 37 46
9	0·113	586·7	0 5 52	59	0·738	89·5	0 38 25
10	0·125	528·0	0 6 31	60	0·750	88·0	0 39 4
11	0·138	480·0	0 7 10	61	0·763	86·6	0 39 43
12	0·150	440·0	0 7 49	62	0·775	85·2	0 40 22
13	0·163	406·1	0 8 28	63	0·788	83·8	0 41 1
14	0·175	377·1	0 9 7	64	0·800	82·5	0 41 40
15	0·188	352·0	0 9 16	65	0·813	81·2	0 42 19
16	0·200	330·0	0 10 25	66	0·825	80·0	0 42 58
17	0·213	310·6	0 11 4	67	0·838	78·8	0 43 37
18	0·225	293·3	0 11 43	68	0·850	77·6	0 44 16
19	0·238	277·9	0 12 22	69	0·863	76·5	0 44 55
20	0·250	264·0	0 13 1	70	0·875	75·4	0 45 34
21	0·263	251·4	0 13 40	75	0·938	70·4	0 48 50
22	0·275	240·0	0 14 20	80	1·000	66·0	0 52 5
23	0·288	229·6	0 14 59	85	1·063	62·1	0 55 20
24	0·300	220·0	0 15 38	90	1·126	58·7	0 58 36
25	0·313	211·2	0 16 17	95	1·188	55·3	1 1 51
26		203·1	0 16 56	100	1·250	52·8	1 5 6
27	0·338	195·6	0 17 35	110	1·375	48·0	1 11 37
28	0·350	188·6	0 18 14	120	1·500	44·0	1 18 7
29	0·363	182·1	0 18 53	130	1·625	40·6	1 24 38
30	0·375	176·0	0 19 32	140	1·750	37·7	1 31 8
31	0·388	170·3	0 20 11	150	1·875	35·2	1 37 38
32	0·400	165·0	0 20 50	160	2·000	33·0	1 44 8
33	0·413	160·0	0 21 29	170	2·125	31·1	1 50 39
34	0·425	155·3	0 22 8	180	2·250	29·3	1 57 9
35	0·438	150·9	0 22 47	190	2·375	27·8	2 3 39
36	0·450	146·7	0 23 26	200	2·500	26·4	2 10 9
37	0·463	142·7	0 24 5	220	2·750	24·0	2 23 9
38	0·475	138·9	0 24 44	240	3·000	22·0	2 36 9
39	0·488	135·4	0 25 23	260	3·250	20·3	2 49 9
40	0·500	132·0	0 26 3	280	3·500		3 2 8
41	0·513	128·8	0 26 42	300	3·750	17·6	3 15 7
42	0·525	125·7	0 27 21	320	4·000	16·5	3 28 6
43	0·538	122·8	0 28 0	340	4·250	15·3	3 41 4
44	0·550	120·0	0 28 39	360	4·500	14·7	3 54 2
45		117·3	0 29 18	380	4·750	13·9	4 6 59
46	0·575	114·8	0 29 57	400	5·000	13·2	4 19 56
47	0·588	112·3	0 30 36	425	5·313	12·4	4 36 7
48	0·600	110·0	0 31 15	450	5·625	11·7	4 52 17
49	0·613	107·8	0 31 54	475	5·938	11·1	5 8 26
50	0·625	105·6	0 32 33	500	6·250	10·6	5 24 35

TABLE III.—To convert Links into Feet.

Links	CHAINS.									
	0	1	2	3	4	5	6	7	8	9
	feet.	feet.	feet.	feet.	feet.	feet.	feet.	feet.	feet.	feet.
0	0·0	66·0	132·0	198·0	264·0	330·0	396·0	462·0	528·0	594·0
1	0·7	66·7	132·7	198·7	264·7	330·7	396·7	462·7	528·7	594·7
2	1·3	67·3	133·3	199·3	265·3	331·3	397·3	463·3	529·3	595·3
3	2·0	68·0	134·0	200·0	266·0	332·0	398·0	464·0	530·0	596·0
4	2·6	68·6	134·6	200·6	266·6	332·6	398·6	464·6	530·6	596·6
5	3·3	69·3	135·3	201·3	267·3	333·3	399·3	465·3	531·3	597·3
6	4·0	70·0	136·0	202·0	268·0	334·0	400·0	466·0	532·0	598·0
7	4·6	70·6	136·6	202·6	268·6	334·6	400·6	466·6	532·6	598·6
8	5·3	71·3	137·3	203·3	269·3	335·3	401·3	467·3	533·3	599·3
9	5·9	71·9	137·9	203·9	269·9	335·9	401·9	467·9	533·9	599·9
10	6·6	72·6	138·6	204·6	270·6	336·6	402·6			600·6
11	7·3	73·3	139·3	205·3	271·3	337·3	403·3			601·3
12	7·9	73·9	139·9	205·9	271·9	337·9	403·9			601·9
13	8·6	74·6	140·6	206·6	272·6	338·6	404·6			602·6
14	9·2	75·2	141·2	207·2	273·2	339·2	405·2			603·2
15	9·9	75·9	141·9	207·9	273·9	339·9	405·9	471·9	537·9	603·9
16	10·6	76·6	142·6	208·6	274·6	340·6	406·6	472·6	538·6	604·6
17	11·2	77·2	143·2	209·2	275·2	341·2	407·2			605·2
18	11·9	77·9	143·9	209·9	275·9	341·9	407·9			605·9
19	12·5	78·5	144·5	210·5	276·5	342·5	408·5	474·5	540·5	606·5
20	13·2	79·2	145·2	211·2	277·2	343·2	409·2	475·2	541·2	
21	13·9	79·9	145·9	211·9	277·9	343·9	409·9	475·9	541·9	
22	14·5	80·5	146·5	212·5	278·5	344·5	410·5	476·5	542·5	608·5
23	15·2	81·2	147·2	213·2	279·2	345·2	411·2	477·2	543·2	609·2
24	15·8	81·8	147·8	213·8	279·8	345·8	411·8	477·8	543·8	
25	16·5	82·5	148·5	214·5	280·5	346·5	412·5	478·5	544·5	610·5
26	17·2	83·2	149·2	215·2	281·2	347·2	413·2	479·2	545·2	611·2
27	17·8	83·8	149·8	215·8	281·8	347·8	413·8	479·8	545·8	611·8
28	18·5	84·5	150·5	216·5	282·5	348·5	414·5	480·5	546·5	612·5
29	19·1	85·1	151·1	217·1	283·1	349·1	415·1	481·1	547·1	613·1
30	19·8	85·8	151·8	217·8	283·8	349·8	415·8	481·8	547·8	613·8
31	20·5	86·5	152·5	218·5	284·5	350·5	416·5	482·5	548·5	614·5
32	21·1	87·1	153·1	219·1	285·1	351·1	417·1	483·1	549·1	615·1
33	21·8	87·8	153·8	219·8	285·8	351·8	417·8	483·8	549·8	615·8
34	22·4	88·4	154·4	220·4	286·4	352·4	418·4	484·4	550·4	616·4
35	23·1	89·1	155·1	221·1	287·1	353·1	419·1	485·1	551·1	617·1
36	23·8	89·8	155·8	221·8	287·8	353·8	419·8	485·8	551·8	617·8
37	24·4	90·4	156·4	222·4	288·4	354·4	420·4	486·4	552·4	618·4
38	25·1	91·1	157·1	223·1	289·1	355·1	421·1	487·1	553·1	619·1
39	25·7	91·7	157·7	223·7	289·7	355·7	421·7	487·7	553·7	619·7
40	26·4	92·4	158·4	224·4	290·4	356·4	422·4	488·4	554·4	620·4
41	27·1	93·1	159·1	225·1	291·1	357·1	423·1	489·1	555·1	621·1
42	27·7	93·7	159·7	225·7	291·7	357·7	423·7	489·7	555·7	621·7
43	28·4	94·4	160·4	226·4	292·4	358·4	424·4	490·4	556·4	622·4
44	29·0	95·0	161·0	227·0	293·0	359·0	425·0	491·0	557·0	623·0
45	29·7	95·7	161·7	227·7	293·7	359·7	425·7	491·7	557·7	623·7
46	30·4	96·4	162·4	228·4	294·4	360·4	426·4	492·4	558·4	624·4
47	31·0	97·0	163·0	229·0	295·0	361·0	427·0	493·0	559·0	625·0
48	31·7	97·7	163·7	229·7	295·7	361·7	427·7	493·7	559·7	625·7
49	32·3	98·3	164·3	230·3	296·3	362·3	428·3	494·3	560·3	626·3

TABLE III.—*To convert Links into Feet (continued).*

CHAINS.										
[Links]	0		3		4		8		9	
	feet.	feet.	feet.	feet.	feet.	feet.	feet.	feet.	feet.	feet.
50	33·0	99·0	165·0	231·0	297·0	363·0	429·0	495·0	561·0	627·0
51	33·7	99·7	165·7	231·7	297·7	363·7	429·7	495·7	561·7	627·7
52	34·3	100·3	166·3	232·3	298·3	364·3	430·3	496·3	562·3	628·3
53	35·0	101·0	167·0	233·0	299·0	365·0	431·0	497·0	563·0	629·0
54	35·6	101·6	167·6	233·6	299·6	365·6	431·6	497·6	563·6	629·6
55	36·3	102·3	168·3	234·3	300·3	366·3	432·3	498·3	564·3	630·3
56	37·0	103·0	169·0	235·0	301·0	367·0	433·0	499·0	565·0	631·0
57	37·6	103·6	169·6	235·6	301·6	367·6	433·6	499·6	565·6	631·6
58	38·3	104·3	170·3	236·3	302·3	368·3	434·3	500·3	566·3	632·3
59	38·9	104·9	170·9	236·9	302·9	368·9	434·9	500·9	566·9	632·9
60	39·6	105·6	171·6	237·6	303·6	369·6	435·6	501·6	567·6	633·6
61	40·3	106·3	172·3	238·3	304·3	370·3	436·3	502·3	568·3	634·3
62	40·9	106·9	172·9	238·9	304·9	370·9	436·9	502·9	568·9	634·9
63	41·6	107·6	173·6	239·6	305·6	371·6	437·6	503·6	569·6	635·6
64	42·2	108·2	174·2	240·2	306·2	372·2	438·2	504·2	570·2	636·2
65	42·9	108·9	174·9	240·9	306·9	372·9	438·9	504·9	570·9	636·9
66	43·6	109·6	175·6	241·6	307·6	373·6	439·6	505·6	571·6	637·6
67	44·2	110·2	176·2	242·2	308·2	374·2	440·2	506·2	572·2	638·2
68	44·9	110·9	176·9	242·9	308·9	374·9	440·9	506·9	572·9	638·9
69	45·5	111·5	17	243·5	309·5	375·5	441·5	507·5	573·5	639·5
70	46·2	112·2	178·2	244·2	310·2	376·2	442·2	508·2	574·2	640·2
71	46·9	112·9	178·9	244·9	310·9	376·9	442·9	508·9	574·9	640·9
72	47·5	113·5	179·5	245·5	311·5	377·5	443·5	509·5	575·5	641·5
73	48·2	114·2	180·2	246·2	312·2	378·2	444·2	510·2	576·2	642·2
74	48·8	114·8	180·8	246·8	312·8	378·8	444·8	510·8	576·8	642·8
75	49·5	115·5	181·5	247·5	313·5	379·5	445·5	511·5	577·5	643·5
76	50·2	116·2	182·2	248·2	314·2	380·2	446·2	512·2	578·2	644·2
77	50·8	116·8	182·8	248·8	314·8	380·8	446·8	512·8	578·8	644·8
78	51·5	117·5	183·5	249·5	315·5	381·5	447·5	513·5	579·5	645·5
79	52·1	118·1	184·1	250·1	316·1	382·1	448·1	514·1	580·1	646·1
80	52·8	118·8	184·8	250·8	316·8	382·8	448·8	514·8	580·8	646·8
81	53·5	119·5	185·5	251·5	317·5	383·5	449·5	515·5	581·5	647·5
82	54·1	120·1	186·1	252·1	318·1	384·1	450·1	516·1	582·1	648·1
83	54·8	120·8	186·8	252·8	318·8	384·8	450·8	516·8	582·8	648·8
84	55·4	121·4	187·4	253·4	319·4	385·4	451·4	517·4	583·4	649·4
85	56·1	122·1	188·1	254·1	320·1	386·1	452·1	518·1	584·1	650·1
86	56·8	122·8	188·8	254·8	320·8	386·8	452·8	518·8	584·8	650·8
87	57·4	123·4	189·4	255·4	321·4	387·4	453·4	519·4	585·4	651·4
88	58·1	124·1	190·1	256·1	322·1	388·1	454·1	520·1	586·1	652·1
89	58·7	124·7	190·7	256·7	322·7	388·7	454·7	520·7	586·7	652·7
90	59·4	125·4	191·4	257·4	323·4	389·4	455·4	521·4	587·4	
91	60·1	126·1	192·1	258·1	324·1	390·1	456·1	522·1	588·1	654·1
92	60·7	126·7	192·7	258·7	324·7	390·7	456·7	522·7	588·7	654·7
93	61·4	127·4	193·4	259·4	325·4	391·4	457·4	523·4		655·4
94	62·0	128·0	194·0	260·0	326·0	392·0	458·0	524·0	590·0	656·0
95	62·7	128·7	194·7	260·7	326·7	392·7	458·7	524·7	590·7	656·7
96	63·4	129·4	195·4	261·4	327·4	393·4	459·4	525·4	591·4	657·4
97	64·0	130·0	196·0	262·0	328·0	394·0	460·0	526·0	592·0	658·0
98	64·7	130·7	196·7	262·7	328·7	394·7	460·7	526·7	592·7	
99	65·3	131·3	197·3	263·3	329·3	395·3	461·3	527·3		

TABLE IV.—*To convert Feet into Links.*

Feet.	FEET.									
	100	200	300	400	500	600	700	800	900	
	links.	links.	links.	links.	links.	links.	links.	links.	links.	links.
0	0·0	151·5	303·0	454·5	606·1	757·6	909·1	1060·6	1212·1	1363·6
1	1·5	153·0	304·5	456·1	607·6	759·1	910·6	1062·1	1213·6	1365·2
2	3·0	154·5	306·1	457·6	609·1	760·6	912·1	1063·6	1215·2	1366·7
3	4·5	156·1	307·6	459·1	610·6	762·1	913·6	1065·2	1216·7	1368·2
4	6·1	157·6	309·1	460·6	612·1	763·6	915·2	1066·7	1218·2	1369·7
5	7·6	159·1	310·6	462·1	613·6	765·2	916·7	1068·2	1219·7	1371·2
6	9·1	160·6	312·1	463·6	615·2	766·7	918·2	1069·7	1221·2	1372·7
7	10·6	162·1	313·6	465·2	616·7	768·2	919·7	1071·2	1222·7	1374·2
8		163·6	315·2	466·7	618·2	769·7	921·2	1072·7	1224·2	1375·8
9		165·2	316·7	468·2	619·7	771·2	922·7	1074·2	1225·8	1377·3
10	15·2	166·7	318·2	469·7	621·2	772·7	924·2	1075·8	1227·3	1378·8
11	16·7	168·2	319·7	471·2	622·7	774·2	925·8	1077·3	1228·8	1380·3
12	18·2	169·7	321·2	472·7	624·2	775·8	927·3	1078·8	1230·3	1381·8
13	19·7	171·2	322·7	474·2	625·8	777·3	928·8	1080·3	1231·8	1383·3
14	21·2	172·7	324·2	475·8	627·3	778·8	930·3	1081·8	1233·3	1384·8
15	22·7	174·2	325·8	477·3	628·8	780·3	931·8	1083·3	1234·8	1386·4
16	24·2	175·8	327·3	478·8	630·3	781·8	933·3	1084·8	1236·4	1387·9
17	25·8	177·3	328·8	480·3	631·8	783·3	934·8	1086·4	1237·9	1389·4
18	27·3	178·8	330·3	481·8	633·3	784·8	936·4	1087·9	1239·4	1390·9
	28·8	180·3	331·8	483·3	634·8	786·4	937·9	1089·4	1240·9	1392·4
20	30·3	181·8	333·3	484·8	636·4	787·9	939·4	1090·9	1242·4	1393·9
21	31·8	183·3	334·8	486·4	637·9	789·4	940·9	1092·4	1243·9	1395·5
22	33·3	184·8	336·4	487·8	639·4	790·9	942·4	1093·9	1245·5	1397·0
23	34·8	186·4	337·9	489·4	640·9	792·4	943·9	1095·5	1247·0	1398·5
24	36·4	187·9	339·4	490·9	642·4	793·9	945·5	1097·0	1248·5	1400·0
25	37·9	189·4	340·9	492·4	643·9	795·5	947·0	1098·5	1250·0	1401·5
26	39·4	191·9	342·4	493·9	645·5	797·0	948·5	1100·0	1251·5	1403·0
27	40·9	192·4	343·9	495·5	647·0	798·5	950·0	1101·5	1253·0	1404·5
28	42·4	193·9	345·5	497·0	648·5	800·0	951·5	1103·0	1254·5	1406·1
29	43·9	195·5	347·0	498·5	650·0	801·5	953·0	1104·5	1256·1	1407·6
30		197·0	348·5	500·0	651·5	803·0	954·5	1106·1	1257·6	1409·1
31	47·0	198·5	350·0	501·5	653·0	804·5	956·1	1107·6	1259·1	1410·6
32	48·5	200·0	351·5	503·0	654·5	806·1	957·6	1109·1	1260·6	1412·1
33	50·0	201·5	353·0	504·5	656·1	807·6	959·1	1110·6	1262·1	1413·6
34	51·5	203·0	354·5	506·1	657·6	809·1	960·6	1112·1	1263·6	1415·2
35	53·0	204·5	356·1	507·6	659·1	810·6	962·1	1113·6	1265·2	1416·7
36	54·5	206·1	357·6	509·1	660·6	812·1	963·6	1115·2	1266·7	1418·2
37	56·1	207·6	359·1	510·6	662·1	813·6	965·2	1116·7	1268·2	1419·7
38	57·6	209·1	360·6	512·1	663·6	815·2	966·7	1118·2	1269·7	1421·2
39	59·1	210·6	362·1	513·6	665·2	816·7	968·2	1119·7	1271·2	1422·7
40	60·6	212·1	363·6	515·2	666·7	818·2	969·7	1121·2	1272·	1424·2
41	62·1	213·6	365·2	516·7	668·2	819·7	971·2	1122·7	1274·2	1425·8
42	63·6	215·2	366·7	518·2	669·7	821·2	972·7	1124·2	1275·8	1427·3
43	65·2	216·7	368·2	519·7	671·2	822·7	974·2	1125·8	1277·3	1428·8
44	66·7	218·2	369·7	521·2	672·7	824·2	975·8	1127·3	1278·8	1430·3
45	68·2	219·7	371·2	522·7	674·2	825·8	977·3	1128·8	1280·3	1431·8
46	69·7	221·2	372·7	524·2	675·8	827·3	978·8	1130·3	1281·8	1433·3
47	71·2	222·7	374·2	525·8	677·3	828·8	980·3	1131·8	1283·3	1434·8
48	72·7	224·2	375·8	527·3	678·8	830·3	981·8	1133·3	1284·8	1436·4
49	74·2	225·8	377·3	528·8	680·3	831·8	983·3	1134·8	1286·4	1437·9

TABLE IV.—*To convert Feet into Links (continued).*

Feet.	FEET.									
	0	100	200	300	400	500	600	700	*800	900
	links.	links.	links.	links.	links.	links.	links.	links.	links.	
50	75.8	227.3	378.8	530.3	681.8	833.3	984.8	1136.4	1287.9	
51	77.3	228.8	380.3	531.8	683.3	834.8	986.4	1137.9	1289.4	
52	78.8	230.3	381.8		684.8	836.4	987.9	1139.4	1290.9	
53	80.3	231.8	383.3	534.8	686.4	837.9	989.4	1140.9	1292.4	
54	81.8	233.3	384.8	536.4	687.9	839.4	990.9	1142.4	1293.9	
55	83.3	234.8	386.4	537.9	689.4	840.9	992.4	1143.9	1295.5	
56	84.8	236.4	387.9	539.4	690.9	842.4	993.9	1145.5	1297.0	
57	86.4	237.9	389.4	540.9	692.4	843.9	995.5	1147.0	1298.5	
58	87.9	239.4	390.9	542.4	693.9	845.5	997.0	1148.5	1300.0	
59	89.4	240.9	392.4	543.9	695.5	847.0	998.5	1150.0	1301.5	
60	90.9	242.4	393.9	545.5	697.0	848	1000.0	1151.5	1303.0	
61	92.4	243.9	395.5	547.0	698.5	850.0	1001.5	1153.0	1304.5	
62	93.9	245.5	397.0	548.5	700.0	851.5	1003.0	1154.5	1306.1	
63	95.5	247.0	398.5	550.0	701.5	853.0	1004.5	1156.1	1307.6	
64	97.0	248.5	400.0	551.5	703.0	854.5	1006.1	1157.6	1309.1	
65	98.5	250.0	401.5	553.0	704.5	856.1	1007.6	1159.1	1310.6	
66	100.0	251.5	403.0	554.5	706.1		1009.1	1160.6	1312.1	
67	101.5	253.0	404.5	556.1	707.6	859.1	1010.6	1162.1	1313.6	
68	103.0	255.5	406.1	557.6	709.1	860.6	1012.1	1163.6	1315.2	
69	104.5	256.1	407.6	559.1	710.6	862.1	1013.6	1165.2	1316.7	
70	106.1	257.6	409.1	560.6	712.1	863.6	1015.2	1166.7	1318.2	
71	107.6	259.1	410.6	562.1	713.6	865.2	1016.7	1168.2	1319.7	
72	109.1	260.6	412.1	563.6	715.2	866.7	1018.2	1169.7	1321.2	
73	110.6	262.1	413.6	565.2	716.7	868.2	1019.7	1171.2	1322.7	
74	112.1	263.6	415.2	566.7	718.2	869.7	1021.2	1172.7	1324.2	
75	113.6	265.2	416.7	568.2	719.7	871.2	1022.7	1174.2	1325.8	
76	115.2	266.7	418.2	569.7	721.2	872.7	1024.2	1175.8	1327.3	
77	116.7	268.2	419.7	571.2	722.7	874.2	1025.8	1177.3	1328.8	
78	118.2	269.7	421.2	572.7	724.2	875.8	1027.3	1178.8	1330.3	
79	119.7	271.2	422.7	574.2	725.8	877.3	1028.8	1180.3	1331.8	
80	121.2	272.7	424.2	575.8	727.3	878.8	1030.3	1181.8	1333.3	
81	122.7	274.2	425.8	577.3	728.8	880.3	1031.8	1183.3	1334.8	
82	124.2	275.8	427.3	578.8	730.3	881.8	1033.3	1184.8	1336.4	
83	125.8	277.3	428.8	580.3	731.8	883.3	1034.8	1186.4	1337.9	
84	127.3	278.8	430.3	581.8	733.3	884.8	1036.4	1187.9	1339.4	
85	128.8	280.3	431.8	583.3	734.8	886.4	1037.9	1189.4	1340.9	
86	130.3	281.8	433.3	584.8	736.4	887.9	1039.4	1190.9	1342.4	
87	131.8	283.3	434.8	586.4	737.9	889.4	1040.9	1192.4	1343.9	
88	133.3	284.8	436.4	587.9	739.4	890.9	1042.4	1193.9	1345.5	
89	134.8	286.4	437.9	589.4	740.9	892.4	1043.9	1195.5	1347.0	
90	136.4	287.9	439.4	590.9	742.4	893.9	1045.5	1197.0	1348.5	
91	137.9	289.4	440.9	592.4	743.9	895.5	1047.0		1350.0	
92	139.4	290.9	442.4	593.9	745.5	897.0	1048.5	1200.0	1351.5	
93	140.9	292.4	443.9	595.5	747.0	898.5	1050.0	1201.5	1353.0	
94	142.4	293.9	445.5	597.0	748.5	900.0	1051.5	1203.0	1354.5	
95	143.9	295.5	447.0	598.5	750.0	901.5	1053.0	1204.5	1356.1	
96	145.5	297.0	448.5	600.0	751.5	903.0	1054.5	1206.1	1357.6	
97	147.0	298.5	450.0	601.5	753.0	904.5	1056.1	1207.6	1359.1	
98	148.5	300.0	451.5	603.0	754.5	906.1	1057.6	1209.1	1360.6	
99	150.0	301.5	453.0	604.5	756.1	907.6	1059.1	1210.6	1362.1	

## APPENDIX.

IN the former edition of this work there was an error in the Rule which appears, in the present edition, at page 65, the word “tangent” having been written for “sine.” A scientific friend, in noticing this mistake, has suggested a convenient method of arranging the figures of the calculation when the perpendicular and base of a right-angled triangle are *both* to be computed from the base-angle and hypotenuse. The directions are as follows :—

“1st. Write down the log. of the measured hypotenuse taken from the table of numbers.

“2nd. Over it place the log. sine of the measured angle from table of log. sines, &c., and draw a line *above*.

“3rd. Under the log. of the hypotenuse write down the cosine of the angle, and then draw a line *under* it. Add the hypotenuse to the sine *upwards*, and it will give the length of the perpendicular sought in the table of numbers. Add the hypotenuse and cosine together *downwards*, and it will give the length of the base in the table of numbers.

“NOTE.—I reject 10 from the log. index in both sums, because radius 10 stood in the first term, as a divisor, in both proportions.



## “ EXAMPLE.

“ From A to C, I found the angle to be  $6^\circ$  rising—and the length of the hypotenuse A C measured 1240 links.

“ I state the calculation thus :

$$2.112657 = \log. 129_{16}^6 \text{ the perp}^r.$$

$$\text{2nd, Sine of } 6^\circ = 9.019235$$

$$\text{1st, Hypotenuse 1240} = 3.093422$$

$$\text{3rd, Cosine of } 6^\circ = 9.997614$$

$$3.091036 = \log. 1233_{16}^2 \text{ the base.}$$

Answer :

Perpendicular . . .  $129_{16}^6$  links.

Base . . . . .  $1233_{16}^2$  do.

Hypotenuse . . . 1240 do.”

We may add here, in reference to the precept in italics, at page 65, that the constant number 1.8195439 is the logarithm of 66, the number of feet in a chain.

ON SETTING OUT THE WIDTHS OF GROUND REQUIRED

FOR THE WORKS OF A

RAILWAY OR CANAL,

DEPENDING UPON THE DEPTH OF CUTTING OR  
HEIGHT OF EMBANKMENT,  
AND THE TRANSVERSE SLOPE OF THE NATURAL SURFACE.

BY

FREDERICK WALTER SIMMS, F.G.S., M.I.C.E.



# ON SETTING OUT THE WIDTHS OF GROUND REQUIRED

FOR THE WORKS OF A

## RAILWAY OR CANAL,

&c., &c.

---

WHEN the natural surface of the ground, both longitudinally and transversely, is upon the same level as that of the intended works, the process of setting and staking out the widths is very simple. Let us take, for example, the case of a railway, the base or bottom width of which when prepared for the reception of the ballasting and permanent way, is to be 36 feet; the ratio of the inclination, or batter, of the slopes to the heights, both in the cuttings and the embankments, to be 2 to 1 beyond which, or at the outward edge, a slip of land 12 feet wide is to be taken on each side of the railway for the fences, &c. First, the centre line must be staked out and carefully levelled: it is customary to drive a stake, about 2 feet long and about  $1\frac{1}{2}$  inch square, into the ground at each chain's length, their tops to be upon the fair level of the natural surface, thus affording good stations for the levelling staves to be held upon; the relative level of each stake being then very accurately determined with respect to some given datum, they become so many zero points for reference in the subsequent operations. From each of the centre stake



ON SETTING OUT THE WIDTHS OF GROUND REQUIRED

FOR THE WORKS OF A

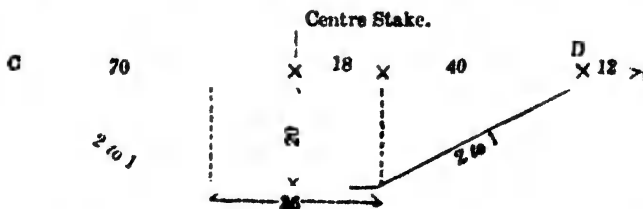
RAILWAY OR CANAL,

&c., &c.

WHEN the natural surface of the ground, both longitudinally and transversely, is upon the same level as that of the intended works, the process of setting and staking out the widths is very simple. Let us take, for example, the case of a railway, the base or bottom width of which, when prepared for the reception of the ballasting and permanent way, is to be 36 feet; the ratio of the inclination, or batter, of the slopes to the heights, both in the cuttings and the embankments, to be 2 to 1; beyond which, or at the outward edge, a slip of land 12 feet wide is to be taken on each side of the railway for the fences, &c. First, the centre line must be staked out and carefully levelled: it is customary to drive a stake, about 2 feet long and about  $1\frac{1}{2}$  inch square, into the ground at each chain's length, their tops to be upon the fair level of the natural surface, thus affording good stations for the levelling staves to be held upon; the relative level of each stake being then very accurately determined with respect to some given datum, they become so many zero points for reference in the subsequent operations. From each of the centre stakes

a line must be set out on both sides, and at right angles to the centre line, or at right angles to a tangent to the centre line at that point, if the centre line be curved: upon these transverse lines the required widths of land must be set out. Now, if the ground at any of the centre stakes is upon the same level as the intended base of the railway, nothing more will be required than to measure on each transverse line, and in both directions from the centre stake, one half the required width, which, in our supposed case, is 18 feet for the half width of the railway, and 12 feet for the fences; in all 30 feet on each side of the centre. But when, as it mostly happens, the ground is not on the proposed level of the railway, the operation is not quite so simple; and if in addition thereto the ground slopes sidewise or at right angles to the general direction of the line, the business is still more complicated, and requires some skill and care to do the work correctly. The method of doing this it is now our business to explain.

The next most simple case to the above is when the cross section of the ground is horizontal, be the depth of cutting or height of embankment what it may.



This is shown in the above diagram, which represents a cross section of a 20 feet cutting, with slopes of two horizontal to one perpendicular. The horizontal line A B at right angles to the centre line represents the natural surface of the ground. Under these circumstances it

will readily be seen that the half width of the cutting, or the distance from the centre to the edge of the slopes C and D, equals the half width of the base (18) added to the batter of the sloping sides (40), and including the 12 feet for the fences, the total half width of land required for the purposes of such railway would be  $18 + 40 + 12 = 70$  feet, and consequently the whole required width to be appropriated and fenced in for a 20 feet cutting or embankment, when the ground does not slope sidewise, would be 140 feet.

The next and more complicated, and also the most frequently occurring case, is, when the cross section of the natural surface is not horizontal, as shown in the annexed diagram, which also represents a cutting of 20 feet.

Let the line A B represent a horizontal line passing through the centre line C of the railway, which, if it coincided with the surface of the ground, would give A C and C B (each half width) 70 feet, as in the former example, the depth of cutting and the slopes being assumed the same.

20 = Centre.

Let the line E H represent the natural surface of the ground upon this transverse section; it will readily be perceived that the real half width C E (on the left of the diagram) is much shorter than the horizontal or computed half width A C, because the ground-line is depressed on that



side of the centre; likewise the half width  $CH$  on the other side of the centre is greater than the said horizontal or computed half width, because the ground is there elevated above the horizontal line  $AB$  passing through the centre. To determine *exactly* the distances  $CE$  and  $CH$  in actual operations in the field, would be attended with some difficulty, and consume much time; but the following method, at the same time that it gives a sufficiently correct approximation in practice, is also a very expeditious one:

Let us suppose that the point  $E$  or distance  $CE$  be known, and that with a spirit-level we determine the difference of level between the points  $C$  and  $E$ , this difference is represented by the line  $EF$ , which suppose to be one foot; now we have a small right-angled triangle  $AEF$ , of which  $EF$  is determined, being the difference of level (one foot), and the slope or ratio of  $AF$  to  $EF$  also given (2 to 1), therefore the side  $AF$  is known (2 feet), which, subtracted from the computed half width  $AC$ , leaves  $FC$  approximately equal to  $EC$ , the required half width, sufficiently exact for all practical purposes, where the cross section of the ground does not differ materially from a horizontal line.

We have been supposing that the point  $E$  is known, whereas that point is the object of our search; in practice, therefore, we proceed thus:—Take the computed half width, and if the ground is *depressed*, let a levelling staff be held somewhat *nearer* the point  $C$  than the said computed half width, for a first approximation to the point  $E$ ; then determine the *difference of level* between this assumed point and the centre point  $C$ , *multiply this difference of level by the ratio of the slopes* (which

•

doubles it when the slope is 2 to 1), and *subtract* the result from the computed half width, which gives a more correct approximation to the point E ; now hold the staff at this *new point* and find the difference of level as before, again multiply by the ratio of the slopes, and deduct the result from the computed half width, which second result will in most cases be sufficiently near the real half width for a *depressed* line for all practical purposes.

EXAMPLE.—Central height (or depth of cutting), 20 feet, slopes 2 to 1, base 36 feet ; the computed half width was therefore 58 feet ; the ground being depressed, we estimated that the point E might fall short of the computed half width 2 feet : we therefore directed a levelling staff to be held at 56 feet from the centre line (or stake) C, at which point another staff was held, and, by means of a spirit-level set up at a convenient distance, we found the difference of level between these points to be 0·87 foot, which multiplied by the ratio of the slopes (2 to 1), gave 1·74 foot to be subtracted from the computed half width 58 feet, leaving 56·26 feet for a first approximation to the half width C E (see last diagram). Now, upon removing the staff to this new point, the difference of level was again taken (or rather we should say that the staff was again read off, as the level had not been disturbed), and found to be 0·91 foot, which, also multiplied by the ratio of the slopes (2 to 1), gave 1·82 foot to be subtracted from 58 feet, leaving 56·18 for the second approximation, and which was adopted as the correct half width for the depressed side of the centre ; indeed, in such a case as is above given, where the ground is so nearly horizontal, the first approximation,

(taken by a person after a little practice) may be assumed as the correct result, for in the above example it differed but  $\cdot 08$  from the second determination, and if it had been taken a third time it could not have been more accurate as far as practice is concerned; this, however, is not the case where the inclination or slope of the ground is considerable, for then (if this method be followed) several approximations will be necessary to bring the result within admissible limits.

When the ground is *elevated* above the horizontal line, as shown on the right hand of the diagram, the mode of procedure will somewhat differ: thus, instead of holding the staff and finding the difference of level at a *less distance* than the computed half width, it must be held at a *greater distance* to obtain the point H by approximation; the difference of level between that point and the centre point C being equal to H I, which multiplied by the ratio of the slopes, will give the distance B I to be added to the computed half width C B, to obtain the half width C H; this may likewise be repeated to obtain a more correct result, as described for the other or depressed side of the centre C. It will also here be obvious to a person possessing but the smallest share of mathematical knowledge, that this result is not strictly correct, inasmuch as the line C H can never be equal to C I, but for practical purposes it is, as before observed, sufficiently correct. It may not be altogether unnecessary to observe, in this place, that the corrections B I, &c., as shown in the foregoing diagrams, are much exaggerated, being far greater in proportion to the computed half width C B, than ever occurs in ordinary practice, but this has been done to make our explanations more distinct than they otherwise would be.

The above particulars have been confined to the case of excavations; we must now show in what the process differs when the ground is to be covered with an embankment.

By reversing page 108 we invert the diagram, which then represents an embankment. The rule for finding the half width for an embankment where the transverse section of the ground is horizontal, remains the same as for the cuttings under like circumstances, as may be seen by an inspection of the inverted figure of the first diagram; but upon inverting the second diagram, it will at once be seen that some variation in the process is required. Thus:

The horizontal line is represented by that marked A B ; C D and C F are the computed half widths; C E the required half width on the depressed side, and C H the required half width on the elevated side, the line K L representing the natural surface of the ground. In the case of an excavation, we have shown that the *real* half width is greater on the *elevated* side than the computed half width, and less on the *depressed* side; but it will be seen by the above diagram that for an embankment the *real* half width is *less* on the *elevated* side, and *greater* on the *depressed* side, than the computed half width; therefore,

in determining the approximate place of the point E on the depressed side for an embankment, the staff must be held *further* from the centre than the computed half width; and for the point H, or the elevated side, it must be held *nearer* to the centre than the computed half width; and finally, for computing the real half widths from the differences of level between the points E and the centre, and H and the centre; on the *depressed* side the difference of level multiplied by the ratio of the slopes is to be *added* to the computed half widths to obtain the point E, and to be *subtracted* from the computed half widths to obtain the point H.

The process above described may appear to the reader a very tedious one; it perhaps is so to read; but a little practice will convince him that it is a very expeditious method, for in most cases one setting up of the level will answer for several stations, and the multiplication by the ratio of the slopes upon such small numbers as mostly occur is easily performed, especially if it be an even number, as 2 to 1. The columns of the field-book may be arranged as in the following example for making the calculations in the field, or may be abridged to suit a more convenient sized book for the pocket, at the pleasure of the surveyor; indeed, all that can be accomplished now of this kind is to give general rules, which can be altered and arranged to suit the convenience of the surveyor, as experience may point out a more suitable mode of proceeding. The example is taken from an extensive field operation by the writer, and shows the work both for a cutting and an embankment; the change from one to the other, or the tailing out of the cutting, as it is called, being included therein. The slope of the

cutting is calculated at  $1\frac{1}{2}$  to 1, and that of the embankment at 2 to 1. The width of the railway was 36 feet, consequently half the said width was 18 feet.

## EXAMPLE.

		an		Difference of Level.		Diff. x 1			
				+ -					
				South.		North.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
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				Feet.		Feet.			
				Feet.		Feet.			
				Feet.		Feet.			
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The first column contains the number of the central stakes, reckoned from the commencement of the work, which are convenient for reference.

The second column contains the depth of cutting or the height of embankment, as the case may be, at that point on the centre line.

The third column, the computed half width from the centre line to the edge of the cutting, or foot of embankment, upon the supposition that the ground is horizontal at right angles to the centre line; this half width, as before explained (p. 109), is found by multiplying the central height by the ratio of the slopes, and adding to the product half the width at the base of the railway.

The fourth, fifth, and sixth columns contain the readings from the levelling staves at the centre stake, and at the approximate points E and H (see last diagram).

The seventh and eighth columns contain the differences of level between the centre stake and the above approximate points. These numbers are simply the differences of the quantities in the three preceding columns (except at stakes 286 and 290, which we will presently explain), and the signs + and - denote whether they are positive or negative quantities, as respects the centre and the approximate points E and H.

The ninth and tenth columns contain the differences of level (contained in columns 7 and 8) multiplied by the ratio of the slopes, and must have the same signs + or - as the corresponding numbers in the preceding columns.

The last two columns contain the final half widths obtained by adding or subtracting, according to the prefixed signs + or -, the numbers in the two pre-

ceding columns to the computed half width contained in column 3.

After the explanations already given, the reader can find no difficulty in tracing the steps of the example, except perhaps with the stakes 286 and 290, where the difference of level on the north side is represented by two numbers bracketed together, one having the sign +, and the other - : for the stake 286 the real difference of level on the north side the centre is a rise of 1.50, that is, the approximate point H is 1.50 foot above the centre stake: but it happens that the height of the embankment itself at that point is to be but 1.43 foot (column 2); therefore the approximate point H is above the intended top of the embankment, and consequently will not represent the foot of an embankment, but the edge of a cutting, and therefore the calculation for the half width on the north side must be treated as for a cutting whose depth is equal to the *height of the approximate point H above the intended top of the embankment*; or, in other words, the *excess* of the difference of level between the centre stake and the approximate point H, above the intended height of the embankment, is the quantity to be entered in the column (7 or 8) "Difference of Level," and to be computed as for a cutting instead of embankment. In the case of stake 286 this excess is 0.07, to which is prefixed the sign plus; this sum multiplied by the ratio of the slope being additive (for a cutting) on the elevated side of the centre, as before explained.

For the stake 290, the north side of the line (column 6) is 1.34 higher than the centre stake, and it, being embankment, would have the sign - prefixed (as shown





ference of level); the remainder, H M (which is the *excess of the difference of level between the centre stake and the approximate point H above the intended height of embankment*), multiplied into the ratio of the slope, must be added to the computed half width, or, in other words, treated as for a cutting, to obtain the said point H, as before stated. By reversing the diagram the corresponding case will become evident; namely, when the centre line is in cutting, and one side on embankment, while the other is in excavation; and the mode of proceeding will at once strike the reader after perusing what we have above written.



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EXAMPLES OF THE MODES  
OF SETTING OUT  
RAILWAY CURVES.

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BY HENRY LAW.



## EXAMPLES OF THE MODES OF SETTING OUT

### RAILWAY CURVES.

THERE are very few lines of railway so favourably situated as to be free from curves of greater or less extent, and occurring more or less frequently in their course ; and consequently a knowledge of the method of correctly and readily laying down these curves upon the ground becomes a very necessary and important qualification in those engaged in setting out lines of railway. It has not therefore been considered out of place to append to a work treating on one of the most important branches of railway surveying, a description of the various methods which may be employed for this purpose.

Previously, however, to proceeding to the more practical part of the subject, it may be desirable to make a few observations upon railway curves in general. The curve which has been almost universally employed in laying down lines of railway, is the arc of a circle, although it may be shown, that under certain circumstances this curve is not *theoretically* that which should be employed, as affording the least danger from the centrifugal tendency of the carriages. It has been generally considered, that the *true curve* was one which commenced with an infinite radius, decreasing in a regular manner

in advancing on the curve, until the minimum radius of curvature required had been attained. This form of curve has, however, been deduced upon the assumption, that the *whole* of the centrifugal tendency of the carriages is balanced by the superelevation of the outer rail, an assumption which is only correct on the supposition of the wheels of the carriages being cylindrical, or no play being allowed between the flanges and the rails—conditions which are never fulfilled in practice. For it may be shown, that with conical wheels, and a certain amount of play, a portion of the centrifugal tendency will always be counteracted by the self-acting adjustment produced by the lateral deviation of the carriage on the rails, however small the radius of curvature may be; and that when the radius exceeds a certain limit, this adjustment is perfect, no superelevation of the outer rail being then required.

It is obvious, therefore, that in curves whose radii are *within* this limit, the true form for the curve is one whose radius of curvature at its commencement should equal this limit, and should decrease in advancing upon the curve, according to such a law, that (assuming the rise in the outer rail to form a regular inclined plane) the unbalanced centrifugal tendency should at every part of the curve be exactly counteracted by the amount of the superelevation of the rail at that part; until the top of the incline being reached, the radius of curvature should then remain constant, being such that the centrifugal tendency of the train should be exactly balanced by the combined effect of the lateral deviation of the carriages on the rails, and the superelevation of the outer rail. When, however, the radius of curvature ex-

ceeds this limit, it may be shown that the arc of a circle is preferable to any other form of curve.

Now if we put  $d$  for the diameter of the wheels of the carriages,  $w$  for the width of the gauge of the line, and  $p$  for the lateral play allowed *on each side*, between the flanges of the wheels and the rails (all the dimensions being expressed in feet), and  $\frac{1}{n}$  being the ratio of inclination of the tire of the wheel; then

$$\frac{n}{4} \frac{dw}{dp} = R \quad . \quad . \quad . \quad . \quad . \quad I.$$

will be the limit above referred to; that is,  $R$  will be the least radius of curvature which may be used without the necessity of raising the outer rail. And for any other smaller radius, putting  $v$  for the velocity of the train in miles per hour, and  $r$  for the radius of the curve in feet ; then

$$782 \frac{r^2 (n d r - 4 p r)}{n d r} = \epsilon \quad . \quad . \quad \text{II.}$$

will be the superelevation of the outer rail in inches, which will be required for that radius, in order that the whole of the centrifugal tendency of the train may be destroyed.

Although we have thus shown, that for all curves having a smaller radius than  $R$ , it would not be correct, *theoretically*, to employ the arc of a circle, it is nevertheless very questionable whether it would be advisable, *in practice* (except under peculiar circumstances), to substitute the theoretical curve in its stead, inasmuch as the circular arc possesses the practical advantage of being laid down upon the ground with far greater facility; and the only real objection which can be made to its use—namely, that of requiring a sudden and instantaneous superelevation of the outer rail at the point where



the curve commences—may in a great measure be removed by commencing to raise the rail before arriving at this point, and making the rise form a gradual inclined plane, whose summit shall be attained at the commencement of the curve. By the adoption of this plan, although the centrifugal tendency (as without it) commences suddenly, the counteracting force, produced by the superelevation of the outer rail, at the same instant attains its maximum, and the two forces therefore balance each other. Whereas, without it, the sudden commencement of the centrifugal force, being entirely unopposed, would at first tend to throw the carriages off the line, until, by the gradual elevation of the outer rail, it had been entirely destroyed.

Having thus far pursued the inquiry as to which form of curve it is most expedient to employ in practice in laying down a line of railway, and having shown that with hardly an exception the arc of a circle is practically the best, we shall confine ourselves to describing a few of the most generally applicable methods by which the circular arc may be traced on the ground.

The first method which we shall describe is that which has in practice been perhaps the most extensively used, although it possesses some objections, which we shall point out in the sequel. Let A B and C D (fig. 1, plate 6) be the two straight portions of the line which it is desired to connect by a curve, B and C being the two points at which the curve falls into the straight lines; and let  $Bb_1, b_1b_2, b_2b_3, \&c.$ , be the distance which it is desired that the points to be found in the curve shall be apart: then measure upon the straight line A B produced, the distance B  $a_1$ , equal  $\delta_1$  in formula IV. below,

and from the point  $a_1$  set off, perpendicular to the same line, the distance  $a_1 b_1$  equal to  $o_1$  in formula III., which will give the first point required in the curve; then range a straight line through the points B,  $b_1$ , and upon this line lay off the distance  $b_1 a_2$ , equal to  $\delta_2$  in formula VI., and from the point  $a_2$  set off, perpendicular to the line B  $a_2$ , the distance  $a_2 b_2$ , equal to  $o_2$ , in formula V., and the point  $b_2$  will be the second point in the curve; then in a similar manner range another line through the points  $b_1 b_2$ , upon which measure the distance  $b_2 a_3$ , equal to the distance  $\delta_2$  or  $b_1 a_2$ , and from  $a_3$  set off as before, perpendicular to the line  $b_1 a_3$ , the distance  $a_3 b_3$ , equal to  $o_2$ , which will determine the third point in the curve: and thus proceed until the whole extent of the curve has been set out.

In order to obtain the values of  $\delta_1$ ,  $\delta_2$ ,  $o_1$ , and  $o_2$ , let  $r$  equal the radius and  $d$  equal the distance B  $b_1$ , or  $b_1 b_2$ , &c., which it is desired that the points found in the curve shall be apart (both expressed in feet): then

$$\frac{d^2}{2r} = o_1 \quad . \quad . \quad . \quad . \quad . \quad . \quad . \quad \text{III.}$$

$$\sqrt{d^2 - o_1^2} = \delta_1 \quad . \quad . \quad . \quad . \quad . \quad . \quad . \quad \text{IV.}$$

VI.

As an example of the application of this method, let the radius of the curve ( $r$ ) be 15 chains or 990 feet, and the distance B  $b_1$  ( $d$ ) one chain or 66 feet; then from formula III.

$$66^2 \quad - \quad o_1^2$$

will be the first offset at  $a_1$ ; and

$$\sqrt{66^2 - 2 \cdot 2^2} = 65 \cdot 963 \text{ feet} = \delta_1$$

will be the distance B  $a_1$ , to be laid off upon the line A B produced to give the place for this offset. Again,

$$\frac{66 \times 65.963}{990} = 4.397 \text{ feet} = o_2$$

will be the offset at  $a_2, a_3, a_4$ , &c. ; and

$$\frac{.2 \cdot 2}{.2} = 65.85 \text{ feet} = \delta_2$$

will be the distance  $b_1 a_2, b_2 a_3$ , &c., to be measured from the points  $b_1, b_2$ , &c., in order to give the points  $a_2, a_3, a_4$ , &c., from which the offsets  $o_2$  are to be taken.

To this method there are, as has been already stated, some practical objections, inasmuch as any error which may be committed, in setting out only one of the points in the curve, will occasion a corresponding error in every succeeding one; and a very trifling inaccuracy in calculating either the distance  $\delta_2$ , or the length of the offset  $o_2$ , from its being frequently repeated, may ultimately cause a very considerable deviation from the true curve. Both these objections, however, may be in a great measure removed by the adoption of the following method of checking the position of about every fifth point; or, which would be better, first determining the position of these points, and then filling in the intermediate ones; and as we consider this modification does away almost entirely with the above-mentioned sources of error, we shall give an example of its application.

Let us suppose  $r$  and  $d$ , or the radius, and the distance the points B,  $b_1, b_2$ , &c., are apart (see figure 2, plate 6), to be the same as in the last example—viz., 990 feet and 66 feet respectively, and let it be determined to check the position of every *fourth* point: then the values of  $\delta_1, \delta_2, o_1$ , and  $o_2$ , will be the same as before; but previous to

setting out the points  $b_1, b_2, b_3$ , &c., we must calculate the distance  $BB_1$  to be measured along the line  $AB$  produced, and the distance  $B_1b_4$  to be set off from the point  $B_1$  to give the position of the fourth point ( $b_4$ ) in the curve, which may be done as follows: Let the distance  $BB_1$  equal  $\Delta_1$ , and  $B_1b_4$  equal  $O_1$ ; and let  $D_1$  be the length of the chord line connecting the two points  $B$  and  $b_4$ , and  $\beta$  be the angle  $\alpha_1 B b_1$ ; then

$$\frac{o_1 \text{ rad}}{d} = \sin \beta,$$

and

$$\frac{2 r \sin 4 \beta}{d} = n.$$

Then, by substituting  $D_1, O_1$ , and  $\Delta_1$ , for  $d, o_1$ , and  $\delta_1$ , in the formulae III., IV., V., and VI., we shall obtain the values of  $O_1, \Delta_1, O_2$ , and  $\Delta_2$ , where  $\Delta_2$  is the distance  $b_4 B_2$  to be measured upon the chord line  $B_1 b_4$  produced, and  $O_2$  is the distance  $B_2 b_8$  to be set off from  $B_2$  in order to give the eighth point ( $b_8$ ) in the curve; for the values of  $r$  and  $d$  given above we shall obtain

$$\begin{aligned} \text{Log } o_1 &= 0.342423 = \log 2.2 \\ \text{Log rad} &= 10.000000 \end{aligned}$$

$$\begin{aligned} &10.342423 \\ \text{Log } d &= 1.819544 = \log 66 \end{aligned}$$

$$\text{Log } \sin \beta = 8.522879$$

and  $\beta = 1^\circ 54' 37'' \therefore 4 \beta = 7^\circ 38' 28''$ ; then

$$\begin{aligned} \text{Log } 2 r &= 3.296665 = \log 1980 \\ \text{Log } \sin 4 \beta &= 9.123745 \end{aligned}$$

$$\begin{aligned} &12.420410 \\ \text{Log rad} &= 10.000000 \end{aligned}$$

$$= 2.420410 = \log 263.27.$$

Then from formula III.

$$263 \cdot 27^2 \text{ — } 0$$

from formula IV.

$$\sqrt{-35^2} = 260 \cdot 92 \text{ feet} =$$

from formula V.

$$\frac{263 \cdot 27 \times 260 \cdot 92}{990} = 69 \cdot 4 = O_2;$$

and from formula VI.

$$990 = 253 \cdot 96 = \Delta_2.$$

These being obtained, the position of every fourth point,  $b_4, b_8, b_{12}, \&c.$ , should be first determined by the dimensions  $\Delta_1, O_1, \Delta_2$ , and  $O_2$ ; and then the intermediate points  $b_1, b_2, b_3, b_5, b_6, \&c.$ , by  $\delta_1, o_1, \delta_2$ , and  $o_2$ , as first described.

The second method which we shall describe may be advantageously employed when the radius of curvature is large and the centre can be seen from every part of the curve.

Let the lines (figure 3, plate 6) A B and C D as before represent the two straight portions of the line required to be connected by a curve having a radius of 80 chains or 1 mile. First set up a theodolite at B and another at C (the two terminations of the straight portions of the line) and from each point range a line at right angles to the lines A B and C D respectively, and at the intersection of these lines (E), which will be the centre of the curve, put up a signal sufficiently conspicuous to be seen from any point between B and C. Then produce the straight lines A B and C D until they intersect in the point F, and on these lines drive in stakes at equal distances,  $a_1, a_2, a_3, \&c.$ , commencing from the points

# RAILWAY CURVES.

B and C. If  $r$  equal the radius, and  $\delta$  equal the distance between the points  $a_1, a_2, a_3$ , &c., both in feet, then

$$-r = o_1$$

will be the distance which must be set off from the first point  $a_1$ , not perpendicular to the line B F, but in the direction  $a_1$  E; in like manner

will be the distance to be set off from the point  $a_2$  in the direction  $a_2$  E; and generally

will be the distance to be set off at the  $n$ th points from B and C.

For example, let  $r$  be 5280 feet and  $\delta$  equal 100 feet: then

$$\sqrt{5280^2 + 100^2} - 5280 = \cdot 94 \text{ feet} = o_1$$

will be the distance  $a_1 b_1$ , which must be set off from  $a_1$  in the direction  $a_1$  E to obtain the first point  $b_1$  in the curve; and proceeding in a similar manner with the others, the following Table will exhibit the distances to be set off at the respective points  $a_1, a_2, a_3$ , &c.:

At $a_1$	or	100 feet from B, the offset will be	$\cdot 94$ feet.
$a_2$	200	" "	3.79
$a_3$	300	" "	8.52
$a_4$	400	" "	15.13
$a_5$	500	" "	23.62
$a_6$	600	" "	33.98
$a_7$	700	" "	46.19
$a_8$	800	" "	60.26
$a_9$	900	" "	76.16
$a_{10}$	1000	" "	93.86
$a_{11}$	1100	" "	113.36
$a_{12}$	1200	" "	134.65
$a_{13}$	1300	" "	157.68
$a_{14}$	1400	" "	182.45
$a_{15}$	1500	" "	208.93

If the extent of the curve is such that the length of the offsets before reaching the point F, where the two tangent lines intersect, become inconveniently long, so as to occasion a loss of time in setting them off, it will be advisable to make use of another tangent line as shown at G I, fig. 4, plate 7; for determining the position of which line the following method may be made use of. Let  $r$ , as before, be the radius,  $\epsilon$  the number of degrees contained by the angle B E C, and  $n$  the number of tangent lines (as B G, G H, H I, I C) intended to be employed; then

$$\frac{r \sin \frac{\epsilon}{n}}{\cos \frac{\epsilon}{n}} = r \tan \frac{\epsilon}{n}$$

will be equal to the length of any one of these tangent lines. As an example, let  $r$  be equal to 5280 feet,  $\epsilon$  equal to  $60^\circ$ , and  $n$  equal to 4, so that the quotient of  $\epsilon$  divided by  $n$  will be  $15^\circ$ : then the calculation for the length of each of the lines B G, G H, &c., will be as follows:—

$$\text{Log } r = 3.722634$$

$$\text{Log } \tan \frac{\epsilon}{n} = 9.428052$$

$$3.150686 = \log 1414.8.$$

Hence the length of each of the lines B G, G H, &c., will be 1414.8 feet.

Now, having ascertained this length, nothing more remains than to set it off from B and C towards F, and then to range a line G I from the two points thus obtained, which will be the required tangent line: this line must then be bisected in the point H, which may

readily be done by ranging a line from F to E, which having been done, proceed as already described to set off the equal distances  $a_1, a_2, a_3, \&c.$ , from B and H towards G, and from H and C towards I; and then by setting off the distances  $a_1, b_1, a_2, b_2, \&c.$ , contained in the Table already given, from the several points  $a_1, a_2, \&c.$ , in directions radiating to the centre E, the course of the curve will be marked by the points  $b_1, b_2, b_3, \&c.$ , thus obtained.

One advantage possessed by the above method is, that, knowing exactly the direction in which to lay off the offsets (and that by the range of a comparatively distant object), the errors which have frequently arisen from their not having been set off perpendicularly, where the eye has been the only criterion, are entirely obviated; and this method is also entirely free from the objections made to the former method.

When the centre point E cannot be seen from every part of the curve, so as to allow the offsets being laid off radially, the more usual method may be adopted of laying off the offsets perpendicularly to the tangent BF, but in this case a cross staff should always be employed to insure accuracy, and the distances to be set off from the points  $a_1, a_2, a_3, \&c.$ , will be greater than those employed in the previous method, and must be calculated from the formula

instead of that given at page 131.

The third method is most applicable where the radius of the curve is small as compared with its extent, and is deduced from the well-known theorem, that all angles



contained in the same segment of a circle are equal to one another.\* The method is as follows:—Place a theodolite at B and another at C (figure 5, plate 7), the two terminations of the straight portions of the line, setting the telescope of the instrument at B on C, and that at C on F, the point of intersection of the lines A B and C D produced; then if the former be moved through an arc of any number of degrees, towards F, and the latter the same number of degrees towards B, the point  $a_1$ , where the lines of collimation of the two telescopes intersect, will be a point in the curve; now let both theodolites be again moved the same number of degrees and in the same directions as before, and their axes produced, or lines of collimation, will again intersect at  $a_2$ , another point in the curve; and in fact, to whatever extent the theodolites are moved, so long as the arc described is equal in both, the point of their intersection will always be in the required curve. Or more generally, suppose the two theodolites to be placed as first described, and then simultaneously to commence to revolve with the same uniform angular velocity, the point of intersection of their lines of collimation will describe the circular arc C,  $a_1$ ,  $a_2$ ,  $a_3$ , . . . . B; and in equal intervals of time, equal portions of the arc will be described, which will be half as great as the arc, which would have been described in the same time, by the same angular velocity, at the centre of the circle (E); from which last-mentioned circumstance, we may readily calculate the magnitude of the angle through which the theodolites at B and C must be successively moved, in order that the points  $a_1$ ,  $a_2$ ,  $a_3$ , &c., at which their axes intersect, may

\* Euclid, Book III., prop. 21.

be at the distance apart which it is desired that they should be. If  $r$  equal the radius of the curve,  $d$  the required distance, and  $\beta$  the angle  $a_1$  B C; then

$$\frac{d \sin \beta}{2r} = \sin \beta \quad . \quad . \quad . \quad . \quad . \quad \text{VII.}$$

As an example of the application of this method, let  $r$  equal 20 chains, or 1320 feet, and let it be required to determine points in the curve at distances of about 100 feet; now, from the above formula we shall obtain

$$\text{Log } d = 2.000000 = \log 100$$

$$\text{Log rad} = 10.000000$$

$$12.000000$$

$$\text{Log } 2r = 3.421604 = \log 2640$$

$$\text{Log } \sin \beta = 8.578396 \therefore \beta = 2^\circ 10' 15''.$$

As it would be inconvenient, however, in practice, to lay off so frequently as would be required, an angle with odd minutes and seconds, we may instead of the above take an angle of 2 degrees, which will make the distance  $d$  equal 92.13 feet. Having thus determined the angle, and placed the theodolites as previously described—viz., that at B in the direction B C, and that at C in the direction C F—the former must be moved  $2^\circ$  towards F, and the latter  $2^\circ$  towards B, and a stake driven down at their point of intersection  $a_1$ ; the former must then be moved  $2^\circ$  more towards F, and the latter  $2^\circ$  more towards B, and another stake put down at their point of intersection  $a_2$ , and so on until the theodolite at B is brought to the direction B F, and that at C to the direction C B, when the whole of the curve will have been staked out as required, the stakes being 92.13 feet

apart. This method, the same as the last, is not liable to the objections that the first method was, and in addition possesses the very important practical advantage, that its accuracy is entirely independent of any undulation or change of level in the surface of the ground, an advantage which is not possessed by any of the other methods which we have described, the whole of which would require to have the distances and offsets corrected in proportion to the slope of the surface of the ground. In a hilly country—and it is in such districts that curves most frequently occur—this circumstance will render the last-described method far superior to either of those which precede it.

The next method which we shall give, is that described by Mr. Rankine, in a communication to the Institution of Civil Engineers, and depends on the theorem \* that the angle subtended by any arc of a circle at the centre of a circle, is double the angle subtended by the same arc at any point in the circumference of the circle. The method of proceeding is as follows: first place a theodolite at B (figure 6, plate 7), the point where the curve commences; and then lay off from the line BF the angle B, calculated from formula VII. (supposing as before  $r$  to represent the radius of the curve and  $d$  the distance required between the points in the curve), and in the direction of the axis of the instrument set off the distance  $d$ , which will give the first point  $a_1$  in the curve; in the same manner lay off from BF the angle  $2\beta$ , and from  $a_1$  set off the same distance  $d$ , and the point where it cuts the axis of the instrument produced will be the second point  $a_2$ ; and generally by

\* Euclid, Book III., prop. 20.

laying off the angle  $n\beta$ , and setting off from the preceding point  $a_m$  the distance  $d$ , the point  $a_n$  will be given.

As an example of the application of this method, let  $r$  equal 19 chains, or 1254 feet, and  $d$  equal 100 feet; then from formula VII. we obtain

$$\begin{aligned}\text{Log } d &= 2.000000 = \log 100 \\ \text{Log rad} &= 10.000000 \\ &12.000000 \\ \text{Log } 2r &= 3.399328 = \log 2508 \\ \text{Log sin } \beta &= 8.600672 \therefore \beta 2^\circ 17' 6'';\end{aligned}$$

then having placed the theodolite at the point B, lay off this angle  $2^\circ 17' 6''$  from the line B F, and upon the line B  $a_1$  thus obtained set off 100 feet, which will give the first point in the curve  $a_1$ ; then with an angle of  $4^\circ 34' 12''$  or  $2\beta$  set off another 100 feet from  $a_1$ , which will give the second point  $a_2$ , and thus proceed until the whole extent of the curve has been set out.

Having now pointed out several methods of procedure, in setting out the curved portions of a line of Railway, and having stated generally their relative advantages and disadvantages, we must leave it to the person using them to determine, from the circumstances attending any particular instance, which of these methods it would be preferable to employ. It may perhaps be necessary to add, that in passing from a curve of greater to one of less radius, and *vice versâ*, or in passing at once from a line curving in one direction to a line curving in the contrary direction, or a curve of contrary flexure, nothing more is requisite than to set off the

tangent line to the curve at the point where the alteration occurs, and then to work from that line as from the line A B in any of the methods given above. .

In conclusion, we would urge that too much care cannot be employed in the operations described above, much of the durability of the permanent way, freedom from jerks and uneasy motion, and also safety in travelling upon lines of Railway, depending upon the accuracy with which the rails are laid, the more especially on the curved portions of the line.

THE FIELD PRACTICE  
OF  
LAYING OUT CIRCULAR CURVES  
FOR  
RAILROADS.

By JOHN C. TRAUTWINE,  
CIVIL ENGINEER.



## PREFACE.

THIS little volume has been prepared almost entirely with reference to the wants of young men who desire to qualify themselves for field service in an Engineer Corps. On that account the plainest language has been used to render the subject intelligible,—dispensing with mathematical brevity.

The Table of Natural Sines and Tangents to single minutes, in a form sufficiently portable for field use, will supply a want which is frequently experienced, not only in the operation of laying out curves, but on many other occasions.

One object in preparing it, was to furnish the profession with a Table that should be not only portable, but *absolutely reliable*. Those whose occupations compel them to resort to the Tables in common use, must have frequently experienced the embarrassment which attends the inaccuracies to which they are all subject. So long as a Table is known to contain a single error, the position of which is not ascertained, its employment is attended with doubt in every instance in which we are obliged to refer to it.

As Hutton's Tables of Natural Sines and Tangents are those most in use among the profession, it will be desirable to those persons who possess them, to be able to correct the following errors, which I detected in comparing them.

*In Hutton's Tables, Fifth Edition, 1811.*

Sine of  $6^{\circ} 8'$ , for  $\cdot 1063425$ , read  $\cdot 1068425$ .

Page 328, at top, for 25 Deg., read 40 Deg.

Tangent of  $44^{\circ} 60'$ , for  $\cdot 1000000$ , read  $1\cdot 000000$ .

Tangent of  $41^{\circ} 60'$ , for  $\cdot 8994040$ , read  $\cdot 9004040$ .



*In Dr. Gregory's corrected Edition (the 8th) of Hutton's Tables, 1838.*

Sine of  $49^{\circ} 14'$ , for  $\cdot 7576751$ , read  $\cdot 7578751$ .

*In Hassler's Tables, 1830.*

Sine of  $78^{\circ} 24'$ , read  $\cdot 9795752$ .

Sine of  $20^{\circ} 60'$ , „  $\cdot 3583679$ .

Sine of  $66^{\circ} 19'$ , „  $\cdot 9157795$ .

Sine of  $56^{\circ} 39'$ , „  $\cdot 8353279$ .

Sine of  $55^{\circ} 20'$ , „  $\cdot 8224751$ .

Sine of  $53^{\circ} 4'$ , „  $\cdot 7993352$ .

Sine of  $48^{\circ} 12'$ , „  $\cdot 7454760$ .

Sine of  $45^{\circ} 3'$ , „  $\cdot 7077236$ .

The discrepancies of 1 in the 7th decimal, are not considered as errors, as they are occasioned by a neglect of the value of the 8th decimal. For calculating curves, it is not necessary to use more than 4 decimals.

It is scarcely necessary to remark that, beyond  $44^{\circ}$ , the Sines, Tangents, &c. are read *upwards*, from the bottom of the page, using the corresponding column of minutes. To find the sine of an angle exceeding  $90^{\circ}$ , subtract the angle from  $180^{\circ}$ , and take out the sine of the remainder—because the sine of an angle, and that of what it wants of  $180^{\circ}$ , are the same.

J. C. T.

# THE FIELD PRACTICE OF LAYING OUT CIRCULAR CURVES FOR RAILROADS.

## ARTICLE I.

### PRINCIPLES OF LAYING OUT CURVES.

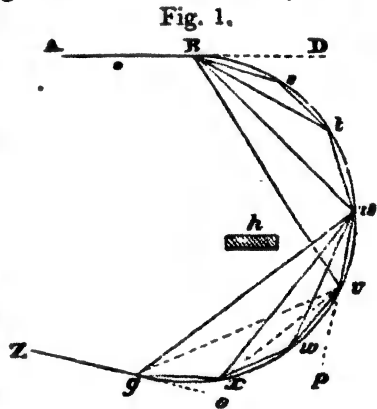
#### METHOD 1.

*To lay out a Curve by means of Tangential Angles.*

If from any point B, fig. 1, in a straight line A D, we lay off a number of equal angles, as D B s, s B t, t B u, u B v, &c., and at the same time make the chords B s, s t, t u, u v, &c., equal to each other, then the points B, s, t, u, v, &c., will be situated in the circumference of a circle, which is tangential to the line A D at the point B.

The first of these angles, D B s, is called the *tangential angle*, as being that by which the curve is connected with the tangent A D; but inasmuch as the others are all equal to it, they also are called *tangential angles*.

If any obstacle, as h, should prevent our seeing from B farther than to v, the curve may be continued by removing the instrument to u, the point preceding v: thence sighting first on v, continue to lay off additional tangential angles v u w, w u x, &c., as before. Or else, moving the instrument to v itself instead of to u, sight back to u, and lay off first the exterior angle p v w, equal to *double* the tangential angle, and afterwards continue the tangential angles w v x, x v g, &c., as before, to the end of the curve.



Finally, in order to pass from the end of the curve at  $g$ , on to a tangent  $g z$ , place the instrument at  $g$ , and sighting back to  $a$ , lay off the tangential angle  $a g o$ ; then  $o g$  continued towards  $Z$  will be the required tangent. (See Art. IV.)

For the tangential angles corresponding to different radii, and chords of 100 feet, see page 160.

## ARTICLE II.

### METHOD 2.

*To lay out a Curve by means of Deflection Angles.*

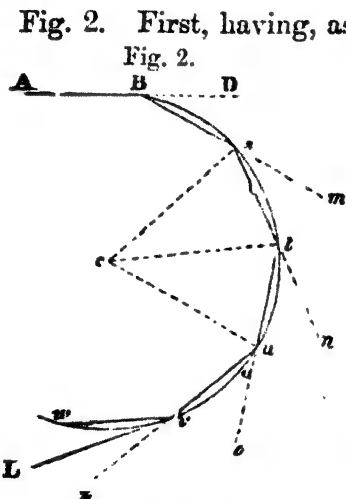


Fig. 2. First, having, as in method 1, laid off a tangential angle  $D B s$ , and measured the chord  $B s$ , remove the instrument to the end  $s$  of the chord, and make the exterior angle  $m s t$  equal to *twice* the tangential angle, and measure the chord  $s t$ ; and so on at the other points  $t u v$ , &c., making each of the exterior angles  $n t u$ ,  $o u v$ , equal to twice the tangential angle, and all the chords equal; then will the points  $B, s, t, u, v$ , &c., be in the circumference of a circle which is tangential to the line  $A D$  at the point  $B$ , as by the first method.

But if, at any of these points, as  $v$ , we wish to pass off to a tangent  $v L$ , employ at that point the *tangential* angle  $z v L$ , equal to half the deflection angle  $z v w$ . (See Art. IV.)

These exterior angles, included between any *chord* and the extension of the preceding *chord*, are called *deflection angles*, or *angles of deflection*, or *angles of curvature*. In any given circle, the angle of deflection is always precisely double the tangential angle, supposing the chords to be equal. At page 160, we give tables of the angles corresponding to circles of different radii, embracing the limits of railroad practice; and calculated for chords 100 feet in length, that being the usual length for a measuring chain on public works.

N.B. The deflection angle of any curve is equal to the angle  $t c u$ , or  $t c s$ , &c., at the centre of the circle, subtended by one of the equal chords  $t u$  or  $t s$ . This angle at the centre, so subtended, is called the *central angle*. The tangential angle, being always half the deflection angle, is, of course, always half the central angle.

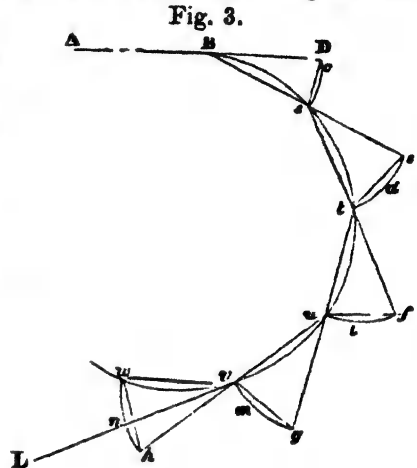
## ARTICLE III.

## METHOD 3.

*To lay out a Curve by Eye.*

The *deflection angles*, fig. 3,  $est$ ,  $ftu$ ,  $guv$ ,  $hvw$ , &c., being double the *tangential angle*  $DBs$ , the *arcs*  $edt$ ,  $fiu$ ,  $gm v$ ,  $hnw$ , &c., are double the arc  $Dcs$ , since the arcs of circles are proportionate to the angles which they subtend; but the *chords*  $et$ ,  $fu$ ,  $gv$ ,  $hw$ , &c., are *not* double the chord  $Ds$ , since the chords of arcs are not proportionate to the arcs or to the angles which they subtend.

The chords  $et$ ,  $fu$ ,  $gv$ ,  $hw$ , &c., which subtend the deflection angles, are called *deflection distances*; and the chord  $Ds$ , which subtends the tan-



gential angle, is called the *tangential distance*. But although, in any given circle, the deflection distance is not truly twice the tangential distance, yet the difference is so trifling in large railroad curves, with chords of but 100 feet, that it may generally be neglected in curves of more than 300 feet radius.

In our tables the *precise* length of both will be found for different radii, and for chords of 100 feet.

Having these respective distances, we may frequently trace a curve on the ground by the eye only, with very tolerable accuracy, sufficient for guiding the excavations and embankments, especially on nearly level ground. Suppose, for instance, it be required to lay out in this manner a curve of 5730 feet radius.

First, find by the table, page 160, or by Art. XVI., the deflection distance  $et$  or  $fu$ , &c., corresponding to a radius of 5730 feet for a chord of 100 feet—viz., 1.745 foot; and also the tangential distance  $Ds$ , .873 of a foot.

Then from the starting-point B, and in line with AB, measure BD, equal 100 feet, and put a pin at D. Also from B, measure the chord Bs, equal 100 feet; at the same time measuring with a graduated rod, from the pin D, the *tangential distance*  $Ds$ , equal to .873 of a foot; and place a stake at s. The pin at D may then be removed.

Next, make  $se$  equal to 100 feet, placing a pin at e, precisely in line with sB; also from s measure  $st$ , equal 100 feet; at the same time measuring with the rod, from the pin e, the *deflection distance*  $et$ ,





Finally, to find the sub-tangential distance  $s n$ , by means of which to pass from  $e$  to the tangent  $e m$ , say, As 10000 is to the square of the sub-chord in feet, so is the *tangential* distance for 100 feet chord to  $s n$ . In this instance, we have, As 10000 is to 625, so is 15.69 feet to .980 foot, or  $s n$ .

## ARTICLE V.

### *Ordinates for Entire Chords.*

It would be both tedious, and liable to inaccuracy, to attempt to fix all the necessary points in railroad curves by the foregoing means, which are employed only for entire chords, or for such sub-chords as may be required at the ends of curves.

The best method is to stretch a piece of twine,  $a b$ , fig. 6, 100 feet

Fig. 6.

long, between two adjacent chord-stakes, and measure off as nearly as may be at right angles to it, with a graduated rod, the previously calculated or-

dinates,  $c d$ ,  $e f$ ,  $g h$ , &c., placing pegs at  $d$ ,  $f$ ,  $h$ , &c.\* Our table of ordinates, page 162, is calculated for distances apart,  $b c$ ,  $c e$ ,  $e g$ , &c., of 5 feet; and for all curves likely to occur in practice. The 5 feet distances on the twine should be marked by knots or otherwise; and those at the centre, and half way between it and the ends, be further distinguished by tying on pieces of tape.

The 5 feet distances are only used (after the excavations and embankments are finished) for placing pegs to guide the laying of the rails, and then only for very sudden curves; for those of large radii, distances of 10 feet are sufficiently near, or even 25 feet for very easy curves. For guiding the curves of the cuttings and fillings, it is not necessary to place the stakes nearer than 50 feet apart; unless for those of less than about 1000 feet radius, when they may be placed 25 feet apart. Ordinates for radii intermediate to those in the table, may either be calculated by the rules given further on, or they may be taken proportionally intermediate of the tabular ones, with sufficient accuracy for practice.

### *Ordinates for Sub-Chords.*

These may readily be calculated *approximately enough for railroad practice*, for curves of over 300 feet radius, and for chords not exceeding 100 feet, thus: In a circle of given radius, not less than about 300 feet, the ordinates of an entire 100 feet chord may be

\* On the tops of these stakes small tacks are driven, to define the precise point in the curve.

assumed to be to those of a sub-chord as the square of the chord is to the square of the sub-chord.

In all our tables the chord is supposed to be 100 feet, the square of which is 10000; the rule therefore becomes, As 10000 feet : square of sub-chord in feet :: Ord. of Chord : Ord. of Sub-Chord *approximately*.

*Example.*—In a curve of 5730 feet radius, the middle ordinate of a 100 feet chord is .218 of a foot; what will be the length of the middle ordinate of a sub-chord of 50 feet? Here,

Sq. of 100 ft. :	Sq. of 50 ft. ::	mid. Ord. of Chord. :	mid. Ord. Sub-Chord <i>approximately</i> .
10000 :	2500 ::	.218 ft. :	.0545 ft.

And so of any other ordinate, always supposing the chord and sub-chord to be divided into the *same number of parts*.

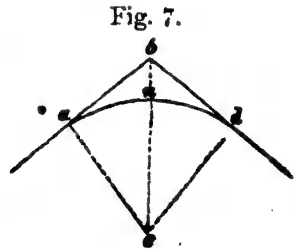
## ARTICLE VI.

*Having given the angle a b d, fig. 7, it is required to find the point a or d, at which to commence a curve of given radius.*

*Rule.*—Subtract half the angle a b d from  $90^\circ$ ; the remainder will be the angle b c a or b c d. From the table of tangents take the natural tangent of b c a, and multiply it by the given radius; the product will be b a or b d.

*Example.*—Let the angle a b d be  $120^\circ$ , how far from b must we begin, at a or d, to lay out a curve, a n d, of 2865 feet radius?

Here, half of the angle a b d =  $60^\circ$ , which taken from  $90^\circ$  leaves the angle b c a =  $30^\circ$ . The natural tangent of  $30^\circ$  = .5773, which, multiplied by the radius of 2865 feet, gives 1653.96 feet for b a or b d. (See Art. XII.)



## ARTICLE VII.

*Having given the angle a b d, fig. 7, and the distance from b to a or d, at one of which we wish to commence a curve, it is required to find what radius, c a or c d, the curve must have, in order to unite with b a and b d tangentially at a and d.*

*Rule.*—Subtract the angle a b c, which is half the angle a b d, from  $90^\circ$ ; the remainder will be the angle b c a or b c d. Then as natural sine of b c a\* is to natural sine of a b c,† so is a b to a c, the radius required.

\* The angle opposite the given side, a b.

† The angle opposite the required side, a c.



*Example.*—Let the angle  $a b d$  be  $120^\circ$ , and the distance  $b a$  or  $b d$  1654 feet; what will be the radius  $a c$  or  $c d$  of a circle that shall touch  $a$  and  $d$  tangentially?

Here, the angle  $a b c =$  half the angle  $a b d$ , is  $60^\circ$ , which, taken from  $90^\circ$ , leaves the angle  $b c a$  or  $b c d = 30^\circ$ . Then, as the natural sine of  $b c a$  ( $30^\circ$ ) = .5000 is to natural sine of  $a b c$  ( $60^\circ$ ) = .8660, so is  $b a$  (1654 feet) to  $a c$  (2865 feet) the radius required.

### ARTICLE VIII.

*Having given the radius  $a c$ , fig. 7, of a curve, and the angle  $a b d$ , it is required to find the number of chords of 100 feet that will constitute the curve.*

*Rule.*—Subtract the angle  $a b d$  from  $180^\circ$ , and divide the remainder by the angle of curvature, or deflection of the curve. The quotient will be the required number of chords.

*Example.*—Let the angle  $a b d$  be  $120^\circ$ , and the radius  $a c$  2865 feet.

Here, the angle  $a b d$ ,  $120^\circ$ , subtracted from  $180^\circ$ , leaves a remainder of  $60^\circ$ ; which, divided by  $2^\circ$ , the angle of deflection for a curve of 2865 feet, gives a quotient of 30; which is the required number of chords of 100 feet.

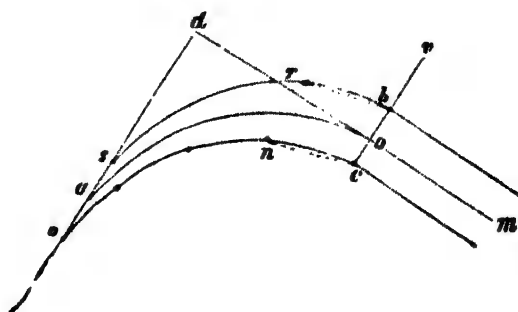
N.B.—Had the quotient contained a *fraction* of a chord, it would have indicated that we should have had to employ a sub-chord at the end of the curve; for instance, had the number of chords been  $30\frac{1}{2}$ , a sub-chord of 50 feet (very approximately) would have been necessary.

### ARTICLE IX.

*How to proceed when the end of a curve does not correctly join the tangent.*

We sometimes find, in running out a curve for the number of chords determined by the Rule in the preceding Article, that, instead of uniting as it should with the previously determined tan-

Fig. 8.



gent  $d m$ , fig. 8, at  $o$ , it ends tangentially to a line *parallel* to said tangent, either *within* it, as at  $c$ ; or *beyond* it, as at  $b$ . Being first certain that no error has occurred in tracing out the curve, ascertain with the compass the bearing of the tangent  $a d$ , and, removing the compass to the end of the curve at  $c$  or  $b$  (as the case may be), run the line  $b o$  or  $c o$ , in the same course as  $a d$ , until it strikes the tangent  $d o m$ ; which may be ascertained by ranging two stakes placed on the tangent.

Then measure  $b o$  or  $c o$  (as the case may be), and if the curve fall *within* the tangent  $o m$ , as at  $c$ , measure *forwards* from  $t$  towards  $d$  the distance  $t a$ , equal to  $c o$ ; or if the curve fall *beyond* the tangent, as at  $b$ , measure *backwards* from  $s$  the distance  $s a$ , equal to  $b o$ . Then the curve retraced from  $a$  will terminate tangentially in  $d m$  at  $o$ .

N.B.—The direction of  $c o$  or  $b o$  may be ascertained without a compass, and better, thus: Multiply the *tangential* angle of the curve by *twice* the number of chords run, *less one*; subtract the product from  $180^\circ$ , and sighting back one chord to  $n$  or  $r$ , lay off the angle  $n c b$  or  $r b v$ , equal to the remainder. For example, if the tangential angle be  $10^\circ$ , and from  $t$  to  $c$  be 4 chords, then 7 times  $10^\circ$  taken from  $180^\circ$  leaves the angle  $n c b$  or  $r b v = 110^\circ$ . When the product exceeds  $180^\circ$ , it must be subtracted from  $360^\circ$  for the angle  $n c b$  or  $r b v$ .\*

This case occurs whenever an error has been made in measuring the distance from  $d$  to  $a$ . If  $d a$  be made too short, the curve  $s b$  is the result; and if too long, the curve  $t c$ .

If the error is small, it may be divided equally among the chords by measure without retracing the curve with an instrument. This method may be employed with perfect security so long as the error does not exceed 1 foot to every chord of 100 feet; and it will never be so great if moderate care be taken.

Thus, if the curve be 20 chords long, and the error 20 feet, the last stake may be moved 20 feet, the next 19, the next 18, &c., as nearly at right angles to the curve as can be judged by the eye.

The same ordinates that would have been used had the curve been correct will answer for the one so adjusted, without perceptible difference. For other cases, see Art. X.

## ARTICLE X.

Again, it may happen that the error is not caused by a mismeasurement of the distance  $a c$ , figs. 9 and 10, as in the last case; but by mistake in obtaining the angle  $a e f$ .

\* In both cases the angle is measured *outwardly* from the curve; but when the curve falls *beyond* the tangent, as at  $b$ , then  $b v$  must be continued *inwardly*, as  $b o$ .

If  $a e f$ , fig. 9, be measured in excess, as  $a e g$ , then the curve  $a b c$ ,

Fig. 9.

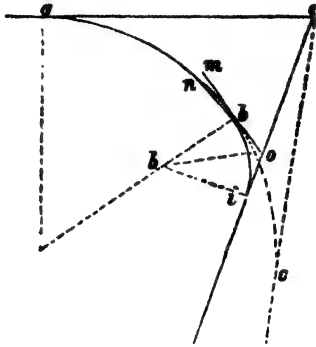
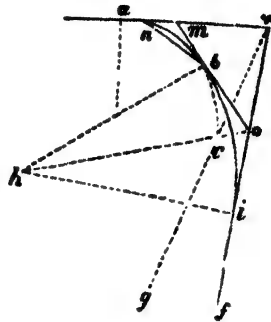


Fig. 10.



calculated for the incorrect angle  $a e g$ , will be found to fall *beyond* the true tangent  $e f$ , as at  $c$ ; and the tangents  $e g$  and  $e f$  not being parallel, the curve cannot be adjusted by either of the methods given in the preceding Article, unless the error be within about 1 foot to each 100 feet length of curve; in which case (supposing no other error to exist), either of those methods may be employed with sufficient accuracy for practice.

Also, if  $a e f$ , fig. 10, be measured too small, as  $a e g$ , then the curve  $a b c$ , calculated for the incorrect angle  $a e g$ , will be found to fall *within* the true tangent  $e f$ , as at  $c$ ; when so, the remarks contained in the preceding sentence are equally applicable here. If the error be within 1 foot to 100 feet length of curve, it may be equally divided among the chords. But if greater, we must either remeasure the angle  $a e f$  correctly, and go over the whole work again, or resort to some other mode of obviating the difficulty. The angle  $a e f$  may be difficult of access; or the curve may be so long that to retrace it would be a work of much labour. We may then adopt the method of *compound curves* (see Art. XIII.), by which much trouble will be avoided, and a considerable portion of the first part of the curve be allowed to remain as it is.

Thus, whether the curve  $a b c$  fall beyond the true tangent  $e f$ , as in fig. 9, or inside of it, as in fig. 10, place the instrument at  $b$ , figs. 9 and 10 (the point at which the change of radius is to take place), and sighting back one chord to  $n$ , lay off the tangential angle  $n b m$  of the curve  $a b c$ , and observe where the tangent  $m b$  continued strikes  $e f$ , as at  $o$ . Measure both  $b o$ , and the angle  $b o f$ . Half the angle  $b o f$  from  $90^\circ$  gives the angle  $b h o$ ; then say,



Then as

$$\left. \begin{array}{l} \text{Sum of Tangts.} \\ 1.8591 \end{array} \right\} \text{ is to } \left\{ \begin{array}{l} \text{Tang. of} \\ 54^\circ 10' \\ 1.3848, \end{array} \right\} \text{ so is } \left\{ \begin{array}{l} b c \\ 950 \text{ feet} \end{array} \right\} \text{ to } \left\{ \begin{array}{l} b i \\ 707.63 \text{ feet,} \end{array} \right.$$

and  $b i$  707.63 feet, taken from  $b c$  950 feet, leaves  $i c$  242.37 feet.

Again, as the

$$\left. \begin{array}{l} \text{Nat. Sine of} \\ \text{angle } b g i \\ .8107 \end{array} \right\} \text{ is to } \left\{ \begin{array}{l} \text{Nat. Sine of} \\ \text{angle } g b i \\ .5854, \end{array} \right\} \text{ so is } \left\{ \begin{array}{l} b i \\ 707.63 \\ \text{feet} \end{array} \right\} \text{ to } \left\{ \begin{array}{l} g \text{ or } h i, \text{ the} \\ \text{required radius,} \\ 510.97 \text{ feet.} \end{array} \right.$$

## ARTICLE XII.

To obtain the angle  $d b e$ , formed by two tangents,  $d b$  and  $b e$ , when the point  $b$  is inaccessible. Figs. 12, 13, 14, and 15.

This is of frequent occurrence.

CASE 1. When the included figure, fig. 12, has but *three* sides.

*Rule.*—Subtract the angle  $a d e$  from  $180^\circ$  for the angle  $b d e$ ; and subtract the angle  $d e c$  from  $180^\circ$  for the angle  $d e b$ . Add together  $b d e$  and  $d e b$ , and subtract their sum from  $180^\circ$  for the angle  $d b e$ . Or, more briefly, subtract  $180^\circ$  from the sum of  $a d e$  and  $d e c$ .

Fig. 12.

Fig. 13.

Fig. 14.

Fig. 15.



CASE 2. When the included figure  $d b e f$  (13, 14) has *four* sides.

*Rule.*—Subtract the sum of the three *internal* angles of the figure marked by dotted segments of circle, from  $360^\circ$  for the angle  $d b e$ .

CASE 3. When the included figure, fig. 15, has *more than four* sides.

*Rule.*—Add together all the *internal* angles, marked by dotted segments of circles; and subtract their sum from twice as many right angles as the figure has sides, less four, for the angle  $d b e$ .

*Example.*—Let the angles denoted by the dotted segments at the different letters be as follows: That at  $d$ ,  $70^\circ$ ; at  $o$ ,  $220^\circ$ ; at  $i$ ,  $150^\circ$ ; at  $s$ ,  $110^\circ$ ; at  $c$ ,  $160^\circ$ ; at  $e$ ,  $100^\circ$ . The sum of these is  $810^\circ$ . The figure has 7 sides; and twice 7 less 4 = 10; and 10 right angles =  $900^\circ$ ; from which the sum of the designated internal angles ( $810^\circ$ ) being subtracted, leaves  $90^\circ$  for the angle  $d b e$ .

N.B.—When the angle  $d b e$  has to be deduced from a figure of many sides, as fig. 15, the errors spoken of in Articles IX. and X. are apt to occur, unless the several sides and the angles  $o i s$ , &c., be measured with much care. For tracing curves with any accuracy and satisfaction, the instrument should be divided at least into minutes;

as before remarked, the transit instrument is the best for the purpose. With moderate care in the preparatory measurement of the sides and angles, errors will seldom occur that may not be adjusted with all the accuracy required in practice, by the very simple method of dividing them equally among the chords, as explained in Articles IX. and X.

### ARTICLE XIII.

*To pass from one curve,  $a m b$ , fig. 16, to another,  $b n c$ , of different radius, but running in the same direction, constituting a COMPOUND curve.*

*Rule.*—Placing the instrument at  $b$ , sight back to the other end of the 100 feet chord at  $a$ ; and lay off the tangential angle  $a b d$  of the curve  $a m b$ ; then from the common tangent  $d b e$  lay off the tangential angle  $e b c$  of the curve  $b n c$ , making at the same time the chord  $b c$ , equal to 100 feet.

N.B.—If running the curve by eye, use the tangential distances instead of the angles.

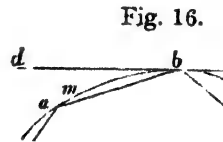


Fig. 16.

### ARTICLE XIV.

*To pass from one curve,  $m n t$ , fig. 17, to another,  $t i o$ , of either the same or of a different radius, but running in an opposite direction, constituting a REVERSE curve.*

*Rule.*—Placing the instrument at  $t$ , sight back to the other end of the 100 feet chord at  $m$ , and lay off the tangential angle  $m t r$  of the curve  $m n t$ ; then from the common tangent  $r t s$  lay off the tangential angle  $s t o$  of the curve  $t i o$ , making at the same time the chord  $t o$ , equal to 100 feet.



Fig. 17.

N.B.—If running the curve by eye, use the tangential distances instead of the angles.

### ARTICLE XV.

#### RADII.

*To find the radius corresponding to any given angle of deflection, and to equal chords of any given length.*

*Rule 1.*—Subtract the angle of deflection from  $180^\circ$ , then say, As natural sine of angle of deflection is to natural sine of half the remainder, so is the given chord to the radius required.

*Example.*—Let the angle of deflection be  $2^\circ$ , and the chord 100 feet; required the radius.

Here,  $2^\circ$  subtracted from  $180^\circ$  leaves  $178^\circ$ , the half of which is  $89^\circ$ ; and as

$$\begin{array}{ccccccc} \text{Nat. Sine of } 2^\circ & : & \text{Nat Sine of } 89^\circ & :: & \text{Chord} & : & \text{Radius} \\ \cdot 034899 & & \cdot 999848 & & 100 \text{ feet} & & 2865 \text{ feet.} \end{array}$$

*Rule 2.*—The radius for 100 feet chords may be found *approximately*, by dividing 5730 by the deflection angle. This rule is very close for radii of not less than 500 feet. For 500 feet it gives eight-tenths of a foot too little, but is more approximate for larger radii.

*Example.*—What is the radius to a deflection angle of  $2^\circ$ , the chords being 100 feet long?

Here, 5730 divided by 2 gives 2865 feet, the radius required.

## ARTICLE XVI.

### TANGENTIAL AND DEFLECTION ANGLES.

*To find either the Tangential or Deflection Angle corresponding to any given radius, and to equal chords of any given length.*

*Rule 1.*—Divide *half* the chord by the radius; the quotient will be the natural sine of the *tangential* angle. Therefore the angle corresponding to this sine, in the Table of Natural Sines, will be the tangential angle required; and the tangential angle multiplied by 2 will give the deflection angle.

*Example.*—Let the radius be 2865 feet, and the chord 100 feet; what will be the tangential and deflection angles?

Here, half the chord (50 feet), divided by the radius (2865 feet), gives  $\cdot 01745$ ; and the tangential angle in the Table corresponding to the natural sine  $\cdot 01745$  is  $1^\circ$ , twice which is  $2^\circ$ , the deflection angle required.

*Rule 2.*—The deflection angle for 100 feet chords may be found approximately by dividing 5730 by the radius. This is very close for curves of over 500 feet radius. For 500 feet it gives about one minute too little.

*Example.*—What is the deflection angle for a radius of 2865 feet, the chords being 100 each?

Here, 5730 divided by the radius 2865 gives  $2^\circ$ , the deflection angle required.

## ARTICLE XVII.

## DEFLECTION DISTANCES.

*To find the Deflection Distance (exactly) for any given radius, when the chords are 100 feet long.*

**Rule.**—Divide the constant number 10000 by the radius in feet; the quotient will be the deflection angle required.\*

**Example.**—What is the deflection distance to a radius of 5730 feet, the chords being 100 feet long?

Here, 10000 divided by 5730 radius gives 1.745 foot, the deflection distance required.

*To find the Deflection Distance for any given radius, and for equal chords of any given length.*

**Rule.**—Divide half the given chord by radius, the quotient will be the natural sine of one-half the deflection angle; and double this natural sine, multiplied by the chord, will give the deflection distance required. By this rule our Table was prepared.

**Example.**—As before, what is the deflection distance to a radius of 5730 feet, the chords being 100 feet long?

Here, half the chord (50 feet), divided by radius (5730 feet), gives .008727, which is the natural sine of half the deflection angle. Now .008727, multiplied by 2, gives .017454, which, multiplied by the chord (100 feet), gives 1.745 foot, the required deflection distance, the same as in the preceding example.

## ARTICLE XVIII.

## TANGENTIAL DISTANCES.

*To find the Tangential Distance corresponding to any given radius, and to equal chords of any given length.*

**Rule.**—First find the tangential angle by Art. XVI., and take from the Table of Natural Sines that corresponding to one-half of the tangential angle. Then multiply double this sine by the given chord for the tangential distance. By this rule our Table was prepared.

\* Because the deflection distance to a radius of 10000 feet, with chords of 100 feet, is 1 foot; and the deflection distances for other radii increase *inversely* as the radii.



*Example.*—Let the radius be 2865 feet, and the chords 100 feet each; what will be the tangential distance?

Here we find, by Art. XVI., the tangential angle  $1^\circ$  for a radius of 2865 feet.

The natural sine corresponding to 30 minutes, or one-half of this tangential angle, is, by the Table of Sines, .008727; the double of which is .017454, which, multiplied by the chord, or 100 feet, gives 1.745 foot for the tangential distance required.

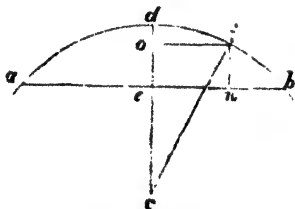
## ARTICLE XIX.

### ORDINATES.

*To find the Middle Ordinate to any given radius, and to any given chord.*

*Rule 1.*—From the square of the radius subtract the square of half the chord; and take the square root of the remainder from the radius, for the middle ordinate.

Fig. 18.



*Example.*—What is the length of the middle ordinate *d e*, fig. 18, the radius *c a* being 819 feet, and the chord *a b* 100 feet?

Here, the square of *c a* (819) is 670761, and the square of *a c* (50) is 2500; which, being subtracted from the former, leaves 668261, the square root of which is *e c*, 817.472; which, taken from the radius 819, leaves 1.528 foot, the required middle ordinate, *d e*.

*Rule 2.*—Subtract the tabular cosine of the tangential angle from 1, and multiply the remainder by the radius.

*Example.*—Same as foregoing; namely, radius 819 feet, angle of deflection  $7^\circ$ , to chords of 100 feet. What will be the length of the middle ordinate?

Here, tabular cosine of  $3\frac{1}{2}^\circ$  (the tangential angle) is .998135; which, subtracted from 1, leaves .001865; which, multiplied by 819, the radius, gives 1.527, the middle ordinate required.

## ARTICLE XX.

*Having given the Middle Ordinate d e, fig. 18, it is required to find any other one, as i n.*

*Rule 1.*—Subtract the middle ordinate *d e* from the radius *d c*, the remainder will be *e c*: then from the square of the radius *c i* subtract the square of the distance *o i*, which the required ordinate *i n* is from the middle ordinate *d e*, and extract the square root of

the remainder. This square root will be  $o c$ . From this square root  $o c$  subtract  $e c$ ; the remainder will be  $o e$ , which is equal to  $i n$ , the required ordinate.

*Example.*—The middle ordinate  $d e$ , of a 100 feet chord  $b a$ , to a radius of 819, being 1.528 foot, it is required to find the length of the ordinate  $i n$ , 20 feet from the middle one.

Here, the middle ordinate  $d e$ , 1.528, subtracted from the radius 819, leaves  $e c$ , 817.472. The square of the radius is 670761; and the square of 20 (the distance of the required ordinate from the middle one) is 400; which, taken from 670761, leaves 670361; the square root of which is 818.756, or  $o c$ ; from which take  $e c$ , or 817.472, and the remainder, 1.284, will be  $o e$ , which is equal to  $i n$ , the required ordinate.

*Rule 2.*—Multiply the ordinates of a  $1^\circ$  curve by the deflection angle of the curve whose ordinates are required (chords being 100 feet). This is a sufficiently close approximation for curves of not less than 500 feet radius; and for placing ordinates *for guiding the excavations and embankments*, it is close enough for the smallest curves in our Table.

## I.—TABLE OF RADII, &amp;c.—Chord 100 feet.

*The Tangential Angle is always one-half of the Angle of Deflection.*

Angle of Deflection.	Radius in feet.	Deflection distance in feet.	Tangential distance in feet.	Angle of Deflection.	Radius in feet.	Deflection distance in feet.	Tangential distance in feet.
• 1	343800	•029	•014	° 51	6741	1•482	•741
2	171900	•058	•029	52	6611	1•511	•755
3	114600	•087	•043	53	6487	1•540	•770
4	85950	•116	•058	54	6367	1•569	•784
5	68760	•145	•072	55	6251	1•598	•799
6	57300	•174	•087	56	6139	1•627	•813
7	49116	•203	•101	57	6032	1•656	•828
8	42975	•232	•116	58	5928	1•685	•842
9	38200	•262	•131	59	5827	1•715	•857
10	34380	•291	•145	1 0	5730	1•745	•872
11	31256	•320	•160	2	5545	1•802	•901
12	28650	•349	•174	4	5372	1•860	•930
13	26446	•378	•189	6	5209	1•918	•959
14	24558	•407	•203	8	5056	1•976	•988
15	22920	•436	•218	10	4912	2•036	1•018
16	21487	•465	•232	12	4775	2•094	1•047
17	20224	•494	•247	14	4646	2•152	1•076
18	19100	•523	•261	16	4524	2•210	1•105
19	18094	•552	•276	18	4408	2•268	1•131
20	17190	•581	•290	20	4298	2•326	1•163
21	16372	•610	•305	22	4193	2•384	1•192
22	15628	•639	•319	24	4093	2•443	1•221
23	14948	•668	•334	26	3998	2•501	1•250
24	14325	•697	•348	28	3907	2•559	1•279
25	13752	•727	•363	30	3820	2•617	1•308
26	13223	•756	•378	32	3737	2•676	1•338
27	12733	•785	•392	34	3657	2•734	1•367
28	12279	•814	•407	36	3581	2•793	1•396
29	11856	•843	•421	38	3508	2•851	1•425
30	11460	•872	•436	40	3438	2•908	1•454
31	11090	•900	•450	42	3370	2•967	1•483
32	10744	•930	•465	44	3306	3•025	1•512
33	10419	•959	•479	46	3243	3•083	1•541
34	10112	•988	•494	48	3183	3•141	1•570
35	9823	1•017	•508	50	3126	3•199	1•599
36	9550	1•046	•523	52	3069	3•258	1•629
37	9292	1•075	•537	54	3016	3•316	1•658
38	9047	1•104	•552	56	2964	3•374	1•687
39	8815	1•133	•566	58	2914	3•432	1•716
40	8595	1•162	•581	2 0	2865	3•490	1•745
41	8385	1•191	•595	2	2818	3•548	1•774
42	8186	1•221	•610	4	2772	3•606	1•803
43	7995	1•250	•625	6	2729	3•665	1•832
44	7814	1•279	•639	8	2686	3•723	1•861
45	7640	1•308	•654	10	2644	3•781	1•890
46	7474	1•337	•668	12	2601	3•839	1•919
47	7315	1•366	•683	14	2566	3•897	1•948
48	7162	1•395	•697	16	2528	3•956	1•978
49	7016	1•424	•712	18	2491	4•014	2•007
50	6876	1•453	•726	20	2456	4•072	2•036

I.—TABLE OF RADII, &amp;c.—continued.

Angle of Deflection.	Radius in feet.	Deflection distance in feet.	Tangential distance in feet.	Angle of Deflection.	Radius in feet.	Deflection distance in feet.	Tangential distance in feet.
2 22	2421	4·130	2·065	4 15	1348	7·416	3·708
24	2387	4·188	2·094	20	1322	7·563	3·781
26	2355	4·246	2·123	25	1298	7·708	3·854
28	2323	4·305	2·152	30	1274	7·853	3·927
30	2292	4·363	2·182	35	1251	7·998	3·999
32	2262	4·421	2·210	40	1228	8·143	4·071
34	2232	4·479	2·239	45	1207	8·289	4·145
36	2204	4·538	2·269	50	1185	8·432	4·216
38	2176	4·596	2·298	55	1166	8·577	4·288
40	2149	4·653	2·326	5 0	1146	8·722	4·361
42	2122	4·712	2·356	5	1127	8·869	4·434
44	2096	4·770	2·385	10	1109	9·014	4·507
46	2071	4·828	2·414	15	1092	9·159	4·579
48	2046	4·886	2·443	20	1074	9·304	4·652
50	2023	4·944	2·472	25	1058	9·449	4·724
52	1999	5·002	2·501	30	1042	9·595	4·798
54	1976	5·060	2·530	35	1026	9·740	4·870
56	1953	5·118	2·559	40	1011	9·885	4·942
58	1932	5·176	2·588	45	996·8	10·03	5·015
3 0	1910	5·235	2·618	50	982·7	10·18	5·090
2	1889	5·293	2·646	55	969·0	10·32	5·160
4	1868	5·351	2·675	6 0	955·4	10·47	5·235
6	1848	5·409	2·704	5	947·5	10·62	5·310
8	1828	5·468	2·734	10	939·7	10·76	5·380
10	1810	5·526	2·763	15	917·0	10·90	5·450
12	1790	5·584	2·792	20	905·0	11·04	5·520
14	1772	5·642	2·821	25	893·5	11·20	5·600
16	1754	5·700	2·850	30	882·0	11·34	5·670
18	1736	5·758	2·879	35	870·7	11·48	5·740
20	1719	5·817	2·908	40	859·5	11·63	5·815
22	1702	5·875	2·937	45	849·3	11·78	5·890
24	1685	5·933	2·966	50	838·9	11·92	5·960
26	1669	5·992	2·996	55	828·9	12·06	6·030
28	1653	6·050	3·025	7 0	819·0	12·21	6·105
30	1637	6·108	3·054	5	813·3	12·36	6·180
32	1621	6·166	3·083	10	807·4	12·50	6·250
34	1606	6·224	3·112	15	790·8	12·64	6·320
36	1591	6·282	3·141	20	781·9	12·79	6·395
38	1577	6·340	3·170	25	773·2	12·94	6·470
40	1563	6·398	3·199	30	764·5	13·08	6·540
42	1549	6·456	3·228	35	756·1	13·22	6·610
44	1534	6·515	3·257	40	748·0	13·37	6·685
46	1521	6·574	3·287	45	739·9	13·51	6·755
48	1508	6·632	3·316	50	732·0	13·66	6·830
50	1495	6·690	3·345	55	724·3	13·80	6·900
52	1482	6·748	3·374	8 0	716·8	13·95	6·975
54	1469	6·806	3·403	15	695·1	14·38	7·190
56	1457	6·864	3·432	30	674·6	14·81	7·405
58	1445	6·922	3·461	45	655·5	15·25	7·625
4 0	1433	6·980	3·490	9 0	637·3	15·68	7·840
5	1403	7·125	3·562	15	620·2	16·12	8·060
10	1375	7·270	3·635	30	603·8	16·55	8·275

I.—TABLE OF RADII, &c.—*continued*.

Angle of Deflection.	Radius in feet.	Deflection distance in feet.	Tangential distance in feet.	Angle of Deflection.	Radius in feet.	Deflection distance in feet.	Tangential distance in feet.
9 45	588.4	16.99	8.495	17 0	338.3	29.56	14.82
10 0	573.7	17.43	8.715	17 30	328.7	30.43	15.25
10 15	559.7	17.87	8.935	18 0	319.6	21.29	15.69
10 30	546.4	18.30	9.150	18 30	311.0	32.15	16.12
10 45	533.8	18.73	9.365	19 0	302.9	33.01	16.56
11 0	521.7	19.17	9.585	19 30	295.3	33.87	16.99
11 15	510.1	19.61	9.805	20 0	287.9	34.73	17.43
11 30	499.1	20.05	10.03	21 0	274.4	36.44	18.30
11 45	488.5	20.50	10.25	22 0	262.0	38.15	19.17
12 0	478.3	20.94	10.47	23 0	250.8	39.87	20.02
12 15	468.7	21.36	10.69	24 0	240.5	41.58	20.91
12 30	459.3	21.79	10.90	25 0	231.0	43.28	21.77
12 45	450.3	22.21	11.12	26 0	222.3	44.98	22.64
13 0	441.7	22.64	11.34	27 0	214.2	46.68	23.51
13 15	433.4	23.07	11.56	28 0	206.7	48.38	24.37
13 30	425.5	23.51	11.77	29 0	199.7	50.07	25.24
13 45	417.7	23.94	11.99	30 0	193.2	51.76	26.11
14 0	410.3	24.37	12.21	31 0	187.1	53.45	26.97
14 15	403.1	24.81	12.43	32 0	181.4	55.13	27.83
14 30	396.2	25.24	12.65	33 0	176.0	56.80	28.70
14 45	389.6	25.67	12.86	34 0	171.0	58.47	29.56
15 0	383.1	26.11	13.08	35 0	166.3	60.14	30.42
15 15	376.9	26.52	13.30	36 0	161.8	61.80	31.29
15 30	370.8	26.94	13.52	37 0	157.6	63.46	32.15
15 45	365.0	27.37	13.73	38 0	153.6	65.11	33.01
16 0	359.3	27.83	13.95	39 0	149.8	66.76	33.87
16 30	348.4	28.70	14.38	40 0	146.2	68.40	34.73

## II.—TABLE OF ORDINATES.

*Ordinates five feet apart.—Chord 100 feet.*

Distances of the Ordinates from the end of the 100 feet Chord.										
Angle of Deflection.	Middle, 50 feet.	45 feet.	40 feet.	35 feet.	30 feet.	25 feet.	20 feet.	15 feet.	10 feet.	5 feet.
2	.007	.007	.007	.006	.006	.005	.003	.003	.002	.001
4	.014	.014	.014	.013	.012	.010	.008	.008	.005	.003
6	.021	.021	.021	.020	.019	.016	.013	.011	.008	.004
8	.029	.029	.028	.026	.024	.022	.018	.015	.010	.005
10	.036	.036	.035	.033	.031	.027	.023	.019	.013	.007
12	.043	.043	.041	.038	.037	.033	.028	.022	.015	.008
14	.050	.050	.048	.044	.043	.038	.032	.026	.017	.010
16	.058	.058	.056	.052	.049	.044	.037	.030	.020	.011
18	.065	.065	.063	.059	.055	.050	.042	.033	.023	.013
20	.073	.072	.070	.066	.061	.055	.047	.037	.026	.014
22	.080	.079	.076	.071	.067	.060	.051	.041	.029	.015

II.—TABLE OF ORDINATES—*continued.*

Distances of the Ordinates from the end of the 100 feet Chord.										
Angle of Deflection.	Middle, 50 feet.	45 feet.	40 feet.	35 feet.	30 feet.	25 feet.	20 feet.	15 feet.	10 feet.	5 feet.
° 24	·087	·086	·083	·077	·074	·066	·056	·045	·031	·017
26	·094	·093	·090	·084	·080	·071	·060	·048	·034	·018
28	·102	·101	·098	·092	·086	·077	·065	·052	·036	·019
30	·109	·108	·105	·099	·092	·082	·070	·055	·039	·020
32	·116	·115	·112	·106	·098	·088	·075	·058	·042	·022
34	·123	·122	·118	·111	·104	·094	·079	·062	·044	·023
36	·131	·130	·126	·119	·110	·099	·084	·066	·047	·024
38	·138	·137	·133	·126	·116	·105	·089	·070	·049	·025
40	·145	·144	·140	·133	·123	·110	·093	·074	·052	·027
42	·152	·150	·146	·138	·128	·115	·098	·077	·055	·028
44	·160	·158	·153	·145	·135	·121	·103	·081	·057	·030
46	·167	·165	·160	·152	·141	·126	·107	·085	·060	·032
48	·174	·172	·167	·158	·147	·132	·112	·088	·062	·033
50	·182	·180	·175	·166	·153	·138	·117	·092	·065	·034
52	·189	·187	·181	·171	·159	·143	·122	·095	·068	·035
54	·196	·194	·188	·178	·165	·148	·126	·099	·070	·036
56	·204	·202	·195	·185	·171	·154	·131	·103	·073	·038
58	·211	·209	·202	·192	·177	·159	·136	·107	·075	·039
1 0	·218	·216	·209	·198	·183	·164	·140	·111	·078	·041
2	·225	·223	·215	·204	·189	·169	·145	·114	·081	·042
4	·233	·231	·223	·211	·196	·175	·150	·118	·083	·043
6	·240	·238	·230	·217	·202	·180	·155	·121	·086	·045
8	·247	·245	·237	·224	·208	·186	·159	·125	·088	·046
10	·254	·252	·244	·231	·214	·191	·163	·130	·091	·048
12	·262	·260	·252	·237	·220	·196	·168	·133	·094	·049
14	·269	·267	·258	·244	·226	·202	·173	·136	·096	·050
16	·276	·274	·265	·251	·232	·207	·177	·140	·099	·052
18	·284	·282	·273	·257	·238	·213	·182	·144	·101	·053
20	·291	·288	·279	·264	·244	·218	·187	·148	·104	·055
22	·298	·295	·285	·270	·250	·224	·192	·151	·107	·056
24	·306	·303	·293	·277	·256	·229	·197	·155	·109	·057
26	·313	·310	·300	·284	·263	·235	·201	·159	·112	·059
28	·320	·317	·307	·291	·269	·240	·206	·163	·114	·060
30	·327	·324	·314	·297	·275	·246	·210	·167	·117	·062
32	·334	·331	·321	·304	·281	·251	·215	·171	·120	·063
34	·341	·338	·328	·310	·287	·257	·219	·174	·122	·065
36	·349	·345	·335	·317	·293	·262	·224	·178	·125	·066
38	·356	·353	·342	·323	·299	·268	·228	·182	·127	·068
40	·364	·360	·349	·330	·305	·273	·233	·185	·130	·069
42	·371	·367	·356	·337	·312	·278	·238	·189	·133	·070
44	·378	·374	·363	·343	·318	·284	·242	·192	·135	·072
46	·385	·382	·370	·350	·324	·289	·247	·196	·138	·073
48	·393	·389	·377	·356	·330	·295	·251	·200	·141	·075
50	·400	·396	·384	·364	·336	·300	·256	·204	·144	·076
52	·407	·403	·391	·370	·342	·305	·261	·208	·147	·077
54	·414	·410	·398	·376	·348	·311	·265	·211	·149	·079
56	·422	·418	·405	·383	·354	·316	·270	·215	·152	·080
58	·429	·425	·412	·389	·360	·322	·275	·219	·154	·082
2 0	·436	·432	·419	·397	·366	·327	·280	·222	·157	·083
2	·443	·439	·426	·402	·373	·332	·284	·226	·160	·084
4	·451	·446	·433	·409	·379	·338	·289	·230	·162	·086

II.—TABLE OF ORDINATES—*continued.*

Distances of the Ordinates from the end of the 100 feet Chord.

Angle of De- Section.	Middle, 50 feet.	45 feet.	40 feet.	35 feet.	30 feet.	25 feet.	20 feet.	15 feet.	10 feet.	5 feet.
2 6	.458	.454	.440	.416	.385	.343	.293	.234	.165	.087
8	.465	.461	.447	.425	.391	.349	.298	.237	.167	.088
10	.473	.468	.454	.430	.397	.355	.303	.241	.170	.089
12	.480	.475	.461	.437	.403	.360	.308	.245	.173	.090
14	.487	.482	.468	.443	.409	.366	.312	.248	.175	.092
16	.495	.490	.475	.450	.415	.371	.317	.252	.178	.093
18	.502	.497	.482	.456	.421	.377	.321	.256	.180	.095
20	.509	.504	.489	.463	.428	.382	.326	.260	.183	.096
22	.516	.511	.496	.470	.434	.387	.330	.264	.186	.097
24	.523	.518	.503	.476	.440	.393	.334	.267	.188	.099
26	.531	.526	.510	.483	.446	.398	.338	.271	.191	.100
28	.538	.533	.517	.489	.452	.404	.346	.275	.194	.102
30	.545	.540	.524	.496	.458	.409	.350	.278	.196	.103
32	.552	.547	.531	.503	.465	.415	.355	.282	.199	.104
34	.560	.554	.538	.509	.471	.420	.359	.285	.201	.106
36	.567	.562	.545	.516	.477	.425	.364	.289	.204	.107
38	.574	.569	.552	.522	.483	.431	.368	.293	.206	.109
40	.582	.576	.559	.529	.489	.436	.373	.297	.209	.110
42	.589	.583	.566	.536	.495	.441	.378	.301	.212	.111
44	.596	.590	.573	.542	.501	.447	.382	.304	.214	.113
46	.603	.598	.580	.549	.507	.452	.387	.308	.217	.114
48	.611	.605	.587	.555	.513	.458	.391	.312	.219	.116
50	.618	.612	.594	.562	.519	.464	.396	.315	.222	.117
52	.625	.619	.601	.569	.526	.469	.401	.319	.225	.118
54	.632	.626	.608	.575	.532	.474	.405	.322	.227	.119
56	.640	.634	.615	.582	.538	.480	.410	.326	.230	.121
58	.647	.641	.622	.588	.544	.485	.414	.330	.232	.123
3 0	.654	.648	.629	.595	.550	.491	.419	.334	.235	.124
2	.661	.655	.636	.602	.556	.497	.424	.338	.238	.125
4	.669	.662	.643	.608	.562	.502	.428	.341	.240	.127
6	.676	.670	.650	.615	.568	.507	.433	.345	.243	.128
8	.683	.677	.657	.621	.574	.512	.438	.349	.246	.130
10	.691	.684	.664	.629	.581	.518	.443	.353	.249	.131
12	.698	.691	.671	.635	.587	.523	.448	.357	.251	.132
14	.706	.698	.678	.642	.593	.529	.452	.360	.254	.134
16	.713	.705	.685	.649	.599	.534	.457	.364	.257	.135
18	.720	.713	.692	.655	.605	.540	.462	.368	.259	.137
20	.727	.720	.699	.662	.611	.545	.466	.371	.262	.138
22	.734	.727	.706	.668	.617	.550	.471	.375	.264	.139
24	.742	.734	.713	.675	.623	.556	.475	.378	.267	.141
26	.749	.742	.720	.682	.629	.561	.480	.382	.270	.142
28	.756	.749	.727	.688	.635	.567	.485	.386	.272	.144
30	.764	.756	.734	.695	.642	.573	.489	.390	.275	.145
32	.771	.763	.741	.702	.648	.578	.494	.394	.278	.146
34	.779	.770	.748	.708	.654	.584	.498	.397	.280	.148
36	.786	.777	.755	.715	.660	.589	.503	.401	.283	.149
38	.793	.785	.762	.721	.666	.594	.508	.405	.285	.151
40	.800	.792	.769	.728	.673	.600	.512	.408	.288	.152
42	.807	.799	.776	.734	.679	.605	.517	.412	.291	.153
44	.814	.806	.783	.741	.685	.611	.521	.415	.293	.155
46	.822	.814	.790	.748	.691	.616	.526	.419	.296	.156

II.—TABLE OF ORDINATES—*continued.*

Distances of the Ordinates from the end of the 100 feet Chord.										
Angle of Deflection.	Middle, 50 feet.	45 feet.	40 feet.	35 feet.	30 feet.	25 feet.	20 feet.	15 feet.	10 feet.	5 feet.
3 48	.829	.821	.797	.754	.697	.621	.531	.423	.298	.158
50	.836	.828	.804	.761	.703	.627	.536	.427	.301	.159
52	.843	.835	.811	.768	.709	.632	.541	.431	.304	.160
54	.850	.842	.818	.774	.715	.638	.545	.434	.306	.162
56	.858	.850	.825	.781	.721	.643	.550	.438	.309	.163
58	.865	.857	.832	.787	.728	.648	.555	.442	.311	.165
4 0	.873	.864	.839	.794	.734	.655	.559	.445	.314	.166
5	.891	.882	.856	.810	.749	.668	.571	.454	.320	.169
10	.909	.900	.874	.827	.764	.682	.582	.464	.327	.173
15	.927	.918	.891	.844	.780	.695	.594	.473	.334	.176
20	.945	.936	.909	.860	.795	.709	.606	.482	.340	.179
25	.963	.954	.926	.877	.810	.723	.617	.491	.347	.183
30	.981	.972	.944	.893	.825	.736	.629	.501	.354	.186
35	.999	.990	.961	.909	.840	.750	.640	.510	.360	.189
40	1.017	1.008	.979	.926	.855	.764	.652	.519	.367	.193
45	1.036	1.026	.996	.943	.871	.777	.664	.529	.373	.196
50	1.054	1.044	1.014	.959	.886	.791	.676	.538	.380	.199
55	1.072	1.062	1.031	.976	.901	.804	.687	.547	.386	.203
5 0	1.091	1.080	1.048	.993	.917	.818	.699	.557	.393	.207
5	1.109	1.098	1.065	1.009	.932	.831	.711	.566	.400	.210
10	1.127	1.116	1.083	1.026	.947	.845	.722	.576	.406	.214
15	1.146	1.134	1.000	1.042	.963	.859	.734	.585	.413	.217
20	1.164	1.152	1.118	1.058	.978	.872	.746	.594	.419	.220
25	1.182	1.170	1.135	1.075	.993	.886	.757	.603	.426	.224
30	1.200	1.188	1.153	1.092	1.009	.900	.769	.613	.432	.228
35	1.218	1.206	1.170	1.108	1.024	.913	.781	.622	.438	.231
40	1.236	1.224	1.188	1.124	1.039	.927	.792	.631	.445	.235
45	1.255	1.242	1.205	1.141	1.055	.941	.804	.640	.452	.238
50	1.273	1.260	1.223	1.157	1.070	.954	.816	.649	.458	.241
55	1.291	1.278	1.240	1.174	1.085	.967	.827	.658	.465	.245
6 0	1.309	1.296	1.258	1.191	1.100	.982	.839	.668	.472	.248
5	1.327	1.314	1.275	1.207	1.115	.995	.851	.677	.478	.251
10	1.345	1.332	1.293	1.224	1.130	1.009	.862	.686	.485	.255
15	1.364	1.350	1.310	1.240	1.146	1.023	.874	.696	.492	.259
20	1.382	1.368	1.328	1.256	1.161	1.036	.886	.705	.498	.262
25	1.400	1.386	1.345	1.273	1.176	1.050	.897	.714	.505	.266
30	1.419	1.404	1.362	1.290	1.192	1.064	.909	.724	.511	.269
35	1.437	1.422	1.379	1.306	1.207	1.077	.921	.733	.517	.272
40	1.455	1.440	1.397	1.323	1.222	1.091	.932	.742	.524	.276
45	1.473	1.458	1.415	1.339	1.238	1.105	.944	.752	.531	.280
50	1.491	1.476	1.432	1.355	1.253	1.118	.956	.761	.537	.283
55	1.509	1.494	1.450	1.372	1.268	1.132	.967	.770	.544	.287
7 0	1.528	1.512	1.467	1.389	1.284	1.146	.979	.779	.551	.290
5	1.546	1.530	1.484	1.405	1.299	1.159	.991	.788	.557	.293
10	1.564	1.548	1.502	1.422	1.314	1.173	1.002	.798	.564	.297
15	1.582	1.566	1.520	1.438	1.330	1.187	1.014	.807	.570	.301
20	1.600	1.584	1.537	1.454	1.345	1.200	1.026	.816	.576	.304
25	1.618	1.602	1.555	1.471	1.360	1.214	1.037	.825	.583	.308
30	1.637	1.620	1.572	1.488	1.375	1.228	1.048	.835	.590	.311
35	1.655	1.638	1.589	1.504	1.390	1.241	1.060	.844	.596	.314
40	1.673	1.656	1.607	1.521	1.405	1.255	1.071	.854	.603	.318



II.—TABLE OF ORDINATES—*continued.*

Distances of the Ordinates from the end of the 100 feet Chord.										
Angle of Deflection.	Middle, 50 feet.	45 feet.	40 feet.	35 feet.	30 feet.	25 feet.	20 feet.	15 feet.	10 feet.	5 feet.
7 45	1.692	1.674	1.624	1.537	1.421	1.269	1.083	.863	.610	.321
50	1.710	1.692	1.641	1.553	1.436	1.282	1.095	.872	.616	.324
55	1.728	1.710	1.659	1.570	1.451	1.296	1.106	.881	.623	.328
8 0	1.746	1.728	1.677	1.587	1.467	1.310	1.118	.891	.629	.332
15	1.801	1.782	1.729	1.637	1.513	1.351	1.153	.918	.649	.342
30	1.855	1.836	1.782	1.687	1.559	1.392	1.188	.946	.669	.353
45	1.910	1.890	1.834	1.737	1.605	1.433	1.223	.974	.689	.363
9 0	1.965	1.944	1.886	1.787	1.651	1.474	1.258	1.002	.708	.373
15	2.019	1.998	1.939	1.837	1.696	1.515	1.293	1.030	.728	.384
30	2.074	2.052	1.991	1.887	1.742	1.556	1.328	1.057	.748	.394
45	2.128	2.106	2.044	1.937	1.788	1.597	1.363	1.085	.767	.405
10 0	2.183	2.161	2.096	1.987	1.834	1.637	1.398	1.114	.787	.415
15	2.238	2.215	2.148	2.037	1.880	1.678	1.433	1.142	.807	.425
30	2.292	2.269	2.201	2.087	1.926	1.719	1.468	1.170	.827	.436
45	2.347	2.323	2.254	2.136	1.972	1.761	1.503	1.198	.846	.446
11 0	2.401	2.377	2.306	2.186	2.018	1.802	1.538	1.226	.866	.457
15	2.456	2.432	2.359	2.236	2.064	1.843	1.574	1.254	.886	.467
30	2.511	2.486	2.411	2.286	2.110	1.884	1.609	1.282	.906	.478
45	2.566	2.540	2.464	2.336	2.156	1.926	1.644	1.310	.926	.488
12 0	2.620	2.594	2.516	2.386	2.203	1.967	1.680	1.339	.946	.499
15	2.675	2.649	2.569	2.436	2.249	2.008	1.715	1.367	.966	.509
30	2.730	2.703	2.621	2.485	2.295	2.049	1.750	1.395	.985	.520
45	2.785	2.757	2.674	2.535	2.341	2.091	1.785	1.423	1.005	.530
13 0	2.839	2.811	2.726	2.585	2.387	2.132	1.820	1.451	1.025	.541
15	2.894	2.865	2.779	2.635	2.433	2.173	1.855	1.479	1.045	.551
30	2.949	2.920	2.832	2.685	2.479	2.214	1.891	1.507	1.065	.562
45	3.000	2.974	2.884	2.735	2.525	2.256	1.926	1.535	1.085	.572
14 0	3.058	3.028	2.937	2.785	2.571	2.297	1.961	1.564	1.105	.583
15	3.113	3.082	2.989	2.834	2.618	2.338	1.996	1.592	1.124	.593
30	3.168	3.136	3.042	2.884	2.664	2.379	2.031	1.620	1.144	.604
45	3.222	3.191	3.094	2.934	2.710	2.421	2.067	1.648	1.164	.614
15 0	3.277	3.245	3.147	2.984	2.756	2.462	2.102	1.676	1.184	.625
15	3.332	3.299	3.200	3.034	2.802	2.503	2.137	1.704	1.204	.635
30	3.387	3.354	3.252	3.084	2.848	2.544	2.172	1.732	1.224	.646
45	3.442	3.408	3.305	3.134	2.895	2.586	2.208	1.760	1.244	.656
16 0	3.496	3.462	3.358	3.184	2.941	2.627	2.243	1.789	1.264	.667
30	3.606	3.571	3.463	3.284	3.033	2.710	2.314	1.845	1.304	.688
17 0	3.716	3.680	3.569	3.384	3.125	2.792	2.384	1.902	1.344	.709
30	3.826	3.788	3.674	3.484	3.218	2.875	2.455	1.958	1.384	.730
18 0	3.935	3.897	3.779	3.584	3.310	2.958	2.525	2.014	1.424	.751
30	4.045	4.006	3.885	3.684	3.403	3.040	2.596	2.071	1.464	.772
19 0	4.155	4.115	3.990	3.784	3.495	3.123	2.666	2.127	1.504	.793
30	4.265	4.223	4.096	3.884	3.588	3.205	2.737	2.184	1.544	.814
20 0	4.375	4.332	4.201	3.984	3.680	3.288	2.808	2.240	1.583	.836
21 0	4.595	4.549	4.412	4.184	3.864	3.454	2.950	2.353	1.663	.879
22 0	4.815	4.768	4.624	4.386	4.050	3.620	3.093	2.467	1.744	.922
23 0	5.035	4.986	4.836	4.587	4.237	3.786	3.236	2.581	1.824	.965
24 0	5.255	5.204	5.048	4.789	4.423	3.952	3.379	2.695	1.905	1.008
25 0	5.476	5.422	5.260	4.989	4.609	4.119	3.522	2.809	1.986	1.051
26 0	5.697	5.642	5.473	5.192	4.798	4.286	3.665	2.924	2.068	1.094
27 0	5.918	5.860	5.685	5.393	4.984	4.454	3.808	3.039	2.150	1.137

II.—TABLE OF ORDINATES—*continued.*

Distances of the Ordinates from the end of the 100 feet Chord.										
Angle of Deflection.	Middle, 50 feet.	45 feet.	40 feet.	35 feet.	30 feet.	25 feet.	20 feet.	15 feet.	10 feet.	5 feet.
28	6.139	6.079	5.898	5.595	5.171	4.622	3.952	3.154	2.232	1.181
29	6.361	6.298	6.110	5.796	5.357	4.790	4.095	3.269	2.314	1.224
30	6.582	6.517	6.323	5.999	5.544	4.958	4.239	3.385	2.396	1.268
31	6.804	6.737	6.537	6.202	5.733	5.127	4.384	3.502	2.481	1.312
32	7.027	6.957	6.751	6.406	5.922	5.297	4.530	3.619	2.565	1.356
33	7.249	7.178	6.965	6.609	6.111	5.467	4.676	3.737	2.649	1.401
34	7.472	7.398	7.179	6.813	6.300	5.637	4.822	3.854	2.733	1.445
35	7.694	7.619	7.393	7.017	6.489	5.807	4.968	3.972	2.817	1.490
36	7.918	7.841	7.609	7.222	6.679	5.978	5.115	4.090	2.901	1.535
37	8.143	8.063	7.825	7.427	6.870	6.149	5.262	4.209	2.985	1.581
38	8.367	8.286	8.041	7.633	7.060	6.320	5.410	4.327	3.069	1.626
39	8.592	8.508	8.257	7.838	7.251	6.491	5.557	4.446	3.153	1.672
40	8.816	8.731	8.474	8.044	7.442	6.663	5.705	4.565	3.238	1.718

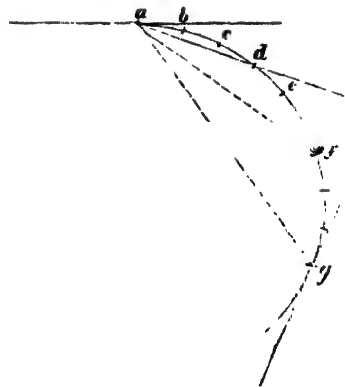
## ARTICLE XXI.

## ON LONG CHORDS.

It is sometimes convenient, in preliminary locations, to lay off curves by chords longer than 100 feet. For instance, in fig. 19, instead of running from *a* by chords *a b*, *b c*, *c d*, &c., of but 100 feet, points *d*, *f*, *g*, &c., may be obtained with less trouble by using three times the tangential or deflection angles of the table (as the case may be), and employing chords *a d*, *d f*, *f g*, &c., nearly three times as long as the chords *a b*, *b c*, &c.; or if *a d*, *d f*, *f g*, be either 2 or 4 stations apart, then 2 or 4 times the tangential and deflection angles would be used; and chords nearly 2 or 4 times 100 feet in length.

The following table contains the precise length of chord required to subtend respectively 1, 2, 3, or 4 stations. It is seldom desirable to exceed the latter limit.

Fig. 19.



## OF LONG CHORDS.

Radii in feet.	Angle of Deflection.	Length of Chord in feet required to subtend.			
		1 Station.	2 Stations.	3 Stations.	4 Stations.
5730.0	1°	100	200.0	300.0	400.0
4584.0	$\frac{1}{4}$	100	200.0	300.0	399.9
3820.0	$\frac{1}{2}$	100	200.0	300.0	399.9
3274.0	$\frac{3}{4}$	100	200.0	300.0	399.8
2865.0	2°	100	200.0	299.9	399.7
2547.0	$\frac{1}{4}$	100	200.0	299.9	399.6
2292.0	$\frac{1}{2}$	100	200.0	299.8	399.5
2084.0	$\frac{3}{4}$	100	200.0	299.8	399.4
1910.0	3°	100	200.0	299.7	399.3
1763.0	$\frac{1}{4}$	100	200.0	299.7	399.2
1637.0	$\frac{1}{2}$	100	200.0	299.6	399.1
1528.0	$\frac{3}{4}$	100	200.0	299.6	399.0
1433.0	4°	100	199.9	299.6	398.9
1348.0	$\frac{1}{4}$	100	199.9	299.5	398.7
1274.0	$\frac{1}{2}$	100	199.9	299.4	398.5
1207.0	$\frac{3}{4}$	100	199.9	299.3	398.3
1146.0	5°	100	199.9	299.2	398.0
1092.0	$\frac{1}{4}$	100	199.8	299.1	397.8
1042.0	$\frac{1}{2}$	100	199.8	299.0	397.6
996.8	$\frac{3}{4}$	100	199.7	298.9	397.5
955.4	6°	100	199.7	298.8	397.3
917.0	$\frac{1}{4}$	100	199.7	298.7	397.0
882.0	$\frac{1}{2}$	100	199.7	298.6	396.7
849.3	$\frac{3}{4}$	100	199.6	298.5	396.5
819.0	7°	100	199.6	298.4	396.2
790.8	$\frac{1}{4}$	100	199.6	298.3	396.0
764.5	$\frac{1}{2}$	100	199.6	298.2	395.7
739.9	$\frac{3}{4}$	100	199.6	298.1	395.4
716.8	8°	100	199.6	298.0	395.1
695.1	$\frac{1}{4}$	100	199.5	297.9	394.8
674.6	$\frac{1}{2}$	100	199.5	297.8	394.5
655.5	$\frac{3}{4}$	100	199.4	297.7	394.3
637.3	9°	100	199.4	297.5	394.1
620.2	$\frac{1}{4}$	100	199.4	297.4	393.7
603.8	$\frac{1}{2}$	100	199.3	297.3	393.2
588.4	$\frac{3}{4}$	100	199.2	297.2	392.8
573.7	10°	100	199.2	297.0	392.4

For radii less than 573.7 feet, it is never required to use longer chords than 100 feet.

When this method of laying out curves by long chords is used, the instrument should be moved to each successive point after it is determined, in order to fix the next one, instead of attempting to obtain more than one point from one position of the instrument: because when the chords are longer than one chain, they cannot be measured in the right direction by eye, but must be guided by the instrument.

It must be especially borne in mind that, in any given curve, only the tangential and deflection angles increase in the same proportion

as the number of 100 feet stations subtended by the long chord. Therefore, *these* long chords cannot be used for laying out curves *by eye*, as their tangential and deflection *distances* are not known.

When it is required to use long chords for turning a curve *by eye*, they must be composed of a number of *whole chains*, being made say 200, 300, or 400, &c., feet in length. The tangential and deflection *distances* of curves of more than 500 feet radius may then be assumed, in practice, to increase as the *squares* of the number of chains in the length of the long chord. For instance, to lay off a  $5^\circ$  curve by chords of 200, 300, or 400 feet in length, the tangential and deflection distances of the table must be multiplied by 4, 9, or 16, as the case may be. In this case the tangential and deflection *angles* are unknown.

This is not mathematically correct, but will answer in practice for the curves of a canal or common road, where great nicety is not needed.

The only proper instrument for running lines of survey is the *transit*, furnished with a compass and with a revolving telescope. The deflections, being measured in *angles*, serve as a check to the numerous sources of error to which the compass is liable, arising from local attraction, electrical action in the glass cover, diurnal variation, &c. Besides, when the compass alone is used, it is necessary to test every course or bearing from each end of each station; and this involves loss of time.

The following is a good form of field-book for the transit and compass combined :

Station.	Distance.	Total Distance.	Course.	Deflection in Degrees.		The right-hand page is left blank for Re- marks, and Sketches of Topography.
				Left.	Right.	

In every locating party there should be one person whose duty is to obtain and record the transverse slopes of the ground at each station. His observations will usually extend to from fifty feet to one hundred yards on each side of the centre stakes, depending on a variety of circumstances of locality which cannot be alluded to here. In preliminary locations these slopes need not be taken with very great nicety, as they will be used chiefly for ascertaining, approximately, the amount of excavation and embankment.

After the final location is made, the slopes should be taken again, with great care, to the nearest quarter of a degree; but need not extend beyond the width actually occupied by the road. Their use in this second operation will be for determining the cubic contents with more precision than before, for final estimates; and also for obtaining the positions of the *side-stakes*.

Should the duty of *recording* these slopes devolve upon the com-

Fig. 20.



passman (which it should not), it will be necessary to add another column to his field-book after that containing the deflections. In this column he will insert the slopes thus (fig. 20), the dot

representing the centre stake. The degrees of slope are written above the lines, and the distance in feet to which they extend below.

The slopes are taken by laying a long rod on the ground, at *right angles to the line of survey*, as nearly as may be judged by eye, and measuring the angles by means of a small *slope instrument* placed upon the rod. These are made by most of our instrument-makers.

## ARTICLE XXII.

### TO ADJUST A TRANSIT INSTRUMENT.

Having placed the transit firmly at *a*, fig. 21, and levelled it, clamp all fast, and direct the cross-hairs, by means of the tangent screw, to some convenient object, *b*. Then, revolving the telescope *vertically*, but without moving it in the least *horizontally*, let the cross-hairs fix upon a second object in the opposite direction, as *c*; or, if there be no

Fig. 21.



such object, place one, as for instance a chain-pin, at any convenient distance. Then unclamp the *lower* clamp, and revolve *horizontally* the entire

upper part of the instrument above the parallel plates. Clamp it again, and fix the cross-hairs upon *b*; then again revolve the *telescope* vertically. If the sight now strikes *c*, as before, it is in adjustment; but if not, place another object, *d*, where it does strike; and with the adjusting pin alter the vertical cross-hair so as to strike half-way between *d* and *c*. The instrument will then be in adjustment. Two or more trials will generally be needed before the adjustment is perfect.

With care, and on a firm floor, the operation may be performed in a long room, or by placing the instrument in a doorway communicating with two rooms of moderate size. Fine pins, or needles, should then be used as the objects to be sighted at. It is better, however, to adjust out of doors, with more distant objects. It is also a good precaution to hang up a long plumb-line, or select some vertical object, and see whether the vertical hair coincides with it, as the telescope is raised or lowered. If from any accident, or carelessness in its construction, it does not, the defect must be remedied by an instrument-maker.

IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1.

0 Deg.

0 Deg.

0 Deg.

	Sine.	Tang.	Cotang.	Cosine.		Sine.	Tang.	Cotang.	Cosine.		Sine.	Tang.	Cotang.	Cosine.	
0	0.0000000	0.00000	Infinite.	1.0000000	60	21	0.0061086	0.06108	163.70010	39	41	-0.119261	-0.11927	83.84350	9999289
1	0.0002909	-0.00291	3437.7460	1.0000000	59	22	0.0063995	0.06399	156.25000	38	42	-0.122170	-0.12217	81.84704	9999284
2	0.0005818	-0.00582	1718.8730	9999998	58	23	0.0066901	0.06690	149.46500	37	43	-0.125079	-0.12508	79.94343	9999218
3	0.0008727	-0.00872	1145.9150	9999996	57	24	0.0069813	0.06981	143.23710	36	44	-0.127987	-0.12799	78.12634	9999181
4	0.0011636	-0.01163	859.4363	9999993	56	25	0.0072721	0.07272	137.50750	35	45	-0.130896	-0.13090	76.39000	9999143
5	0.0014544	-0.01454	687.4588	9999989	55	26	0.0075630	0.07563	132.21850	34	46	-0.133805	-0.13381	74.72916	9999105
6	0.0017453	-0.01745	572.9572	9999985	54	27	0.0078539	0.07854	127.32130	33	47	-0.136713	-0.13672	73.13899	9999065
7	0.0020362	-0.02036	491.1060	9999979	53	28	0.0081448	0.08145	122.77300	32	48	-0.139622	-0.13963	71.61507	9999025
8	0.0023271	-0.02327	429.7175	9999973	52	29	0.0084357	0.08436	118.54010	31	49	-0.142530	-0.14254	70.15334	9998984
9	0.0026180	-0.02618	381.9709	9999966	51	30	0.0087265	0.08726	114.58860	30	50	-0.145439	-0.14545	68.75008	9998942
10	0.0029089	-0.02908	343.7737	9999958	50	31	0.0090174	0.09017	110.89200	29	51	-0.148348	-0.14836	67.40185	9998900
11	0.0031998	-0.03199	312.5213	9999949	49	32	0.0093083	0.09308	107.42640	28	52	-0.151256	-0.15127	66.10547	9998856
12	0.0034907	-0.03490	286.4777	9999939	48	33	0.0095992	0.09599	104.17090	27	53	-0.154165	-0.15418	64.85800	9998812
13	0.0037815	-0.03781	264.4408	9999928	47	34	0.0098900	0.09890	101.10690	26	54	-0.157073	-0.15709	63.65674	9998766
14	0.0040724	-0.04072	245.5519	9999917	46	35	0.0101809	0.10181	98.21791	25	55	-0.159982	-0.16000	62.49015	9998720
15	0.0043633	-0.04363	229.1816	9999905	45	36	0.0104718	0.10472	95.48917	24	56	-0.162890	-0.16291	61.38290	9998673
16	0.0046542	-0.04654	214.8576	9999892	44	37	0.0107627	0.10763	92.90848	23	57	-0.165799	-0.16582	60.30582	9998625
17	0.0049451	-0.04945	200.9841	9999878	43	38	0.0110535	0.11054	90.46333	22	58	-0.168707	-0.16873	59.26587	9998577
18	0.0052360	-0.05236	192.9817	9999863	42	39	0.0113444	0.11345	88.14357	21	59	-0.171616	-0.17164	58.26117	9998527
19	0.0055268	-0.05526	180.9322	9999847	41	40	0.0116353	0.11636	85.83079	20	60	-0.174524	-0.17455	57.28996	9998477
20	0.0058177	-0.05817	171.8854	9999831	40										
	Cosine.	Cotan.	Tang.	Sine.			Cosine.	Cotan.	Tang.			Cosine.	Cotan.	Tang.	Sine.

Deg. 89.

Deg. 89.

Deg. 89.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

• 1 Deg.

1 Deg.

1 Deg.

	Sine.	Tang.	Cotang.	Cosine.			Sine.	Tang.	Cotang.	Cosine.			Sine.	Tang.	Cotang.	Cosine.	
0	-0174524	-017455	57.28996	.9998477	60	21	-0235598	-023566	42.43346	.9997224	39	41	-0293755	-029388	34.02730	.9995684	19
1	-0177432	-017746	56.35059	.9998426	59	12	-0238506	-023857	41.91579	.9997156	38	42	-0296662	-029679	33.69350	.9995599	18
2	-0180341	-018037	55.44151	.9998374	58	23	-0241414	-024148	41.41058	.9997086	37	43	-0299570	-029970	33.36619	.9995512	17
3	-0183249	-018328	54.56130	.9998321	57	24	-0244322	-024439	40.91741	.9997015	36	44	-0302478	-030261	33.04517	.9995424	16
4	-0186158	-018619	53.70858	.9998267	56	25	-0247230	-024730	40.43583	.9996933	35	45	-0305385	-030552	32.73026	.9995336	15
5	-0189066	-018910	52.88211	.9998213	55	26	-0250138	-025021	39.96546	.9996871	34	46	-0308293	-030843	32.42129	.9995247	14
6	-0191974	-019201	52.08067	.9998157	54	27	-0253046	-025312	39.50589	.9996798	33	47	-0311200	-031135	32.11809	.9995157	13
7	-0194883	-019492	51.30315	.9998101	53	28	-0255954	-025603	39.05677	.9996724	32	48	-0314108	-031426	31.82051	.9995066	12
8	-0197791	-019783	50.54850	.9998044	52	29	-0258862	-025894	38.61773	.9996649	31	49	-0317015	-031717	31.52839	.9994974	11
9	-0200699	-020074	49.81572	.9997986	51	30	-0261769	-026185	38.18815	.9996573	30	50	-0319922	-032008	31.24157	.9994881	10
10	-0203608	-020365	49.10388	.9997927	50	31	-0264677	-026477	37.76861	.9996497	29	51	-0322830	-032299	30.95992	.9994788	9
11	-0206516	-020656	48.41208	.9997867	49	32	-0267585	-026768	37.35789	.9996419	28	52	-0325737	-032591	30.68330	.9994693	8
12	-0209424	-020947	47.73950	.9997807	48	33	-0270493	-027059	36.95600	.9996341	27	53	-0328644	-032882	30.41158	.9994598	7
13	-0212332	-021238	47.08534	.9997745	47	34	-0273401	-027350	36.56265	.9996262	26	54	-0331552	-033173	30.14461	.9994502	6
14	-0215241	-021529	46.44886	.9997683	46	35	-0276309	-027641	36.17759	.9996182	25	55	-0334459	-033464	29.88229	.9994405	5
15	-0218149	-021820	45.82935	.9997620	45	36	-0279216	-027932	35.80055	.9996101	24	56	-0337366	-033755	29.62449	.9994308	4
16	-0221057	-022111	45.22614	.9997556	44	37	-0282124	-028223	35.43128	.9996020	23	57	-0340274	-034047	29.37110	.9994209	3
17	-0223965	-022402	44.63859	.9997492	43	38	-0285032	-028514	35.06954	.9995937	22	58	-0343181	-034338	29.12200	.9994110	2
18	-0226873	-022693	44.06611	.9997426	42	39	-0287940	-028805	34.71511	.9995854	21	59	-0346088	-034629	28.87708	.9994009	1
19	-0229781	-022984	43.50812	.9997360	41	40	-0290847	-029097	34.36777	.9995770	20	60	-0348995	-034920	28.63625	.9993908	0
20	-0232690	-023275	42.96107	.9997292	40												
	Cosine.	Cotan.	Tang.	Sine.			Cosine.	Cotan.	Tang.	Sine.			Cosine.	Cotan.	Tang.	Sine.	

Deg. 88.

Deg. 88.

Deg. 88.

IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—*continued*.

2 Deg.

2 Deg.

2 Deg.

	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.
0	-0348995	-034920	28.63625	.9993908	60	21	-0410037	-041038	24.36750	.9991590	39	41	-0468159	-046867	21.33685	.9989035
1	-0351902	-035212	28.39939	.9993806	59	22	-0412944	-041329	24.19571	.9991470	38	42	-0471065	-047158	21.20494	.9988899
2	-0354809	-035503	28.16642	.9993704	58	23	-0415850	-041621	24.02632	.9991350	37	43	-0473970	-047450	21.07466	.9988761
3	-0357716	-035794	27.93723	.9993600	57	24	-0418757	-041912	23.85927	.9991228	36	44	-0476876	-047741	20.94596	.9988623
4	-0360623	-036085	27.71174	.9993495	56	25	-0421663	-042203	23.69453	.9991106	35	45	-0479781	-048033	20.81182	.9988484
5	-0363530	-036377	27.48985	.9993390	55	26	-0424569	-042495	23.53205	.9990983	34	46	-0482687	-048325	20.69322	.9988344
6	-0366437	-036668	27.27148	.9993284	54	27	-0427475	-042786	23.37177	.9990859	33	47	-0485592	-048616	20.56911	.9988203
7	-0369344	-036959	27.05655	.9993177	53	28	-0430382	-043078	23.21366	.9990734	32	48	-0488498	-048908	20.44648	.9988061
8	-0372251	-037250	26.84498	.9993069	52	29	-0433288	-043369	23.05767	.9990609	31	49	-0491403	-049199	20.32530	.9987919
9	-0375158	-037542	26.63659	.9992960	51	30	-0436191	-043660	22.90376	.9990482	30	50	-0494308	-049491	20.20555	.9987775
10	-0378065	-037833	26.43160	.9992851	50	31	-0439100	-043952	22.75189	.9990355	29	51	-0497214	-049782	20.08719	.9987631
11	-0380971	-038124	26.22963	.9992740	49	32	-0442006	-044243	22.60201	.9990227	28	52	-0500119	-050074	19.97021	.9987486
12	-0383878	-038416	26.03073	.9992629	48	33	-0444912	-044535	22.45409	.9990098	27	53	-0503024	-050366	19.85459	.9987340
13	-0386785	-038707	25.83482	.9992517	47	34	-0447818	-044826	22.30809	.9989968	26	54	-0505929	-050657	19.74029	.9987194
14	-0389692	-038998	25.64183	.9992404	46	35	-0450724	-045118	22.16398	.9989837	25	55	-0508835	-050949	19.62729	.9987046
15	-0392598	-039290	25.45170	.9992290	55	36	-0453630	-045409	22.02171	.9989706	24	56	-0511740	-051241	19.51558	.9986898
16	-0395505	-039581	25.26436	.9992176	44	37	-0456536	-045701	21.88125	.9989573	23	57	-0514645	-051532	19.40513	.9986748
17	-0398411	-039872	25.07975	.9992060	43	38	-0459442	-045992	21.74256	.9989440	22	58	-0517550	-051824	19.29592	.9986598
18	-0401318	-040164	24.89782	.9991944	42	39	-0462347	-046284	21.60563	.9989306	21	59	-0520455	-052116	19.18793	.9986447
19	-0404224	-040455	24.71851	.9991827	41	40	-0465253	-046575	21.47040	.9989171	20	60	-0523360	-052407	19.08113	.9986295
20	-0407131	-040746	24.54175	.9991709	40											

Deg. 87.

Deg. 87.

Deg. 87.



## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

3 Deg.

3 Deg.

3 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	0.0523360	0.52407	19.08113	0.9986295	60	21	0.581352	0.58535	17.08372	99.82912	39	41	0.642120	0.64375	15.53398
1	0.0526264	0.52699	18.97552	0.9986143	59	22	0.587256	0.58827	16.99895	99.82742	38	42	0.645323	0.64667	15.46381
2	0.0529169	0.52991	18.87106	0.9985989	58	23	0.590160	0.59119	16.91502	99.82570	37	43	0.648226	0.64959	15.39427
3	0.0532074	0.53282	18.76775	0.9985835	57	24	0.593064	0.59410	16.83191	99.82398	36	44	0.651129	0.65251	15.32535
4	0.0534979	0.53574	18.66556	0.9985680	56	25	0.595967	0.59702	16.74961	99.82225	35	45	0.654031	0.65543	15.25705
5	0.0537883	0.53866	18.56447	0.9985524	55	26	0.598871	0.59994	16.66811	99.82052	34	46	0.656934	0.65835	15.18934
6	0.0540788	0.54158	18.46447	0.9985367	54	27	0.601775	0.60286	16.58739	99.81877	33	47	0.659836	0.66127	15.12224
7	0.0543693	0.54449	18.36533	0.9985209	53	28	0.604678	0.60578	16.50743	99.81701	32	48	0.662739	0.66419	15.05572
8	0.0546597	0.54741	18.26765	0.9985050	52	29	0.607582	0.60870	16.42827	99.81525	31	49	0.665641	0.66712	14.98978
9	0.0549502	0.55033	18.17080	0.9984891	51	30	0.610485	0.61162	16.34983	99.81348	30	50	0.668544	0.67004	14.92441
10	0.0552406	0.55325	18.07497	0.9984731	50	31	0.613389	0.61454	16.27217	99.81170	29	51	0.671446	0.67296	14.85961
11	0.0555311	0.55616	17.98015	0.9984570	49	32	0.616292	0.61746	16.19522	99.80991	28	52	0.674349	0.67588	14.79537
12	0.0558215	0.55908	17.88631	0.9984408	48	33	0.619196	0.62038	16.11899	99.80811	27	53	0.677251	0.67880	14.73167
13	0.0561119	0.56200	17.79344	0.9984245	47	34	0.622099	0.62330	16.04348	99.80631	26	54	0.680153	0.68173	14.66832
14	0.0564021	0.56492	17.70152	0.9984081	46	35	0.625002	0.62622	15.96866	99.80450	25	55	0.683055	0.68465	14.60591
15	0.0566928	0.56784	17.61055	0.9983917	45	36	0.627905	0.62914	15.89454	99.80267	24	56	0.685957	0.68757	14.54383
16	0.0569832	0.57075	17.52051	0.9983751	44	37	0.630808	0.63206	15.82110	99.80084	23	57	0.688859	0.69049	14.48227
17	0.0572736	0.57367	17.43138	0.9983585	43	38	0.633711	0.63498	15.74833	99.79900	22	58	0.691761	0.69342	14.42123
18	0.0575640	0.57659	17.34315	0.9983418	42	39	0.636614	0.63790	15.67623	99.79716	21	59	0.694663	0.69634	14.36069
19	0.0578544	0.57951	17.25580	0.9983250	41	40	0.639517	0.64082	15.60478	99.79530	20	60	0.697565	0.69926	14.30066
20	0.0581448	0.58243	17.16933	0.9983082	40										
'	Cosine.	Tang.	Sine.	'	'	Cosine.	Tang.	Cotan.	'	'	Sine.	Tang.	Cotan.	Cosine.	'

Deg. 86.

Deg. 86.

Deg. 86

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

4 Deg.				4 Deg.				4 Deg.				
'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	
0	0.0697565	-0.69926	14.30066	9975641	60	21	-0.758489	-0.76068	13.14612	-9971193	39	41
1	0.7000467	-0.70219	14.24113	9975437	59	22	-0.761390	-0.76360	13.09575	-9970972	38	42
2	0.703368	-0.70511	14.18209	9975233	58	23	-0.764290	-0.76653	13.04576	-9970750	37	43
3	0.706270	-0.70803	14.12553	9975028	57	24	-0.767190	-0.76945	12.99616	-9970528	36	44
4	0.709171	-0.71096	14.06545	9974822	56	25	-0.770091	-0.77238	12.94692	-9970304	35	45
5	0.712073	-0.71388	14.00785	9974615	55	26	-0.772991	-0.77531	12.89805	-9970080	34	46
6	0.714974	-0.71680	13.95071	9974408	54	27	-0.775891	-0.77823	12.84955	-9969854	33	47
7	0.717876	-0.71973	13.89404	9974199	53	28	-0.778791	-0.78116	12.80141	-9969628	32	48
8	0.720777	-0.72265	13.83782	9973990	52	29	-0.781691	-0.78409	12.75363	-9969401	31	49
9	0.723678	-0.72558	13.78206	9973780	51	30	-0.784591	-0.78701	12.70620	-9969173	30	50
10	0.726580	-0.72850	13.72673	9973569	50	31	-0.787491	-0.78994	12.65912	-9968945	29	51
11	0.729481	-0.73143	13.67185	9973357	49	32	-0.790391	-0.79287	12.61239	-9968715	28	52
12	0.732382	-0.73435	13.61740	9973145	48	33	-0.793290	-0.79579	12.56599	-9968485	27	53
13	0.735283	-0.73727	13.56339	9972931	47	34	-0.796190	-0.79872	12.51994	-9968254	26	54
14	0.738184	-0.74020	13.50979	9972717	46	35	-0.799090	-0.80165	12.47422	-9968022	25	55
15	0.741085	-0.74312	13.45662	9972502	45	36	-0.801989	-0.80458	12.42883	-9967789	24	56
16	0.743986	-0.74605	13.40386	9972286	44	37	-0.804889	-0.80750	12.38376	-9967555	23	57
17	0.746887	-0.74897	13.35151	9972069	43	38	-0.807788	-0.81043	12.33902	-9967321	22	58
18	0.749787	-0.75190	13.29957	9971851	42	39	-0.810687	-0.81336	12.29460	-9967085	21	59
19	0.752688	-0.75482	13.24803	9971633	41	40	-0.813587	-0.81629	12.25050	-9966849	20	60
20	0.755589	-0.75775	13.19688	9971413	40							
4 Deg.				4 Deg.				4 Deg.				
'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	
19	9966612	-0.81922	12.20671	-0.816486	41	9966612	-0.81922	12.20671	-0.816486	41	9966612	
18	9966374	-0.82215	12.16323	-0.819385	42	9966374	-0.82215	12.16323	-0.819385	42	9966374	
17	9966135	-0.82507	12.12006	-0.822284	43	9966135	-0.82507	12.12006	-0.822284	43	9966135	
16	9965895	-0.82800	12.07719	-0.825183	44	9965895	-0.82800	12.07719	-0.825183	44	9965895	
15	9965655	-0.83093	12.03462	-0.828082	45	9965655	-0.83093	12.03462	-0.828082	45	9965655	
14	9965414	-0.83386	11.99234	-0.830981	46	9965414	-0.83386	11.99234	-0.830981	46	9965414	
13	9965172	-0.83679	11.95037	-0.833880	47	9965172	-0.83679	11.95037	-0.833880	47	9965172	
12	9964929	-0.83972	11.90868	-0.836778	48	9964929	-0.83972	11.90868	-0.836778	48	9964929	
11	9964685	-0.84265	11.86728	-0.839677	49	9964685	-0.84265	11.86728	-0.839677	49	9964685	
10	9964440	-0.84558	11.82616	-0.842576	50	9964440	-0.84558	11.82616	-0.842576	50	9964440	
9	9964195	-0.84851	11.78533	-0.845474	51	9964195	-0.84851	11.78533	-0.845474	51	9964195	
8	9963948	-0.85144	11.74477	-0.848373	52	9963948	-0.85144	11.74477	-0.848373	52	9963948	
7	9963701	-0.85437	11.70450	-0.851271	53	9963701	-0.85437	11.70450	-0.851271	53	9963701	
6	9963453	-0.85730	11.66449	-0.854169	54	9963453	-0.85730	11.66449	-0.854169	54	9963453	
5	9963204	-0.86023	11.62476	-0.857067	55	9963204	-0.86023	11.62476	-0.857067	55	9963204	
4	9962954	-0.86316	11.58529	-0.859966	56	9962954	-0.86316	11.58529	-0.859966	56	9962954	
3	9962704	-0.86609	11.54609	-0.862864	57	9962704	-0.86609	11.54609	-0.862864	57	9962704	
2	9962452	-0.86902	11.50715	-0.865762	58	9962452	-0.86902	11.50715	-0.865762	58	9962452	
1	9962200	-0.87195	11.46847	-0.868660	59	9962200	-0.87195	11.46847	-0.868660	59	9962200	
0	9961947	-0.87488	11.43005	-0.871557	60	9961947	-0.87488	11.43005	-0.871557	60	9961947	

Deg. 85.

Deg. 85.

Deg. 85.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

• 5 Deg.

5 Deg.

5 Deg.

	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'
0	0.0871557	0.87488	11.43005	0.9961947	60	21	0.932395	0.93647	10.67834	0.956437	39	41	0.999303	0.99519	10.048280	0.9950844	19
1	0.0874455	0.87781	11.39188	0.9961693	59	22	0.935291	0.93940	10.64499	0.956165	38	42	0.999197	0.99813	10.018710	0.9950556	18
2	0.0877353	0.88074	11.35397	0.9961438	58	23	0.938187	0.94234	10.61184	0.955892	37	43	0.999092	1.00107	9.999305	0.9950266	17
3	0.0880251	0.88368	11.31630	0.9961183	57	24	0.941083	0.94527	10.57889	0.955620	36	44	0.998986	1.00400	9.996072	0.9949976	16
4	0.0883148	0.88661	11.27888	0.9960926	56	25	0.943979	0.94821	10.54615	0.955345	35	45	1.001881	1.00694	9.931008	0.9949685	15
5	0.0886046	0.88954	11.24171	0.9960669	55	26	0.946875	0.95114	10.51360	0.955070	34	46	1.004775	1.00988	9.902112	0.9949393	14
6	0.0888943	0.89247	11.20478	0.9960411	54	27	0.949771	0.95408	10.48126	0.954794	33	47	1.007669	1.01282	9.873382	0.9949101	13
7	0.0891840	0.89540	11.16808	0.9960152	53	28	0.952666	0.95701	10.44911	0.954517	32	48	1.010563	1.01576	9.844816	0.9948807	12
8	0.0894738	0.89834	11.13163	0.9959892	52	29	0.955562	0.95995	10.41715	0.954240	31	49	1.013457	1.01870	9.816414	0.9948513	11
9	0.0897635	0.90127	11.09541	0.9959631	51	30	0.958458	0.96289	10.38539	0.953962	30	50	1.016351	1.02164	9.788173	0.9948217	10
10	0.0900532	0.90420	11.05943	0.9959370	50	31	0.961353	0.96582	10.35382	0.953683	29	51	1.019245	1.02458	9.760092	0.9947921	9
11	0.0903429	0.90713	11.02367	0.9959107	49	32	0.964248	0.96876	10.32244	0.953403	28	52	1.022138	1.02752	9.732171	0.9947625	8
12	0.0906326	0.91007	10.98815	0.9958844	48	33	0.967144	0.97169	10.29125	0.953122	27	53	1.025032	1.03046	9.704407	0.9947327	7
13	0.0909223	0.91300	10.95285	0.9958580	47	34	0.970039	0.97463	10.26024	0.952840	26	54	1.027925	1.03339	9.676800	0.9947028	6
14	0.0912119	0.91593	10.91777	0.9958315	46	35	0.972934	0.97757	10.22942	0.952557	25	55	1.030819	1.03634	9.649347	0.9946729	5
15	0.0915016	0.91887	10.88292	0.9958049	45	36	0.975829	0.98050	10.19878	0.952274	24	56	1.033712	1.03928	9.622048	0.9946428	4
16	0.0917913	0.92180	10.84828	0.9957783	44	37	0.978724	0.98344	10.16833	0.951990	23	57	1.036605	1.04222	9.594902	0.9946127	3
17	0.0920809	0.92473	10.81387	0.9957515	43	38	0.981619	0.98638	10.13805	0.951705	22	58	1.039499	1.04516	9.567906	0.9945825	2
18	0.0923706	0.92767	10.77967	0.9957247	42	39	0.984514	0.98932	10.10795	0.951419	21	59	1.042392	1.04810	9.541061	0.9945523	1
19	0.0926602	0.93060	10.74568	0.9956978	41	40	0.987408	0.99225	10.07803	0.951132	20	60	1.045285	1.05104	9.514364	0.9945219	0
20	0.0929499	0.93354	10.71191	0.9956708	40												

Deg. 84.

Deg. 84.

Deg. 84.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

6 Deg.				6 Deg.				6 Deg.				6 Deg.					
'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'		
0	·1045285	·105104	9·514364	·9945219	60	21	·1106017	·111284	8·985984	·9938648	39	41	·1163818	·117178	8·534017	·9932045	19
1	·1048178	·105398	9·487814	·9944914	59	22	·1108908	·111578	8·962266	·9938326	38	42	·1166707	·117473	8·512594	·9931706	18
2	·1051070	·105692	9·461411	·9944609	58	23	·1111799	·111873	8·938672	·9938003	37	43	·1169596	·117767	8·491277	·9931367	17
3	·1053963	·105986	9·435153	·9944303	57	24	·1114689	·112168	8·915200	·9937679	36	44	·1172485	·118062	8·470065	·9931026	16
4	·1056856	·106280	9·409038	·9943996	56	25	·1117580	·112462	8·891850	·9937355	35	45	·1175374	·118357	8·448957	·9930685	15
5	·1059748	·106575	9·383066	·9943688	55	26	·1120471	·112757	8·868620	·9937029	34	46	·1178263	·118652	8·427953	·9930342	14
6	·1062641	·106869	9·357255	·9943379	54	27	·1123361	·113051	8·845510	·9936703	33	47	·1181151	·118947	8·407051	·9929999	13
7	·1065533	·107163	9·331545	·9943070	53	28	·1126252	·113346	8·822518	·9936375	32	48	·1184040	·119242	8·386251	·9929655	12
8	·1068425	·107457	9·305993	·9942760	52	29	·1129142	·113641	8·799644	·9936047	31	49	·1186928	·119537	8·365553	·9929310	11
9	·1071318	·107751	9·280580	·9942448	51	30	·1132032	·113935	8·776887	·9935719	30	50	·1189816	·119832	8·344955	·9928965	10
10	·1074210	·108046	9·255303	·9942136	50	31	·1134922	·114230	8·754246	·9935389	29	51	·1192704	·120127	8·324457	·9928618	9
11	·1077102	·108340	9·230162	·9941823	49	32	·1137812	·114525	8·731719	·9935058	28	52	·1195593	·120423	8·304058	·9928271	8
12	·1079994	·108634	9·205156	·9941510	48	33	·1140702	·114819	8·709307	·9934727	27	53	·1198481	·120718	8·283757	·9927922	7
13	·1082885	·108929	9·180283	·9941195	47	34	·1143592	·115114	8·687008	·9934395	26	54	·1201368	·121013	8·263554	·9927573	6
14	·1085777	·109223	9·155543	·9940880	46	35	·1146482	·115409	8·664822	·9934062	25	55	·1204256	·121308	8·243448	·9927224	5
15	·1088669	·109517	9·130934	·9940563	45	36	·1149372	·115703	8·642747	·9933728	24	56	·1207144	·121603	8·223438	·9926873	4
16	·1091560	·109812	9·106456	·9940246	44	37	·1152261	·115998	8·620783	·9933393	23	57	·1210031	·121898	8·203523	·9926521	3
17	·1094452	·110106	9·082107	·9939928	43	38	·1155151	·116293	8·598929	·9933057	22	58	·1212919	·122194	8·183704	·9926169	2
18	·1097343	·110401	9·057886	·9939610	42	39	·1158040	·116588	8·577183	·9932721	21	59	·1215806	·122489	8·163978	·9925816	1
19	·1100234	·110695	9·033793	·9939290	41	40	·1160929	·116883	8·555546	·9932384	20	60	·1218693	·122784	8·144346	·9925462	0
20	·1103126	·110989	9·009826	·9938969	40												
'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'		
Deg. 83.																	
Deg. 83.																	
Deg. 83.																	

Deg. 83.

Deg. 83.

Deg. 83.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

7 Deg.										7 Deg.									
'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.
0	121863	122784	8.144346	.9925462	60	21.1279302	128990	7.752536	.9917832	39	41.1336979	134909	7.412397	.9910221	19				
1	1221581	123079	8.124807	.9925107	59	22.1282186	129885	7.734802	.9917459	38	42.1339862	135205	7.396159	.9909832	18				
2	1224468	123375	8.103359	.9924751	58	23.1285071	129581	7.717148	.9917086	37	43.1342744	135501	7.379990	.9909442	17				
3	1227355	123670	8.086004	.9924394	57	24.1287956	129877	7.699573	.9916712	36	44.1345627	135797	7.363891	.9909051	16				
4	1230241	123965	8.067739	.9924037	56	25.1290841	130173	7.682076	.9916337	35	45.1348509	136094	7.347861	.9908659	15				
5	1233128	124261	8.047564	.9923679	55	26.1293725	130469	7.664658	.9915901	34	46.1351392	136390	7.331896	.9908266	14				
6	1236015	124556	8.028479	.9923319	54	27.1296609	130764	7.647317	.9915584	33	47.1354274	136686	7.316004	.9907873	13				
7	1238901	124852	8.009483	.9922959	53	28.1299494	131060	7.630053	.9915206	32	48.1357156	136983	7.300178	.9907478	12				
8	1241788	125147	7.990575	.9922599	52	29.1302378	131356	7.612865	.9914828	31	49.1360038	137279	7.284418	.9907083	11				
9	1244674	125442	7.971755	.9922237	51	30.1305262	131652	7.595754	.9914449	30	50.1362919	137575	7.268725	.9906687	10				
10	1247560	125738	7.953022	.9921874	50	31.1308146	131948	7.578717	.9914069	29	51.1365801	137872	7.253098	.9906290	9				
11	1250446	126033	7.934375	.9921511	49	32.1311030	132244	7.561756	.9913688	28	52.1368683	138168	7.237537	.9905893	8				
12	1253332	126329	7.915815	.9921147	48	33.1313913	132540	7.544869	.9913306	27	53.1371564	138465	7.222042	.9905494	7				
13	1256218	126624	7.897339	.9920782	47	34.1316797	132836	7.528057	.9912923	26	54.1374445	138761	7.206611	.9905095	6				
14	1259104	126920	7.878948	.9920416	46	35.1319681	133132	7.511317	.9912540	25	55.1377327	139058	7.191245	.9904694	5				
15	1261990	127216	7.860612	.9920049	45	36.1322564	133428	7.494651	.9912155	24	56.1380208	139354	7.175943	.9904293	4				
16	1264875	127511	7.842419	.9919682	44	37.1325447	133724	7.478057	.9911770	23	57.1383089	139651	7.160705	.9903891	3				
17	1267761	127807	7.824279	.9919314	43	38.1328330	134020	7.461535	.9911384	22	58.1385970	139947	7.145530	.9903489	2				
18	1270646	128103	7.806221	.9918944	42	39.1331213	134316	7.445085	.9910997	21	59.1388850	140244	7.130419	.9903085	1				
19	1273531	128398	7.788245	.9918574	41	40.1334096	134612	7.428706	.9910610	20	60.1391731	140540	7.115369	.9902681	0				
20	1276416	128694	7.770350	.9918204	40														
'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.

Deg. 82.

Deg. 82.

Deg. 82.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

8 Deg.

8 Deg.

8 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	1391731	140540	7-115369	9902681	60	21	1452197	146775	6-813122	9893994	39	41	1509733	152723	19
1	1394612	140837	7-100382	9902275	59	22	1455075	147072	6-799356	9893572	38	42	1512608	153021	18
2	1397492	141134	7-085457	9901869	58	23	1457953	147369	6-785644	9893148	37	43	1515484	153319	17
3	1400372	141430	7-070593	9901462	57	24	1460830	147667	6-771986	9892723	36	44	1518359	153617	16
4	1403252	141727	7-055790	9901055	56	25	1463708	147964	6-758382	9892298	35	45	1521234	153914	15
5	1406132	142024	7-041648	9900646	55	26	1466585	148261	6-744831	9891872	34	46	1524109	154212	14
6	1409012	142321	7-026366	9900237	54	27	1469463	148559	6-731334	9891445	33	47	1526984	154510	13
7	1411892	142617	7-011744	9899826	53	28	1472340	148856	6-717889	9891017	32	48	1529858	154808	12
8	1414772	142914	6-997180	9899415	52	29	1475217	149153	6-704496	9890588	31	49	1532733	155106	11
9	1417651	143211	6-982678	9899003	51	30	1478094	149451	6-691156	9890159	30	50	1535607	155404	10
10	1420531	143508	6-968233	9898590	50	31	1480971	149748	6-677867	9889728	29	51	1538482	155701	9
11	1423410	143805	6-953847	9898177	49	32	1483848	150045	6-664630	9889297	28	52	1541356	155999	8
12	1426289	144102	6-939519	9897762	48	33	1486724	150343	6-651444	9888865	27	53	1544230	156297	7
13	1429168	144399	6-925248	9897347	47	34	1489601	150640	6-638310	9888432	26	54	1547104	156595	6
14	1432047	144696	6-911035	9896931	46	35	1492477	150938	6-625225	9887998	25	55	1550978	156893	5
15	1434926	144993	6-896879	9896514	45	36	1495353	151235	6-612191	9887564	24	56	1553851	157191	4
16	1437805	145290	6-882780	9896096	44	37	1498230	151533	6-599208	9887128	23	57	1556725	157490	3
17	1440684	145587	6-868737	9895677	43	38	1501106	151830	6-586273	9886692	22	58	1559598	157788	2
18	1443562	145884	6-854750	9895258	42	39	1503981	152128	6-573389	9886255	21	59	1562472	158086	1
19	1446440	146181	6-840819	9894838	41	40	1506857	152426	6-560553	9885817	20	60	1565345	158384	0
20	1449319	146478	6-826943	9894416	40										

Deg. 81.

Deg. 81.

Deg. 81.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

9 Deg.					9 Deg.					9 Deg.				
'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.
0	1564345	158384	6313751	9876883	50	21	1624650	164652	6073397	39	41	1682026	170633	5860505
1	1567218	158682	6301886	9876428	59	22	1627520	164951	6062396	38	42	1684894	170933	5850241
2	1570091	158980	6290065	9875972	58	23	1630390	165250	6051434	37	43	1687761	171232	5840011
3	1572963	159279	6278286	9875514	57	24	1633260	165548	6040510	36	44	1690628	171532	5829817
4	1575836	159577	6266551	9875057	56	25	1636129	165847	6029624	35	45	1693495	171831	5819657
5	1578708	159875	6254858	9874598	55	26	1638999	166146	6018777	34	46	1696362	172130	5809531
6	1581581	160174	6243208	9874138	54	27	1641868	166445	6007967	33	47	1699228	172430	5799440
7	1584453	160472	6231600	9873678	53	28	1644738	166744	5997195	32	48	1702095	172730	5789382
8	1587325	160770	6220034	9873216	52	29	1647607	167043	5986461	31	49	1704961	173029	5779358
9	1590197	161069	6208510	9872754	51	30	1650476	167342	5975764	30	50	1707828	173329	5769368
10	1593069	161367	6197027	9872291	50	31	1653345	167641	5965104	29	51	1710694	173628	5759412
11	1595940	161666	6185586	9871827	49	32	1656214	167940	5954481	28	52	1713560	173928	5749488
12	1598812	161964	6174186	9871363	48	33	1659082	168239	5943895	27	53	1716425	174228	5739598
13	1601683	162263	6162827	9870897	47	34	1661951	168539	5933345	26	54	1719291	174527	5729741
14	1604555	162561	6151508	9870431	46	35	1664819	168838	5922832	25	55	1722156	174827	5719917
15	1607426	162860	6140230	9869964	45	36	1667687	169137	5912355	24	56	1725022	175127	5710125
16	1610297	163159	6128992	9869496	44	37	1670556	169436	5901913	23	57	1727887	175427	5700366
17	1613167	163457	6117794	9869027	43	38	1673423	169735	5891508	22	58	1730752	175727	5690639
18	1616038	163756	6106636	9868557	42	39	1676291	170035	5881138	21	59	1733617	176027	5680944
19	1618909	164055	6095517	9868087	41	40	1679159	170334	5870804	20	60	1736482	176327	5671281
20	1621779	164353	6084438	9867615	40									5661618
'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.

Deg. 80.

Deg. 80.

Deg. 80.



## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

10 Deg.

10 Deg.

10 Deg.

	Sine.	Tang.	Cotang.	Cosine.		Sine.	Tang.	Cotang.	Cosine.		Sine.	Tang.	Cotang.	Cosine.	
0	.1736482	.176327	5.671281	.9848078	60	.21	.1796607	.182632	5.475478	39	.41	.1853808	.188650	5.300801	.9826668
1	.1739346	.176626	5.661650	.9847372	59	.22	.1799169	.182933	5.466481	38	.42	.1856666	.188952	5.292350	.9826128
2	.1742211	.176926	5.652051	.9847066	58	.23	.1802330	.183233	5.457512	37	.43	.1859524	.189253	5.283925	.9825587
3	.1745075	.177226	5.642483	.9846558	57	.24	.1805191	.183534	5.448571	36	.44	.1862382	.189554	5.275525	.9825046
4	.1747939	.177527	5.632917	.9846050	56	.25	.1808052	.183835	5.439659	35	.45	.1865240	.189855	5.267151	.9824504
5	.1750803	.177827	5.623342	.9845542	55	.26	.1810913	.184135	5.430775	34	.46	.1868098	.190157	5.258803	.9823961
6	.1753667	.178127	5.613968	.9845032	54	.27	.1813774	.184436	5.421918	33	.47	.1870956	.190458	5.250480	.9823417
7	.1756531	.178427	5.604524	.9844521	53	.28	.1816635	.184737	5.413090	32	.48	.1873813	.190760	5.242183	.9822873
8	.1759395	.178727	5.595112	.9844010	52	.29	.1819495	.185038	5.404290	31	.49	.1876670	.191061	5.233911	.9822327
9	.1762258	.179027	5.585730	.9843498	51	.30	.1822355	.185339	5.395517	30	.50	.1879528	.191363	5.225664	.9821781
10	.1765121	.179327	5.576378	.9842985	50	.31	.1825215	.185639	5.386771	29	.51	.1882385	.191664	5.217442	.9821234
11	.1767984	.179628	5.567057	.9842471	49	.32	.1828075	.185940	5.378053	28	.52	.1885241	.191966	5.209245	.9820686
12	.1770847	.179928	5.557766	.9841956	48	.33	.1830935	.186241	5.369363	27	.53	.1888098	.192268	5.201073	.9820137
13	.1773710	.180228	5.548505	.9841441	47	.34	.1833795	.186542	5.360699	26	.54	.1890954	.192569	5.192926	.9819587
14	.1776573	.180529	5.539274	.9840924	46	.35	.1836654	.186843	5.352062	25	.55	.1893811	.192871	5.184803	.9819037
15	.1779435	.180829	5.530072	.9840407	45	.36	.1839514	.187144	5.343452	24	.56	.1896667	.193173	5.176705	.9818485
16	.1782298	.181129	5.520900	.9839889	44	.37	.1842373	.187445	5.334869	23	.57	.1899523	.193474	5.168631	.9817933
17	.1785160	.181430	5.511757	.9839370	43	.38	.1845232	.187747	5.326313	22	.58	.1902379	.193776	5.160581	.9817380
18	.1788022	.181730	5.502644	.9838850	42	.39	.1848091	.188048	5.317783	21	.59	.1905234	.194078	5.152555	.9816826
19	.1790884	.182031	5.493560	.9838330	41	.40	.1850949	.188349	5.309279	20	.60	.1908090	.194380	5.144554	.9816272
20	.1793746	.182331	5.484505	.9837808	40										

Deg. 79.

Deg. 79.

Deg. 79.



## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

11 Deg.

11 Deg.

11 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'
0	1908090	194380	5.144554	9816272	60	21	1968018	200727	4.981881	9804433	39	41	2025024	206786	4.835901	9792818	19
1	1910945	194682	5.136576	9815716	59	22	1970870	201030	4.974381	9803860	38	42	2027873	207090	4.828817	9792228	18
2	1913801	194984	5.128622	9815160	58	23	1973722	201332	4.966903	9803286	37	43	2030721	207393	4.821753	9791638	17
3	1916656	195286	5.120692	9814603	57	24	1976573	201635	4.959447	9802712	36	44	2033569	207696	4.814709	9791047	16
4	1919510	195588	5.112785	9814045	56	25	1979425	201938	4.952012	9802136	35	45	2036418	208000	4.807685	9790455	15
5	1922365	195890	5.104902	9813486	55	26	1982276	202240	4.944599	9801560	34	46	2039265	208303	4.800680	9789862	14
6	1925220	196192	5.097042	9812927	54	27	1985127	202543	4.937206	9800983	33	47	2042113	208607	4.793695	9789268	13
7	1928074	196494	5.089206	9812366	53	28	1987978	202846	4.929835	9800405	32	48	2044961	208910	4.786730	9788674	12
8	1930928	196796	5.081392	9811805	52	29	1990829	203149	4.922485	9799827	31	49	2047808	209214	4.779783	9788079	11
9	1933782	197098	5.073602	9811243	51	30	1993679	203452	4.915157	9799247	30	50	2050655	209518	4.772856	9787483	10
10	1936636	197400	5.065835	9810680	50	31	1996530	203755	4.907849	9798667	29	51	2053502	209821	4.765949	9786886	9
11	1939490	197703	5.058090	9810116	49	32	1999380	204058	4.900562	9798086	28	52	2056349	210125	4.759060	9786288	8
12	1942344	198005	5.050369	9809552	48	33	2002230	204361	4.893295	9797504	27	53	2059195	210429	4.752190	9785689	7
13	1945197	198307	5.042670	9808986	47	34	2005080	204664	4.886049	9796921	26	54	2062042	210733	4.745340	9785090	6
14	1948050	198610	5.034993	9808420	46	35	2007930	204967	4.878824	9796337	25	55	2064888	211036	4.738508	9784490	5
15	1950903	198912	5.027339	9807853	45	36	2010779	205270	4.871620	9795752	24	56	2067734	211340	4.731695	9783889	4
16	1953756	199214	5.019707	9807285	44	37	2013629	205573	4.864435	9795167	23	57	2070580	211644	4.714901	9783287	3
17	1956609	199517	5.012098	9806716	43	38	2016478	205876	4.857271	9794581	22	58	2073426	211948	4.718125	9782684	2
18	1959461	199819	5.004511	9806147	42	39	2019327	206180	4.850128	9793994	21	59	2076272	212252	4.711368	9782080	1
19	1962314	200122	4.996945	9805576	41	40	2022176	206483	4.843004	9793406	20	60	2079117	212556	4.704630	9781476	0
20	1965166	200424	4.989402	9805005	40												
'	Cosine.	Cotan.	Tang.	Sine.	'	'	Cosine.	Cotan.	Tang.	Sine.	'	'	Cosine.	Cotan.	Tang.	Sine.	'

Deg. 78.

Deg. 78.

Deg. 78.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

12 Deg.

12 Deg.

12 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'		
0	·2079117	·212556	4·704630	·9781476	60	21	·2138829	·218949	4·567261	·9768393	39	41	·2195624	·225054	4·443376	·9755985	19
1	·2081962	·212860	4·697910	·9780871	59	22	·2141671	·219254	4·560911	·9767970	38	42	·2198462	·225359	4·437350	·9755345	18
2	·2084807	·213164	4·691208	·9780265	58	23	·2144512	·219559	4·554577	·9767347	37	43	·2201300	·225665	4·431339	·9754706	17
3	·2087652	·213468	4·684524	·9779658	57	24	·2147353	·219864	4·548260	·9766723	36	44	·2204137	·225971	4·425343	·9754065	16
4	·2090497	·213773	4·677859	·9779050	56	25	·2150194	·220169	4·541960	·9766098	35	45	·2206974	·226276	4·419364	·9753423	15
5	·2093341	·214077	4·671212	·9778441	55	26	·2153035	·220474	4·535677	·9765472	34	46	·2209811	·226582	4·413399	·9752781	14
6	·2096186	·214381	4·664583	·9777832	54	27	·2155876	·220779	4·529410	·9764845	33	47	·2212648	·226888	4·407450	·9752138	13
7	·2099030	·214685	4·657972	·9777222	53	28	·2158716	·221084	4·523160	·9764217	32	48	·2215485	·227194	4·401516	·9751494	12
8	·2101874	·214990	4·651378	·9776611	52	29	·2161556	·221389	4·516926	·9763589	31	49	·2218321	·227500	4·395597	·9750849	11
9	·2104718	·215294	4·644803	·9775999	51	30	·2164396	·221694	4·510708	·9762960	30	50	·2221158	·227806	4·389694	·9750203	10
10	·2107561	·215589	4·638245	·9775386	50	31	·2167236	·221999	4·504507	·9762330	29	51	·2223994	·228112	4·383805	·9749556	9
11	·2110405	·215903	4·631705	·9774773	49	32	·2170076	·222305	4·498322	·9761699	28	52	·2226830	·228418	4·377931	·9748909	8
12	·2113248	·216207	4·625183	·9774159	48	33	·2172915	·222610	4·492153	·9761067	27	53	·2229666	·228724	4·372073	·9748261	7
13	·2116091	·216512	4·618678	·9773544	47	34	·2175754	·222915	4·486000	·9760435	26	54	·2232501	·229030	4·366229	·9747612	6
14	·2118934	·216816	4·612190	·9772928	46	35	·2178593	·223221	4·479863	·9759802	25	55	·2235337	·229336	4·360400	·9746962	5
15	·2121777	·217121	4·605720	·9772311	45	36	·2181432	·223526	4·473742	·9759168	24	56	·2238172	·229642	4·354586	·9746311	4
16	·2124619	·217425	4·599268	·9771693	44	37	·2184271	·223831	4·467637	·9758533	23	57	·2241007	·229949	4·348786	·9745660	3
17	·2127462	·217730	4·592832	·9771075	43	38	·2187110	·224137	4·461548	·9757897	22	58	·2243842	·230255	4·343001	·9745008	2
18	·2130304	·218035	4·586414	·9770456	42	39	·2189948	·224442	4·455475	·9757260	21	59	·2246676	·230561	4·337231	·9744355	1
19	·2133146	·218340	4·580012	·9769836	41	40	·2192786	·224748	4·449418	·9756623	20	60	·2249511	·230868	4·331475	·9743701	0
20	·2135988	·218644	4·573628	·9769215	40												
'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'		

Deg. 77.

Deg. 77.

Deg. 77.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

13 Deg.

13 Deg.

13 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	.2249311	.230868	4.331475	.9743701	60	.21	.2308989	.237311	4.213869	.9729777	39	41	.2365555	.243465	4.107356
1	.2252345	.231174	4.327374	.9743016	59	22	.2311819	.237618	4.208419	.9729105	38	42	.2368381	.243773	4.102164
2	.2255379	.231481	4.323007	.9742390	58	23	.2314649	.237926	4.202983	.9728432	37	43	.2371207	.244081	4.096985
3	.2258013	.231787	4.318295	.9741734	57	24	.2317479	.238233	4.197560	.9727759	36	44	.2374033	.244390	4.091817
4	.2260846	.232094	4.313859	.9741077	56	25	.2320309	.238541	4.192151	.9727084	35	45	.2376859	.244698	4.086662
5	.2263680	.232400	4.309213	.9740419	55	26	.2323138	.238848	4.186754	.9726409	34	46	.2379684	.245006	4.081519
6	.2266513	.232707	4.304724	.9739760	54	27	.2325967	.239156	4.181371	.9725733	33	47	.2382510	.245315	4.076389
7	.2269346	.233014	4.299244	.9739100	53	28	.2328796	.239463	4.176001	.9725056	32	48	.2385335	.245623	4.071270
8	.2272179	.233320	4.293947	.9738439	52	29	.2331625	.239771	4.170644	.9724378	31	49	.2388159	.245932	4.066164
9	.2275012	.233627	4.288594	.9737778	51	30	.2334454	.240078	4.165299	.9723699	30	50	.2390984	.246240	4.061070
10	.2277844	.233934	4.283219	.9737116	50	31	.2337282	.240386	4.159968	.9723020	29	51	.2393808	.246549	4.055987
11	.2280677	.234241	4.277807	.9736453	49	32	.2340110	.240694	4.154650	.9722339	28	52	.2396633	.246857	4.050917
12	.2283509	.234547	4.272352	.9735789	48	33	.2342938	.241001	4.149344	.9721658	27	53	.2399457	.247166	4.045859
13	.2286341	.234854	4.266950	.9735124	47	34	.2345766	.241309	4.144031	.9720976	26	54	.2402280	.247475	4.040812
14	.2289172	.235161	4.261529	.9734458	46	35	.2348594	.241617	4.138771	.9720294	25	55	.2405104	.247783	4.035777
15	.2292004	.235468	4.256148	.9733792	45	36	.2351421	.241925	4.133504	.9719610	24	56	.2407927	.248092	4.030755
16	.2294835	.235775	4.250750	.9733125	44	37	.2354248	.242233	4.128249	.9718926	23	57	.2410751	.248401	4.025744
17	.2297666	.236082	4.245380	.9732457	43	38	.2357075	.242541	4.123007	.9718240	22	58	.2413574	.248710	4.020744
18	.2300497	.236390	4.239927	.9731789	42	39	.2359902	.242849	4.117778	.9717554	21	59	.2416396	.249019	4.015757
19	.2303328	.236697	4.234808	.9731119	41	40	.2362729	.243157	4.112561	.9716867	20	60	.2419219	.249328	4.010780
20	.2306159	.237004	4.229331	.9730449	40										

Deg. 76.

Deg. 76.

Deg. 76.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

14 Deg.

14 Deg.

14 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'		
0	2419219	249328	4-010780	9702957	60	21	2478445	255826	3-908901	9687998	39	41	2534766	262034	3-816295	9673415	19
1	2422041	249637	4-005816	9702253	59	22	2481263	256136	3-904171	9687277	38	42	2537579	262345	3-811773	9672678	18
2	2424863	249946	4-000863	9701548	58	23	2484081	256446	3-899451	9686555	37	43	2540393	262656	3-807260	9671939	17
3	2427685	250255	3-995922	9700842	57	24	2486899	256756	3-894742	9685832	36	44	2543206	262967	3-802758	9671200	16
4	2430507	250564	3-990992	9700135	56	25	2489716	257066	3-890044	9685108	35	45	2546019	263278	3-798266	9670459	15
5	2433329	250873	3-986073	9699428	55	26	2492533	257376	3-885357	9684383	34	46	2548832	263589	3-793783	9669718	14
6	2436150	251182	3-981166	9698720	54	27	2495350	257686	3-880680	9683658	33	47	2551645	263900	3-789310	9668977	13
7	2438971	251491	3-976271	9698011	53	28	2498167	257997	3-876014	9682931	32	48	2554458	264211	3-784848	9668234	12
8	2441792	251801	3-971386	9697301	52	29	2500984	258307	3-871358	9682204	31	49	2557270	264522	3-780395	9667490	11
9	2444613	252110	3-966513	9696591	51	30	2503800	258617	3-866713	9681476	30	50	2560082	264833	3-775951	9666746	10
10	2447433	252420	3-961651	9695879	50	31	2506616	258928	3-862078	9680748	29	51	2562894	265145	3-771518	9666001	9
11	2450254	252729	3-956801	9695167	49	32	2509432	259238	3-857453	9680018	28	52	2565705	265456	3-767094	9665255	8
12	2453074	253038	3-951961	9694453	48	33	2512248	259548	3-852839	9679288	27	53	2568517	265768	3-762680	9664508	7
13	2455894	253348	3-947133	9693740	47	34	2515063	259859	3-848235	9678557	26	54	2571328	266079	3-758276	9663761	6
14	2458713	253658	3-942315	9693025	46	35	2517879	260169	3-843642	9677825	25	55	2574139	266390	3-753881	9663012	5
15	2461533	253967	3-937509	9692309	45	36	2520694	260480	3-839059	9677092	24	56	2576950	266702	3-749496	9662263	4
16	2464352	254277	3-932714	9691593	44	37	2523508	260791	3-834486	9676358	23	57	2579760	267014	3-745120	9661513	3
17	2467171	254587	3-927929	9690875	43	38	2526323	261101	3-829923	9675624	22	58	2582570	267325	3-740754	9660762	2
18	2469990	254896	3-923156	9690157	42	39	2529137	261412	3-825370	9674888	21	59	2585381	267637	3-736398	9660011	1
19	2472809	255206	3-918393	9689438	41	40	2531952	261723	3-820828	9674152	20	60	2588190	267949	3-732050	9659258	0
20	2475627	255516	3-913642	9688719	40												
'	Cosine.	Tang.	Cotang.	Sine.	'	Cosine.	Tang.	Cotang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'		

Deg. 75.

Deg. 75.

Deg. 75.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

• 15 Deg.

15 Deg.

15 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'
0	2388190	267949	3.732050	9659258	60	21	2647147	274507	3.642891	9643268	39	41	2703204	280773	3.561590	9627704	19
1	2591000	268261	3.727713	9658805	59	22	2649932	274820	3.638744	9642497	38	42	2706004	281087	3.557613	9626917	18
2	2593810	268572	3.723384	9657751	58	23	2652757	275133	3.634606	9641176	37	43	2708805	281401	3.553644	9626130	17
3	2596619	268884	3.719065	9656996	57	24	2655561	275445	3.630477	9640954	36	44	2711605	281715	3.549684	9625342	16
4	2599428	269196	3.714756	9656240	56	25	2658366	275758	3.626356	9640181	35	45	2714404	282029	3.545732	9624552	15
5	2602237	269508	3.710455	9655484	55	26	2661170	276071	3.622244	9639407	34	46	2717204	282343	3.541788	9623762	14
6	2605045	269820	3.706164	9654726	54	27	2663973	276385	3.618141	9638633	33	47	2720003	282657	3.537852	9622972	13
7	2607853	270132	3.701883	9653968	53	28	2666777	276698	3.614046	9637858	32	48	2722802	282971	3.533925	9622180	12
8	2610662	270444	3.697610	9653209	52	29	2669581	277011	3.609960	9637081	31	49	2725601	283285	3.530005	9621387	11
9	2613469	270757	3.693346	9652449	51	30	2672384	277324	3.605883	9636305	30	50	2728400	283599	3.526093	9620594	10
10	2616277	271069	3.689092	9651689	50	31	2675187	277637	3.601814	9635527	29	51	2731198	283914	3.522190	9619800	9
11	2619085	271381	3.684847	9650927	49	32	2677989	277951	3.597754	9634748	28	52	2733997	284228	3.518294	9619005	8
12	2621892	271694	3.680611	9650165	48	33	2680792	278264	3.593702	9633969	27	53	2736794	284543	3.514407	9618210	7
13	2624699	272006	3.676384	9649402	47	34	2683594	278578	3.589659	9633189	26	54	2739592	284857	3.510527	9617413	6
14	2627506	272318	3.672166	9648638	46	35	2686396	278891	3.585624	9632408	25	55	2742390	285172	3.506655	9616616	5
15	2630312	272631	3.667957	9647873	45	36	2689198	279205	3.581597	9631626	24	56	2745187	285486	3.502791	9615818	4
16	2633118	272943	3.663757	9647108	44	37	2692000	279518	3.577579	9630843	23	57	2747984	285801	3.498935	9615019	3
17	2635925	273256	3.659566	9646341	43	38	2694801	279832	3.573569	9630060	22	58	2750781	286115	3.495087	9614219	2
18	2638730	273569	3.655384	9645574	42	39	2697602	280145	3.569568	9629275	21	59	2753577	286430	3.491247	9613418	1
19	2641536	273881	3.651211	9644806	41	40	2700403	280459	3.565574	9628490	20	60	2756374	286745	3.487414	9612617	0
20	2644342	274194	3.647046	9644037	40												

Deg. 74.

Deg. 74.

Deg. 74.

# IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

## FOR RAILROADS.

18

16 Deg.

16 Deg.

16 Deg.

	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	
0	2756374	286745	3487414	9612617	60	21	2815042	293368	3408688	39	41	2870819	299697	3336699	9579060	19
1	2759170	287060	3483589	9611815	59	22	2817833	293683	3405021	38	42	2873605	300014	3333173	9578225	18
2	2761965	287375	3479772	9611012	58	23	2820624	293999	3401361	37	43	2876391	300331	3329654	9577389	17
3	2764761	287690	3475963	9610208	57	24	2823415	294316	3397708	36	44	2879177	300648	3326141	9576552	16
4	2767556	288005	3472161	9609403	56	25	2826205	294632	3394063	35	45	2881963	300965	3322636	9575714	15
5	2770352	288320	3468367	9608598	55	26	2828995	294948	3390424	34	46	2884748	301283	3319137	9574875	14
6	2773147	288635	3464581	9607792	54	27	2831785	295264	3386793	33	47	2887533	301600	3315645	9574035	13
7	2775941	288950	3460802	9606984	53	28	2834575	295580	3383169	32	48	2890318	301917	3312159	9573195	12
8	2778736	289265	3457031	9606177	52	29	2837364	295897	3379553	31	49	2893103	302235	3308681	9572354	11
9	2781530	289580	3453267	9605368	51	30	2840153	296213	3375943	30	50	2895887	302552	3305209	9571512	10
10	2784324	289896	3449512	9604558	50	31	2842942	296529	3372340	29	51	2898671	302870	3301743	9570669	9
11	2787118	290211	3445763	9603748	49	32	2845731	296846	3368745	28	52	2901455	303187	3298285	9569825	8
12	2789911	290526	3442022	9602937	48	33	2848520	297163	3365156	27	53	2904239	303505	3294833	9568981	7
13	2792704	290842	3438289	9602125	47	34	2851308	297479	3361575	26	54	2907022	303823	3291387	9568136	6
14	2795497	291157	3434563	9601312	46	35	2854096	297796	3358000	25	55	2909805	304141	3287948	9567290	5
15	2798290	291473	3430844	9600499	45	36	2856884	298112	3354433	24	56	2912588	304458	3284516	9566443	4
16	2801083	291789	3427133	9599684	44	37	2859671	298429	3350872	23	57	2915371	304776	3281090	9565595	3
17	2803875	292104	3423429	9598869	43	38	2862458	298746	3347319	22	58	2918153	305094	3277671	9564747	2
18	2806667	292420	3419733	9598053	42	39	2865246	299063	3343772	21	59	2920935	305412	3274258	9563898	1
19	2809459	292736	3416044	9597236	41	40	2868032	299380	3340232	20	60	2923717	305730	3270852	9563048	0
20	2812251	293052	3412362	9596418	40											
	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	

Deg. 73.

Deg. 73.

Deg. 73.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

17 Deg.

17 Deg.

17 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	.2923717	.305730	3.270852	.9563048	60	.21 .2982079	.312422	3.200789	.9545009	39	.41 .3037559	.318820	3.136563	.9527499	19
1	.2926499	.306048	3.267432	.9562197	59	.22 .2984856	.312742	3.197521	.9544141	38	.42 .3040331	.319140	3.133414	.9526615	18
2	.2929280	.306367	3.264059	.9561345	58	.23 .2987632	.313061	3.194239	.9543273	37	.43 .3043102	.319461	3.130270	.9525730	17
3	.2932061	.306685	3.260672	.9560492	57	.24 .2990408	.313381	3.191003	.9542403	36	.44 .3045872	.319781	3.127131	.9524844	16
4	.2934842	.307003	3.257292	.9559639	56	.25 .2993184	.313700	3.187754	.9541533	35	.45 .3048643	.320102	3.123999	.9523958	15
5	.2937623	.307321	3.253918	.9558785	55	.26 .2995959	.314020	3.184310	.9540662	34	.46 .3051413	.320423	3.120872	.9523071	14
6	.2940403	.307640	3.250550	.9557930	54	.27 .2998734	.314339	3.181272	.9539790	33	.47 .3054183	.320744	3.117750	.9522183	13
7	.2943183	.307958	3.247189	.9557074	53	.28 .3001509	.314659	3.178040	.9538917	32	.48 .3056953	.321064	3.114635	.9521294	12
8	.2945963	.308277	3.243834	.9556218	52	.29 .3004284	.314979	3.174814	.9538044	31	.49 .3059723	.321385	3.111525	.9520404	11
9	.2948743	.308595	3.240486	.9555361	51	.30 .3007058	.315298	3.171594	.9537170	30	.50 .3062492	.321706	3.108421	.9519514	10
10	.2951522	.308914	3.237143	.9554502	50	.31 .3009832	.315618	3.168380	.9536294	29	.51 .3065261	.322027	3.105322	.9518623	9
11	.2954302	.309233	3.233807	.9553643	49	.32 .3012606	.315938	3.165172	.9535418	28	.52 .3068030	.322348	3.102229	.9517731	8
12	.2957081	.309551	3.230478	.9552784	48	.33 .3015380	.316258	3.161970	.9534542	27	.53 .3070798	.322670	3.099141	.9516838	7
13	.2959859	.309870	3.227154	.9551923	47	.34 .3018153	.316578	3.158774	.9533664	26	.54 .3073566	.322991	3.096039	.9515944	6
14	.2962638	.310189	3.223837	.9551062	46	.35 .3020926	.316898	3.155584	.9532786	25	.55 .3076334	.323312	3.092983	.9515050	5
15	.2965416	.310508	3.220526	.9550199	45	.36 .3023699	.317218	3.152399	.9531907	24	.56 .3079102	.323633	3.089912	.9514154	4
16	.2968194	.310827	3.217221	.9549336	44	.37 .3026471	.317538	3.149220	.9531027	23	.57 .3081869	.323955	3.086846	.9513258	3
17	.2970971	.311146	3.213922	.9548473	43	.38 .3029244	.317859	3.146047	.9530146	22	.58 .3084636	.324276	3.083786	.9512361	2
18	.2973749	.311465	3.210630	.9547608	42	.39 .3032016	.318179	3.142880	.9529264	21	.59 .3087403	.324598	3.080732	.9511464	1
19	.2976526	.311784	3.207344	.9546743	41	.40 .3034788	.318499	3.139719	.9528382	20	.60 .3090170	.324919	3.077683	.9510565	0
20	.2979303	.312103	3.204063	.9545876	40										

Deg. 72.

Deg. 72.

Deg. 72.



## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

18 Deg.

18 Deg.

18 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	3090170	324919	3-077683	9500565	60	21	3148209	331686	3-014892	9491511	39	41	3203374	338157	19
1	3092936	325241	3-074640	9509666	59	22	3150969	332009	3-011960	9490595	38	42	3206130	338481	18
2	3095702	325563	3-071602	9508766	58	23	3153730	332332	3-009033	9489678	37	43	3208885	338805	17
3	3098468	325884	3-068569	9507865	57	24	3156490	332655	3-006110	9488760	36	44	3211640	339129	16
4	3101234	326206	3-065542	9506963	56	25	3159250	332978	3-003193	9487842	35	45	3214395	339454	15
5	3103999	326528	3-062520	9506061	55	26	3162010	333302	3-000282	9486922	34	46	3217149	339778	14
6	3106764	326850	3-059503	9505157	54	27	3164770	333625	2-997375	9486002	33	47	3219903	340103	13
7	3109529	327172	3-056492	9504253	53	28	3167529	333948	2-994473	9485081	32	48	3222657	340427	12
8	3112294	327494	3-053487	9503348	52	29	3170288	334271	2-991576	9484159	31	49	3225411	340752	11
9	3115058	327816	3-050486	9502443	51	30	3173047	334595	2-988685	9483237	30	50	3228164	341077	10
10	3117822	328138	3-047491	9501536	50	31	3175805	334918	2-985798	9482313	29	51	3230917	341401	9
11	3120586	328461	3-044501	9500629	49	32	3178563	335242	2-982916	9481389	28	52	3233670	341726	8
12	3123349	328783	3-041517	9499721	48	33	3181321	335566	2-980040	9480464	27	53	3236422	342051	7
13	3126112	329105	3-038538	9498812	47	34	3184079	335889	2-977168	9479538	26	54	3239174	342376	6
14	3128875	329428	3-035564	9497902	46	35	3186836	336213	2-974301	9478612	25	55	3241926	342701	5
15	3131638	329750	3-032595	9496991	45	36	3189593	336537	2-971439	9477684	24	56	3244678	343026	4
16	3134400	330073	3-029632	9496080	44	37	3192350	336861	2-968583	9476756	23	57	3247429	343351	3
17	3137163	330395	3-026673	9495168	43	38	3195106	337185	2-965731	9475827	22	58	3250180	343677	2
18	3139925	330718	3-023720	9494255	42	39	3197863	337509	2-962884	9474897	21	59	3252931	344002	1
19	3142686	331041	3-020772	9493341	41	40	3200619	337833	2-960042	9473966	20	60	3255682	344327	0
20	3145448	331363	3-017830	9492426	40										
'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'

Deg. 71.

Deg. 71.

Deg. 71.





## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

20 Deg.

20 Deg.

20 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	.3420201	.363970	2.747477	.9396926	60	.21	.3477540	.370903	.2696118	.9375858	39	41	.3532027	.377536	2.648753
1	.3422935	.364299	2.744992	.9395931	59	22	.3480267	.371234	.2693714	.9374846	38	42	.3534748	.377868	2.646423
2	.3425668	.364629	2.742512	.9394935	58	23	.3482994	.371565	.2691314	.9373833	37	43	.3537469	.378201	2.644096
3	.3428400	.364958	2.740035	.9393938	57	24	.3485720	.371896	.2688919	.9372820	36	44	.3540190	.378533	2.641774
4	.3431133	.365288	2.737562	.9392940	56	25	.3488447	.372227	.2686526	.9371806	35	45	.3542910	.378866	2.639454
5	.3433865	.365618	2.735093	.9391942	55	26	.3491173	.372559	.2684138	.9370790	34	46	.3545630	.379198	2.637139
6	.3436597	.365948	2.732628	.9390943	54	27	.3493898	.372890	.2681753	.9369774	33	47	.3548350	.379531	2.634827
7	.3439329	.366277	2.730167	.9389943	53	28	.3496624	.373221	.2679372	.9368758	32	48	.3551070	.379864	2.632518
8	.3442060	.366607	2.727710	.9388942	52	29	.3499349	.373553	.2676995	.9367740	31	49	.3553789	.380197	2.630213
9	.3444791	.366937	2.725256	.9387940	51	30	.3502074	.373884	.2674621	.9366722	30	50	.3556508	.380530	2.627912
10	.3447521	.367268	2.722807	.9386938	50	31	.3504798	.374216	.2672251	.9365703	29	51	.3559226	.380863	2.625614
11	.3450252	.367598	2.720362	.9385934	49	32	.3507523	.374547	.2669885	.9364683	28	52	.3561944	.381196	2.623319
12	.3452982	.367928	2.717920	.9384930	48	33	.3510246	.374879	.2667522	.9363662	27	53	.3564662	.381529	2.621028
13	.3455712	.368258	2.715482	.9383925	47	34	.3512970	.375211	.2665163	.9362641	26	54	.3567380	.381862	2.618741
14	.3458441	.368589	2.713048	.9382920	46	35	.3515693	.375543	.2662808	.9361618	25	55	.3570097	.382196	2.616457
15	.3461171	.368919	2.710618	.9381913	45	36	.3518416	.375875	.2660456	.9360595	24	56	.3572814	.382529	2.614176
16	.3463900	.369250	2.708192	.9380906	44	37	.3521139	.376207	.2658108	.9359571	23	57	.3575531	.382863	2.611899
17	.3466628	.369580	2.705769	.9379898	43	38	.3523862	.376539	.2655764	.9358547	22	58	.3578248	.383196	2.609625
18	.3469357	.369911	2.703351	.9378889	42	39	.3526584	.376871	.2653423	.9357521	21	59	.3580964	.383530	2.607355
19	.3472085	.370242	2.700936	.9377880	41	40	.3529306	.377203	.2651086	.9356495	20	60	.3583679	.383864	2.605089
20	.3474812	.370572	2.698525	.9376869	40										2.602818
'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'

Deg. 69.

Deg. 69.

Deg. 69.

IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—*continued*.

21 Deg.

21 Deg.

21 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'
0	3583679	383864	2.605089	9335804	60	21	3640641	390889	2.558268	9313739	39	41	3694765	397611	2.515018	9291401	19
1	3586393	384197	2.602825	9334761	59	22	3643351	391224	2.556075	9312679	38	42	3697468	397948	2.512889	9291326	18
2	3589110	384531	2.600565	9333718	58	23	3646059	391560	2.553885	9311619	37	43	3700170	398285	2.510762	9290250	17
3	3591825	384865	2.598309	9332673	57	24	3648768	391895	2.551699	9310558	36	44	3702872	398622	2.508639	9289173	16
4	3594540	385199	2.596056	9331628	56	25	3651476	392231	2.549516	9309496	35	45	3705574	398959	2.506519	9288096	15
5	3597254	385533	2.593806	9330582	55	26	3654184	392567	2.547335	9308434	34	46	3708276	399296	2.504402	9287017	14
6	3599968	385867	2.591560	9329535	54	27	3656891	392902	2.545159	9307370	33	47	3710977	399634	2.502289	9285938	13
7	3602682	386202	2.589317	9328488	53	28	3659599	393238	2.542985	9306306	32	48	3713678	399971	2.500178	9284858	12
8	3605395	386536	2.587078	9327439	52	29	3662306	393574	2.540815	9305241	31	49	3716379	400308	2.498070	9283778	11
9	3608108	386870	2.584842	9326390	51	30	3665012	393910	2.538647	9304176	30	50	3719079	400646	2.495966	9282696	10
10	3610821	387205	2.582609	9325340	50	31	3667719	394246	2.536483	9303109	29	51	3721780	400984	2.493864	9281614	9
11	3613534	387539	2.580380	9324290	49	32	3670425	394582	2.534323	9302042	28	52	3724479	401321	2.491766	9280531	8
12	3616246	387874	2.578153	9323238	48	33	3673130	394918	2.532165	9300974	27	53	3727179	401659	2.489670	9279447	7
13	3618958	388209	2.575931	9322186	47	34	3675836	395255	2.530011	9299905	26	54	3729878	401997	2.487578	9278363	6
14	3621669	388543	2.573711	9321133	46	35	3678541	395591	2.527859	9298835	25	55	3732577	402335	2.485488	9277277	5
15	3624380	388878	2.571495	9320079	45	36	3681246	395928	2.525711	9297765	24	56	3735275	402673	2.483402	9276191	4
16	3627091	389213	2.569283	9319024	44	37	3683950	396264	2.523566	9296694	23	57	3737973	403011	2.481319	9275104	3
17	3629802	389548	2.567073	9317969	43	38	3686654	396601	2.521424	9295622	22	58	3740671	403349	2.479238	9274016	2
18	3632512	389883	2.564867	9316912	42	39	3689358	396937	2.519286	9294549	21	59	3743369	403687	2.477161	9272928	1
19	3635222	390218	2.562664	9315855	41	40	3692061	397274	2.517150	9293475	20	60	3746066	404026	2.475086	9271839	0
20	3637932	390554	2.560464	9314797	40												
'	Cosine.	Cotan.	Tang.	Sine.	'	'	Cosine.	Cotan.	Tang.	Sine.	'	'	Cosine.	Cotan.	Tang.	Sine.	'

Deg. 68.

Deg. 68.

Deg. 68.

IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

FOR RAILROADS.

193

22 Deg.				22 Deg.				22 Deg.				
'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	
0	.3746066	.404026	2.475086	.9271839	60	21	.3802634	.411149	2.432204	.9248782	39	41
1	.3748763	.404364	2.473015	.9270748	59	22	.3805324	.411489	2.430193	.9247676	38	42
2	.3751459	.404703	2.470947	.9269658	58	23	.3808014	.411830	2.428186	.9246568	37	43
3	.3754156	.405041	2.468881	.9268566	57	24	.3810704	.412170	2.426181	.9245460	36	44
4	.3756852	.405380	2.466819	.9267474	56	25	.3813393	.412510	2.424180	.9244351	35	45
5	.3759547	.405719	2.464759	.9266380	55	26	.3816082	.412851	2.422181	.9243242	34	46
6	.3762243	.406057	2.462703	.9265286	54	27	.3818770	.413191	2.420185	.9242131	33	47
7	.3764938	.406396	2.460649	.9264192	53	28	.3821459	.413532	2.418191	.9241020	32	48
8	.3767632	.406735	2.458598	.9263096	52	29	.3824147	.413872	2.416201	.9239908	31	49
9	.3770327	.407074	2.456551	.9262000	51	30	.3826834	.414213	2.414213	.9238795	30	50
10	.3773021	.407413	2.454506	.9260902	50	31	.3829522	.414554	2.412228	.9237682	29	51
11	.3775714	.407753	2.452464	.9259805	49	32	.3832209	.414895	2.410246	.9236567	28	52
12	.3778408	.408092	2.450425	.9258706	48	33	.3834895	.415236	2.408267	.9235452	27	53
13	.3781101	.408431	2.448389	.9257606	47	34	.3837582	.415577	2.406290	.9234336	26	54
14	.3783794	.408771	2.446355	.9256506	46	35	.3840268	.415918	2.404316	.9233220	25	55
15	.3786486	.409110	2.444325	.9255405	45	36	.3842953	.416259	2.402345	.9232102	24	56
16	.3789178	.409450	2.442298	.9254303	44	37	.3845639	.416601	2.400377	.9230984	23	57
17	.3791870	.409790	2.440273	.9253201	43	38	.3848324	.416942	2.398411	.9229865	22	58
18	.3794562	.410129	2.438251	.9252097	42	39	.3851008	.417284	2.396449	.9228745	21	59
19	.3797253	.410469	2.436233	.9250993	41	40	.3853693	.417625	2.394488	.9227624	20	60
20	.3799944	.410809	2.434217	.9249888	40							
'	Cosine.	Cotan.	Tang.	Sine.	'	'	Cosine.	Cotan.	Tang.	Sine.	'	'

Deg. 67.

Deg. 67.

Deg. 67.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

23 Deg.

23 Deg.

23 Deg

	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'
0	.3907311	.424474	2.355852	.9205049	60	21	.3963468	.431703	2.316407	.9181009	39	41	.4016814	.438622	2.279865	.9157795	19
1	.3909989	.424818	2.353948	.9203912	59	22	.3966613	.432048	2.314557	.9179855	38	42	.4019478	.438969	2.278063	.9156626	18
2	.3912666	.425161	2.352046	.9202774	58	23	.3969809	.432393	2.312709	.9178701	37	43	.4022141	.439316	2.276264	.9155456	17
3	.3915343	.425505	2.350148	.9201635	57	24	.3971479	.432738	2.310863	.9177546	36	44	.4024804	.439663	2.274467	.9154286	16
4	.3918019	.425848	2.348251	.9200496	56	25	.3974148	.433084	2.309020	.9176391	35	45	.4027467	.440010	2.272672	.9153115	15
5	.3920695	.426192	2.346358	.9199356	55	26	.3976818	.433429	2.307180	.9175234	34	46	.4030129	.440357	2.270880	.9151943	14
6	.3923371	.426536	2.344467	.9198215	54	27	.3979486	.433775	2.305342	.9174077	33	47	.4032791	.440705	2.269090	.9150770	13
7	.3926047	.426880	2.342578	.9197073	53	28	.3982155	.434120	2.303506	.9172919	32	48	.4035453	.441052	2.267303	.9149597	12
8	.3928722	.427223	2.340692	.9195931	52	29	.3984823	.434466	2.301673	.9171760	31	49	.4038114	.441400	2.265518	.9148422	11
9	.3931397	.427568	2.338809	.9194788	51	30	.3987491	.434812	2.299842	.9170601	30	50	.4040775	.441747	2.263735	.9147247	10
10	.3934071	.427912	2.336928	.9193644	50	31	.3990158	.435158	2.298014	.9169440	29	51	.4043436	.442095	2.261955	.9146072	9
11	.3936745	.428256	2.335050	.9192499	49	32	.3992825	.435504	2.296188	.9168279	28	52	.4046096	.442443	2.260177	.9144895	8
12	.3939419	.428600	2.333174	.9191353	48	33	.3995492	.435850	2.294365	.9167118	27	53	.4048756	.442791	2.258401	.9143718	7
13	.3942093	.428944	2.331301	.9190207	47	34	.3998158	.436196	2.292544	.9165955	26	54	.4051416	.443139	2.256628	.9142540	6
14	.3944766	.429289	2.329431	.9189060	46	35	.4000825	.436542	2.290725	.9164791	25	55	.4054075	.443487	2.254857	.9141361	5
15	.3947439	.429633	2.327563	.9187912	45	36	.4003490	.436889	2.288909	.9163627	24	56	.4056734	.443835	2.253088	.9140181	4
16	.3950111	.429978	2.325697	.9186763	44	37	.4006156	.437235	2.287095	.9162462	23	57	.4059393	.444183	2.251322	.9139001	3
17	.3952783	.430323	2.323834	.9185614	43	38	.4008821	.437582	2.285284	.9161297	22	58	.4062051	.444531	2.249558	.9137819	2
18	.3955455	.430668	2.321974	.9184464	42	39	.4011486	.437928	2.283475	.9160130	21	59	.4064709	.444880	2.247796	.9136637	1
19	.3958127	.431012	2.320116	.9183313	41	40	.4014150	.438275	2.281669	.9158963	20	50	.4067366	.445228	2.246036	.9135455	0
20	.3960798	.431357	2.318260	.9182161	40												

Deg. 66.

Deg. 66.

Deg. 66.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

24 Deg.

24 Deg.

24 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	.4087366	.445228	2.246036	.9135455	60	21	.4123096	.452568	2.209611	.9110438	39	41	.4176028	.459596	2.175822
1	.4070024	.445577	2.244279	.9134271	59	22	.4125745	.452918	2.207901	.9109238	38	42	.4178671	.459948	2.174155
2	.4072681	.445926	2.242524	.9133087	58	23	.4128395	.453269	2.206193	.9108038	37	43	.4181313	.460301	2.172491
3	.4075337	.446274	2.240772	.9131902	57	24	.4131044	.453620	2.204487	.9106837	36	44	.4183956	.460653	2.170828
4	.4077993	.446623	2.239021	.9130716	56	25	.4133693	.453970	2.202784	.9105635	35	45	.4186597	.461006	2.169167
5	.4080649	.446972	2.237273	.9129529	55	26	.4136342	.454321	2.201083	.9104432	34	46	.4189239	.461359	2.167509
6	.4083305	.447321	2.235528	.9128342	54	27	.4138990	.454672	2.199384	.9103228	33	47	.4191880	.461711	2.165852
7	.4085960	.447670	2.233784	.9127154	53	28	.4141638	.455023	2.197687	.9102024	32	48	.4194521	.462064	2.164198
8	.4088615	.448020	2.232043	.9125965	52	29	.4144285	.455375	2.195992	.9100819	31	49	.4197161	.462417	2.162546
9	.4091269	.448369	2.230304	.9124775	51	30	.4146932	.455726	2.194299	.9099613	30	50	.4199801	.462771	2.160895
10	.4093923	.448718	2.228567	.9123584	50	31	.4149579	.456077	2.192609	.9098406	29	51	.4202441	.463124	2.159247
11	.4096577	.449068	2.226833	.9122393	49	32	.4152226	.456429	2.190921	.9097199	28	52	.4205080	.463477	2.157601
12	.4099230	.449417	2.225100	.9121201	48	33	.4154872	.456780	2.189234	.9095990	27	53	.4207719	.463831	2.155957
13	.4101883	.449767	2.223370	.9120008	47	34	.4157517	.457132	2.187551	.9094781	26	54	.4210358	.464184	2.154315
14	.4104536	.450117	2.221633	.9118815	46	35	.4160163	.457483	2.185869	.9093572	25	55	.4212966	.464538	2.152675
15	.4107189	.450467	2.219917	.9117620	45	36	.4162808	.457835	2.184189	.9092361	24	56	.4215534	.464891	2.151037
16	.4109841	.450817	2.218194	.9116425	44	37	.4165453	.458187	2.182511	.9091150	23	57	.4218122	.465245	2.149402
17	.4112492	.451167	2.216473	.9115229	43	38	.4168097	.458539	2.180836	.9089938	22	58	.4220909	.465599	2.147768
18	.4115144	.451517	2.214754	.9114033	42	39	.4170741	.458891	2.179163	.9088725	21	59	.4223546	.465953	2.146136
19	.4117795	.451867	2.213037	.9112835	41	40	.4173385	.459243	2.177492	.9087511	20	60	.4226183	.466307	2.144506
20	.4120445	.452217	2.211323	.9111637	40										
'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'

Deg. 65.

Deg. 65.

Deg. 65.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

• 25 Deg.

25 Deg.

25 Deg.

	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	4226183	466307	2144506	9063078	60	21	4281467	473765	2110747	9037093	39	41	4333970	480909	2079394
1	4228819	466661	2142879	9061848	59	22	4284095	474222	2109161	9035847	38	42	4336591	481267	2077846
2	4231455	467016	2141253	9060618	58	23	4286723	474748	2107577	9034600	37	43	4339212	481625	2076300
3	4234090	467370	2139630	9059386	57	24	4289351	475274	2105995	9033353	36	44	4341832	481984	2074756
4	4236725	467725	2138008	9058154	56	25	4291979	475791	2104415	9032105	35	45	4344453	482342	2073214
5	4239360	468079	2136389	9056922	55	26	4294606	476308	2102836	9030856	34	46	4347072	482701	2071674
6	4241994	468434	2134771	9055688	54	27	4297233	476825	2101260	9029606	33	47	4349692	483060	2070135
7	4244628	468789	2133155	9054454	53	28	4299859	477342	2099686	9028356	32	48	4352311	483418	2068599
8	4247262	469143	2131542	9053219	52	29	4302485	477859	2098114	9027105	31	49	4354930	483777	2067064
9	4249895	469498	2129930	9051983	51	30	4305111	478375	2096543	9025853	30	50	4357548	484136	2065531
10	4252528	469853	2128321	9050746	50	31	4307736	478892	2094975	9024600	29	51	4360166	484495	2064000
11	4255161	470209	2126713	9049509	49	32	4310361	479409	2093408	9023347	28	52	4362784	484855	2062471
12	4257793	470564	2125108	9048271	48	33	4312986	479926	2091843	9022092	27	53	4365401	485214	2060944
13	4260425	470919	2123504	9047032	47	34	4315610	480443	2090280	9020838	26	54	4368018	485573	2059418
14	4263056	471275	2121903	9045792	46	35	4318234	480960	2088720	9019582	25	55	4370634	485933	2057895
15	4265687	471630	2120303	9044551	45	36	4320857	481477	2087161	9018325	24	56	4373251	486293	2056373
16	4268318	471986	2118705	9043310	44	37	4323481	481994	2085603	9017068	23	57	4375866	486652	2054853
17	4270949	472342	2117110	9042068	43	38	4326103	482511	2084048	9015810	22	58	4378482	487012	2053334
18	4273579	472697	2115516	9040825	42	39	4328726	483028	2082495	9014551	21	59	4381097	487372	2051818
19	4276208	473053	2113924	9039582	41	40	4331348	483545	2080943	9013292	20	60	4383711	487732	2050303
20	4278838	473409	2112334	9038338	40										8987940

Deg. 64.

Deg. 64.

Deg. 64.



IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

26 Deg.

26 Deg.

26 Deg.

	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	4383711	487732	2-050303	8987940	60	21	4435334	495317	2-018908	8900994	39	41	4490591	502583	19
1	4386326	488092	2-048791	8986665	59	22	4441140	495679	2-017433	8959703	38	42	4493190	502947	18
2	4388940	488453	2-047280	8985389	58	23	444746	496041	2-015959	8958411	37	43	4495789	503312	17
3	4391553	488813	2-045770	8984112	57	24	445352	496404	2-014486	8957118	36	44	4498387	503676	16
4	4394166	489173	2-044263	8982834	56	25	445957	496766	2-013016	8955824	35	45	4500984	504041	15
5	4396779	489534	2-042757	8981555	55	26	446562	497129	2-011547	8954529	34	46	4503582	504406	14
6	4399392	489894	2-041254	8980276	54	27	447167	497492	2-010080	8953234	33	47	4506179	504771	13
7	4402004	490255	2-039751	8978996	53	28	447771	497855	2-008615	8951938	32	48	4508775	505136	12
8	4404615	490616	2-038251	8977715	52	29	448375	498218	2-007151	8950641	31	49	4511372	505501	11
9	4407227	490977	2-036753	8976433	51	30	448978	498581	2-005689	8949344	30	50	4513962	505866	10
10	4409838	491338	2-035256	8975151	50	31	449581	498944	2-004229	8948045	29	51	4516563	506232	9
11	4412448	491699	2-033761	8973868	49	32	4467184	499308	2-002771	8946746	28	52	4519158	506597	8
12	4415059	492061	2-032268	8972584	48	33	4463786	499671	2-001314	8945446	27	53	4521753	506963	7
13	4417668	492422	2-030776	8971299	47	34	4472388	500035	1-999859	8944146	26	54	4524347	507329	6
14	4420278	492783	2-029287	8970014	46	35	4477990	500398	1-998405	8942844	25	55	4526941	507694	5
15	4422887	493145	2-027799	8968727	45	36	4477591	500762	1-996953	8941542	24	56	4529535	508060	4
16	4425496	493507	2-026313	8967440	44	37	4480192	501126	1-995503	8940240	23	57	4532128	508426	3
17	4428104	493868	2-024828	8966153	43	38	4482792	501490	1-994055	8938936	22	58	4534721	508792	2
18	4430712	494230	2-023346	8964864	42	39	4485392	501854	1-992608	8937632	21	59	4537313	509159	1
19	4433319	494592	2-021865	8963575	41	40	4487992	502218	1-991163	8936326	20	60	4539905	509525	0
20	4435927	494954	2-020386	8962285	40										

Deg. 63.

Deg. 63.

Deg. 63.



## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

27 Deg.				27 Deg.				27 Deg.				
'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	'	
0	4539905	509525	1.962610	8910065	60	21	4594248	517244	1.933323	8882166	39	41
1	4542497	509891	1.961200	8908744	59	22	4596832	517612	1.931945	8880830	38	42
2	4545088	510258	1.959791	8907423	58	23	4599415	517981	1.930569	8878492	37	43
3	4547679	510625	1.958383	8906100	57	24	4601998	518350	1.929195	8879154	36	44
4	4550269	510991	1.956978	8904777	56	25	4604580	518719	1.927822	8876815	35	45
5	4552859	511358	1.955573	8903453	55	26	4607162	519089	1.926451	8875475	34	46
6	4555449	511725	1.954171	8902128	54	27	4609744	519458	1.925081	8874134	33	47
7	4558038	512093	1.952770	8900803	53	28	4612325	519827	1.923713	8872793	32	48
8	4560627	512460	1.951371	8899476	52	29	4614906	520197	1.922347	8871451	31	49
9	4563216	512827	1.949973	8898149	51	30	4617486	520567	1.920982	8870108	30	50
10	4565804	513195	1.948577	8896822	50	31	4620066	520936	1.919618	8868765	29	51
11	4568392	513562	1.947182	8895493	49	32	4622646	521306	1.918256	8867420	28	52
12	4570979	513930	1.945789	8894164	48	33	4625225	521676	1.916896	8866075	27	53
13	4573566	514298	1.944398	8892834	47	34	4627804	522046	1.915537	8864730	26	54
14	4576153	514666	1.943008	8891503	46	35	4630382	522417	1.914179	8863383	25	55
15	4578739	515033	1.941620	8890171	45	36	4632960	522787	1.912823	8862036	24	56
16	4581325	515401	1.940233	8888839	44	37	4635538	523157	1.911469	8860688	23	57
17	4583910	515770	1.938848	8887506	43	38	4638115	523528	1.910116	8859339	22	58
18	4586496	516138	1.937464	8886172	42	39	4640692	523899	1.908764	8857989	21	59
19	4589080	516506	1.936082	8884838	41	40	4643269	524269	1.907414	8856639	20	60
20	4591665	516875	1.934702	8883503	40							
27 Deg.				27 Deg.				27 Deg.				
'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	'	
	8855288	1.906066	5.24640	4645845	19				4645845	1.906066	5.24640	
	8853936	1.904719	5.25011	4648420	18				4648420	1.904719	5.25011	
	8852584	1.903373	5.25382	4650996	17				4650996	1.903373	5.25382	
	8851230	1.902029	5.25754	4653571	16				4653571	1.902029	5.25754	
	8849876	1.900687	5.26125	4656145	15				4656145	1.900687	5.26125	
	8848522	1.899346	5.26496	4658719	14				4658719	1.899346	5.26496	
	8847166	1.898006	5.26868	4661293	13				4661293	1.898006	5.26868	
	8844453	1.896668	5.27240	4663866	12				4663866	1.896668	5.27240	
	8841736	1.895332	5.27612	4666439	11				4666439	1.895332	5.27612	
	8839017	1.893997	5.27983	4669012	10				4669012	1.893997	5.27983	
	8836295	1.892663	5.28356	4671584	9				4671584	1.892663	5.28356	
	8833569	1.891331	5.28728	4674156	8				4674156	1.891331	5.28728	
	8830841	1.890000	5.29100	4676727	7				4676727	1.890000	5.29100	
	8828117	1.888671	5.29472	4679298	6				4679298	1.888671	5.29472	
	8825393	1.887343	5.29845	4681869	5				4681869	1.887343	5.29845	
	8822669	1.886017	5.30217	4684439	4				4684439	1.886017	5.30217	
	8819945	1.884692	5.30590	4687009	3				4687009	1.884692	5.30590	
	8817221	1.883369	5.30963	4689578	2				4689578	1.883369	5.30963	
	8814497	1.882047	5.31336	4692147	1				4692147	1.882047	5.31336	
	8811773	1.880726	5.31709	4694716	0				4694716	1.880726	5.31709	

Deg. 62.

Deg. 62.

Deg. 62.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

28 Deg.

28 Deg.

28 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	
0	•4694716	•531709	1•880726	•8829476	60	21	•4748564	•539370	•1853325	39	41	•4799683	•547106	1•827799	•8772858	19
1	•4697284	•532082	1•879407	•8828110	59	22	•4751124	•539946	•1852035	38	42	•4802235	•547484	1•826537	•8771462	18
2	•4699852	•532455	1•878089	•8826743	58	23	•4753683	•540322	•1850747	37	43	•4804786	•547862	1•825276	•8770064	17
3	•4702419	•532829	1•876773	•8825376	57	24	•4756242	•540698	•1849461	36	44	•4807337	•548240	1•824017	•8768666	16
4	•4704986	•533202	1•875458	•8824007	56	25	•4758801	•541074	•1848176	35	45	•4809888	•548618	1•822759	•8767268	15
5	•4707553	•533576	1•874145	•8822638	55	26	•4761359	•541450	•1846892	34	46	•4812438	•548997	1•821502	•8765868	14
6	•4710119	•533950	1•872833	•8821269	54	27	•4763917	•541826	•1845609	33	47	•4814987	•549375	1•820247	•8764468	13
7	•4712685	•534324	1•871523	•8819898	53	28	•4766474	•542202	•1844328	32	48	•4817537	•549754	1•818993	•8763067	12
8	•4715250	•534698	1•870214	•8818527	52	29	•4769031	•542579	•1843049	31	49	•4820086	•550133	1•817740	•8761665	11
9	•4717815	•535072	1•868906	•8817155	51	30	•4771588	•542955	•1841770	30	50	•4822634	•550512	1•816489	•8760263	10
10	•4720380	•535446	1•867600	•8815782	50	31	•4774144	•543332	•1840494	29	51	•4825182	•550891	1•815239	•8758859	9
11	•4722944	•535820	1•866295	•8814409	49	32	•4776700	•543709	•1839218	28	52	•4827730	•551270	1•813990	•8757455	8
12	•4725508	•536195	1•864992	•8813035	48	33	•4779255	•544086	•1837944	27	53	•4830277	•551650	1•812743	•8756051	7
13	•4728071	•536569	1•863690	•8811660	47	34	•4781810	•544463	•1836671	26	54	•4832824	•552029	1•811496	•8754645	6
14	•4730634	•536944	1•862389	•8810284	46	35	•4784364	•544840	•1835399	25	55	•4835370	•552409	1•810252	•8753239	5
15	•4733197	•537319	1•861090	•8808907	45	36	•4786919	•545217	•1834129	24	56	•4837916	•552789	1•809008	•8751832	4
16	•4735759	•537694	1•859792	•8807530	44	37	•4789472	•545595	•1832861	23	57	•4840462	•553168	1•807766	•8750425	3
17	•4738321	•538069	1•858496	•8806152	43	38	•4792026	•545972	•1831593	22	58	•4843007	•553548	1•806525	•8749016	2
18	•4740882	•538444	1•857201	•8804774	42	39	•4794579	•546350	•1830327	21	59	•4845552	•553928	1•805286	•8747607	1
19	•4743443	•538819	1•855908	•8803394	41	40	•4797131	•546728	•1829062	20	60	•4848096	•554309	1•804047	•8746197	0
20	•4746004	•539195	1•854615	•8802014	40											
'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	

Deg. 61.

Deg. 61.

Deg. 61.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

29 Deg.

29 Deg.

29 Deg.

	Sine.	Tang.	Cotang.	Cosine.			Sine.	Tang.	Cotang.	Cosine.			Sine.	Tang.	Cotang.	Cosine.	
0	4848096	554309	1801047	8746197	60	21	4901433	562321	1778310	8716419	39	41	4952060	570004	1754372	8687756	19
1	4850640	554689	1802810	8744786	59	22	4903968	562704	1777130	8714993	38	42	4954587	570389	1753186	8686315	18
2	4853184	555069	1804575	8743375	58	23	4906503	563087	1775921	8713566	37	43	4957113	570775	1752002	8684874	17
3	4855727	555450	1806340	8741963	57	24	4909038	563471	1774714	8712138	36	44	4959639	571161	1750819	8683431	16
4	4858270	555831	1799107	8740550	56	25	4911572	563854	1773507	8710710	35	45	4962165	571547	1749637	8681988	15
5	4860812	556211	1797875	8739137	55	26	4914105	564237	1772302	8709281	34	46	4964690	571933	1748456	8680544	14
6	4863354	556592	1796645	8737722	54	27	4916638	564621	1771098	8707851	33	47	4967215	572319	1747276	8679100	13
7	4865895	556973	1795416	8736307	53	28	4919171	565005	1769895	8706420	32	48	4969740	572705	1746098	8677655	12
8	4868436	557355	1794188	8734891	52	29	4921704	565388	1768694	8704989	31	49	4972264	573091	1744921	8676209	11
9	4870977	557736	1792961	8733475	51	30	4924236	565772	1767494	8703557	30	50	4974787	573478	1743745	8674762	10
10	4873517	558117	1791736	8732058	50	31	4926767	566156	1766295	8702124	29	51	4977310	573864	1742570	8673314	9
11	4876057	558499	1790512	8730640	49	32	4929298	566541	1765097	8700691	28	52	4979833	574251	1741396	8671866	8
12	4878597	558881	1789289	8729221	48	33	4931829	566925	1763900	8699256	27	53	4982355	574638	1740224	8670417	7
13	4881136	559262	1788067	8727801	47	34	4934359	567309	1762705	8697821	26	54	4984877	575025	1739053	8668967	6
14	4883674	559644	1786847	8726381	46	35	4936889	567694	1761511	8696386	25	55	4987399	575412	1737883	8667517	5
15	4886212	560026	1785624	8724960	45	36	4939419	568079	1760318	8694949	24	56	4989920	575799	1736714	8666066	4
16	4888750	560409	1784410	8723538	44	37	4941948	568463	1759126	8693512	23	57	4992441	576187	1735546	8664614	3
17	4891288	560791	1783194	8722116	43	38	4944476	568848	1757936	8692074	22	58	4994961	576574	1734380	8663161	2
18	4893825	561173	1781979	8720693	42	39	4947005	569233	1756747	8690636	21	59	4997481	576962	1733214	8661708	1
19	4896361	561556	1780765	8719269	41	40	4949532	569619	1755559	8689196	20	60	5000000	577350	1732050	8660254	0
20	4898897	561939	1779552	8717844	40												

Deg. 60.

Deg. 60.

Deg. 60.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

30 Deg.

30 Deg.

30 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'
0	5000000	577350	1732050	8660254	60	21	5052809	585524	1707871	8629549	39	41	5102928	593363	1685308	8600007	19
1	5002519	577738	1730887	8658799	59	22	5055319	585914	1706732	8628079	38	42	5105429	593756	1684191	8598523	18
2	5005037	578126	1729726	8657344	58	23	5057828	586305	1705595	8626608	37	43	5107930	594150	1683076	8597037	17
3	5007556	578514	1728565	8655887	57	24	5060338	586696	1704458	8625137	36	44	5110431	594543	1681962	8595551	16
4	5010073	578902	1727406	8654430	56	25	5062846	587087	1703323	8623664	35	45	5112931	594937	1680848	8594064	15
5	5012591	579291	1726247	8652973	55	26	5065355	587478	1702189	8622191	34	46	5115431	595331	1679736	8592576	14
6	5015107	579679	1725090	8651514	54	27	5067863	587870	1701055	8620717	33	47	5117930	595725	1678625	8591088	13
7	5017624	580068	1723934	8650055	53	28	5070370	588261	1699923	8619243	32	48	5120429	596119	1677515	8589599	12
8	5020140	580457	1722779	8648595	52	29	5072877	588653	1698792	8617768	31	49	5122927	596514	1676406	8588109	11
9	5022655	580846	1721626	8647134	51	30	5075384	589045	1697663	8616292	30	50	5125425	596908	1675298	8586619	10
10	5025170	581235	1720473	8645673	50	31	5077890	589436	1696534	8614815	29	51	5127923	597303	1674192	8585127	9
11	5027685	581624	1719322	8644211	49	32	5080396	589828	1695406	8613337	28	52	5130420	597697	1673086	8583635	8
12	5030199	582013	1718172	8642748	48	33	5082901	590221	1694280	8611859	27	53	5132916	598092	1671981	8582143	7
13	5032713	582403	1717023	8641284	47	34	5085406	590613	1693155	8610380	26	54	5135413	598487	1670878	8580649	6
14	5035227	582793	1715875	8639820	46	35	5087910	591005	1692030	8608901	25	55	5137908	598882	1669775	8579155	5
15	5037740	583182	1714728	8638355	45	36	5090414	591398	1690907	8607420	24	56	5140404	599278	1668674	8577660	4
16	5040252	583572	1713582	8636889	44	37	5092918	591791	1689785	8605939	23	57	5142899	599673	1667574	8576164	3
17	5042765	583962	1712438	8635423	43	38	5095421	592183	1688664	8604457	22	58	5145393	600069	1666474	8574668	2
18	5045278	584352	1711294	8633956	42	39	5097924	592576	1687544	8602975	21	59	5147887	600464	1665376	8573171	1
19	5047788	584743	1710152	8632488	41	40	5100426	592969	1686426	8601491	20	60	5150381	600860	1664279	8571673	0
20	5050298	585133	1709011	8631019	40												
'	Cosine.	Cotan.	Tang.	Sine.	'	'	Cosine.	Cotan.	Tang.	Sine.	'	'	Cosine.	Cotan.	Tang.	Sine.	'

Deg. 59.

Deg. 59.

Deg. 59.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

31 Deg.					31 Deg.					31 Deg.				
'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Tang.	Cotang.	Cosine.	'	'	Sine.	Cotang.
0	·5150381	·608860	1·664279	·8571673	60	21	·5202646	·609205	1·641482	·8540051	39	41	·5252241	·617210
1	·5152874	·601256	1·663183	·8570174	59	22	·5205130	·609604	1·640408	·8538538	38	42	·5254717	·617612
2	·5155367	·601652	1·662208	·8568675	58	23	·5207613	·610003	1·639335	·8537023	37	43	·5257191	·618014
3	·5157859	·602049	1·660994	·8567175	57	24	·5210096	·610402	1·638263	·8535508	36	44	·5259665	·618416
4	·5160351	·602445	1·659901	·8565674	56	25	·5212579	·610801	1·637191	·8533992	35	45	·5262139	·618818
5	·5162842	·602841	1·658809	·8564173	55	26	·5215061	·611201	1·636121	·8532475	34	46	·5264613	·619221
6	·5165333	·603238	1·657718	·8562671	54	27	·5217543	·611601	1·635052	·8530958	33	47	·5267085	·619623
7	·5167824	·603635	1·656629	·8561168	53	28	·5220024	·612000	1·633984	·8529440	32	48	·5269558	·620026
8	·5170314	·604032	1·655540	·8559664	52	29	·5222505	·612400	1·632917	·8527921	31	49	·5272030	·620429
9	·5172804	·604429	1·654452	·8558160	51	30	·5224986	·612800	1·631851	·8526402	30	50	·5274502	·620832
10	·5175293	·604826	1·653366	·8556655	50	31	·5227466	·613201	1·630786	·8524881	29	51	·5276973	·621235
11	·5177782	·605224	1·652280	·8555149	49	32	·5229945	·613601	1·629722	·8523360	28	52	·5279443	·621638
12	·5180270	·605621	1·651196	·8553643	48	33	·5232424	·614001	1·628659	·8521839	27	53	·5281914	·622041
13	·5182758	·606019	1·650112	·8552135	47	34	·5234903	·614402	1·627597	·8520316	26	54	·5284383	·622445
14	·5185246	·606417	1·649030	·8550627	46	35	·5237381	·614803	1·626536	·8518793	25	55	·5286853	·622848
15	·5187733	·606814	1·647949	·8549119	45	36	·5239859	·615204	1·625476	·8517269	24	56	·5289322	·623252
16	·5190219	·607213	1·646868	·8547609	44	37	·5242336	·615605	1·624417	·8515745	23	57	·5291790	·623656
17	·5192705	·607611	1·645789	·8546099	43	38	·5244813	·616006	1·623359	·8514219	22	58	·5294258	·624060
18	·5195191	·608009	1·644711	·8544588	42	39	·5247290	·616407	1·622302	·8512693	21	59	·5296726	·624465
19	·5197676	·608408	1·643633	·8543077	41	40	·5249766	·616809	1·621246	·8511167	20	60	·5299193	·624869
20	·5200161	·608806	1·642557	·8541564	40									
'	Cosine.	Cotan.	Tang.	Sine.	'	'	Cosine.	Cotan.	Tang.	Sine.	'	'	Cosine.	Tang.

Deg. 58.

Deg. 58

Deg. 58.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

32 Deg.

32 Deg.

32 Deg.

	Sine.	Tang.	Cotang.	Cosine.			Sine.	Tang.	Cotang.	Cosine.			Sine.	Tang.	Cotang.	Cosine.	
0	5299193	624869	1.600334	8480481	60	21	5350898	633395	1.578791	8447952	39	41	5399955	641577	1.558657	8416679	19
1	5301659	625273	1.599299	8478939	59	22	5353355	633803	1.577776	8446395	38	42	5402403	641988	1.557660	8415108	18
2	5304125	625678	1.598264	8477397	58	23	5355812	634211	1.576761	8444838	37	43	5404851	642399	1.556663	8413536	17
3	5306591	626083	1.597231	8475853	57	24	5358268	634619	1.575747	8443279	36	44	5407298	642810	1.555668	8411963	16
4	5309057	626488	1.596198	8474309	56	25	5360724	635027	1.574735	8441720	35	45	5409745	643221	1.554674	8410390	15
5	5311521	626893	1.595167	8472765	55	26	5363179	635435	1.573723	8440161	34	46	5412191	643632	1.553680	8408816	14
6	5313986	627298	1.594136	8471219	54	27	5365634	635844	1.572712	8438600	33	47	5414637	644044	1.552688	8407241	13
7	5316450	627704	1.593107	8469673	53	28	5368089	636252	1.571702	8437039	32	48	5417082	644456	1.551696	8405666	12
8	5318913	628109	1.592078	8468126	52	29	5370543	636661	1.570693	8435477	31	49	5419527	644867	1.550705	8404090	11
9	5321376	628515	1.591050	8466579	51	30	5372996	637070	1.569685	8433914	30	50	5421971	645279	1.549715	8402513	10
10	5323839	628921	1.590023	8465030	50	31	5375449	637479	1.568678	8432351	29	51	5424415	645691	1.548726	8400936	9
11	5326301	629327	1.588997	8463481	49	32	5377902	637888	1.567672	8430787	28	52	5426859	646104	1.547738	8399357	8
12	5328763	629733	1.587973	8461932	48	33	5380354	638297	1.566666	8429222	27	53	5429302	646516	1.546751	8397778	7
13	5331224	630139	1.586949	8460381	47	34	5382806	638707	1.565662	8427657	26	54	5431744	646929	1.545764	8396199	6
14	5333685	630546	1.585926	8458830	46	35	5385257	639116	1.564659	8426091	25	55	5434187	647341	1.544779	8394618	5
15	5336145	630953	1.584904	8457278	45	36	5387708	639526	1.563656	8424524	24	56	5436628	647754	1.543794	8393037	4
16	5338605	631359	1.583883	8455726	44	37	5390158	639936	1.562654	8422956	23	57	5439069	648167	1.542810	8391455	3
17	5341065	631766	1.582862	8454172	43	38	5392608	640346	1.561654	8421388	22	58	5441510	648580	1.541828	8389873	2
18	5343523	632173	1.581843	8452618	42	39	5395058	640756	1.560654	8419819	21	59	5443951	648994	1.540846	8388290	1
19	5345982	632581	1.580825	8451064	41	40	5397507	641167	1.559655	8418249	20	60	5446390	649407	1.539865	8386706	0
20	5348440	632988	1.579807	8449508	40												
	Cosine.	Cotan.	Tang.	Sine.			Cosine.	Cotan.	Tang.	Sine.			Cosine.	Cotan.	Tang.	Sine.	

Deg. 57.

Deg. 57.

Deg. 57.

#### IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

[illegible]



## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

34 Deg.

34 Deg.

34 Deg.

	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	5591929	674508	1.482561	8290376	60	21	5642467	683433	1.463200	8256062	39	41	5690403	692002	19
1	5593430	674931	1.481631	8288749	59	22	5644869	683860	1.462287	8254420	38	42	5692795	692432	18
2	5596751	675355	1.480702	8287121	58	23	5647270	684287	1.461374	8252778	37	43	5695187	692863	17
3	5599162	675779	1.479773	8285493	57	24	5649670	684714	1.460463	8251135	36	44	5697577	693293	16
4	5601572	676202	1.478846	8283864	56	25	5652070	685141	1.459552	8249491	35	45	5699968	693724	15
5	5603981	676626	1.477919	8282234	55	26	5654469	685569	1.458642	8247847	34	46	5702357	694155	14
6	5606390	677050	1.476993	8280603	54	27	5656868	685996	1.457732	8246202	33	47	5704747	694586	13
7	5608798	677475	1.476068	8278972	53	28	5659267	686424	1.456824	8244555	32	48	5707136	695018	12
8	5611206	677899	1.475144	8277340	52	29	5661665	686852	1.455916	8242909	31	49	5709524	695449	11
9	5613614	678324	1.474221	8275708	51	30	5664062	687281	1.455009	8241262	30	50	5711912	695881	10
10	5616021	678749	1.473298	8274074	50	31	5666459	687709	1.454102	8239614	29	51	5714299	696313	9
11	5618428	679174	1.472376	8272440	49	32	5668856	688137	1.453197	8237965	28	52	5716686	696745	8
12	5620834	679599	1.471455	8270806	48	33	5671252	688566	1.452292	8236316	27	53	5719073	697177	7
13	5623239	680024	1.470535	8269170	47	34	5673648	688995	1.451388	8234666	26	54	5721459	697609	6
14	5625645	680450	1.469615	8267534	46	35	5676043	689424	1.450485	8233015	25	55	5723844	698042	5
15	5628049	680875	1.468696	8265897	45	36	5678437	689853	1.449582	8231364	24	56	5726229	698474	4
16	5630453	681301	1.467778	8264260	44	37	5680832	690283	1.448680	8229712	23	57	5728614	698907	3
17	5632857	681727	1.466861	8262622	43	38	5683225	690712	1.447779	8228059	22	58	5730998	699340	2
18	5635260	682153	1.465945	8260983	42	39	5685619	691142	1.446879	8226405	21	59	5733381	699774	1
19	5637663	682580	1.465029	8259343	41	40	5688011	691572	1.445980	8224751	20	60	5735764	700207	0
20	5640066	683006	1.464114	8257703	40										
	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'

Deg. 55.

Deg. 55.

Deg. 55.



## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

• 35 Deg.

35 Deg.

35 Deg.

°	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	5735764	700207	1.428148	8191520	60	21	5785696	709350	1.409740	8156330	39	41	5833050	718131	1.392501
1	5738147	700641	1.427264	8189852	59	22	5788069	709787	1.408871	8154647	38	42	5835412	718572	1.391647
2	5740529	701074	1.426381	8188182	58	23	5790440	710225	1.408003	8152963	37	43	5837774	719014	1.390793
3	5742911	701508	1.425498	8186512	57	24	5792812	710663	1.407136	8151278	36	44	5840136	719455	1.389940
4	5745292	701943	1.424617	8184841	56	25	5795183	711100	1.406270	8149593	35	45	5842497	719897	1.389087
5	5747672	702377	1.423736	8183169	55	26	5797553	711539	1.405404	8147906	34	46	5844857	720338	1.388235
6	5750053	702811	1.422856	8181497	54	27	5799923	711977	1.404539	8146220	33	47	5847217	720780	1.387384
7	5752432	703246	1.421976	8179824	53	28	5802292	712415	1.403674	8144532	32	48	5849577	721222	1.386534
8	5754811	703681	1.421097	8178151	52	29	5804661	712854	1.402811	8142844	31	49	5851936	721665	1.385684
9	5757190	704116	1.420220	8176476	51	30	5807030	713293	1.401948	8141155	30	50	5854294	722107	1.384835
10	5759568	704551	1.419342	8174801	50	31	5809397	713732	1.401086	8139466	29	51	5856652	722550	1.383986
11	5761946	704986	1.418466	8173125	49	32	5811765	714171	1.400224	8137775	28	52	5859010	722993	1.383139
12	5764323	705422	1.417590	8171449	48	33	5814132	714610	1.399363	8136084	27	53	5861367	723436	1.382292
13	5766700	705858	1.416715	8169772	47	34	5816498	715050	1.398503	8134393	26	54	5863724	723879	1.381445
14	5769076	706294	1.415840	8168094	46	35	5818864	715489	1.397644	8132701	25	55	5866080	724322	1.380600
15	5771452	706730	1.414967	8166416	45	36	5821230	715929	1.396785	8131008	24	56	5868435	724766	1.379755
16	5773827	707166	1.414094	8164736	44	37	5823595	716369	1.395927	8129314	23	57	5870790	725210	1.378910
17	5776202	707602	1.413222	8163056	43	38	5825959	716810	1.395069	8127620	22	58	5873145	725654	1.378067
18	5778576	708039	1.412350	8161376	42	39	5828323	717250	1.394213	8125925	21	59	5875499	726098	1.377224
19	5780950	708476	1.411479	8159695	41	40	5830687	717691	1.393357	8124229	20	60	5877853	726542	1.376381
20	5783323	708913	1.410609	8158013	40										

Deg. 54.

Deg. 54.

Deg. 54.



## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

37 Deg.

37 Deg.

37 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	·6018150	753554	1·327044	·7986355	60	21	·6066824	763175	1·310314	·7949144	39	41	·6112969	772423	19
1	·6020473	754010	1·326242	·7984604	59	22	·6069136	763636	1·309523	·7947578	38	42	·6115270	772887	18
2	·6022795	754466	1·325439	·7982853	58	23	·6071447	764096	1·308734	·7945913	37	43	·6117572	773352	17
3	·6025117	754923	1·324638	·7981100	57	24	·6073758	764557	1·307945	·7944146	36	44	·6119873	773817	16
4	·6027439	755379	1·323837	·7979347	56	25	·6076069	765018	1·307157	·7942379	35	45	·6122173	774282	15
5	·6029760	755836	1·323036	·7977594	55	26	·6078379	765480	1·306369	·7940611	34	46	·6124473	774748	14
6	·6032080	756294	1·322237	·7975839	54	27	·6080689	765941	1·305582	·7938843	33	47	·6126772	775213	13
7	·6034400	756751	1·321437	·7974084	53	28	·6082998	766403	1·304796	·7937074	32	48	·6129071	775679	12
8	·6036719	757209	1·320639	·7972329	52	29	·6085306	766864	1·304010	·7935304	31	49	·6131369	776145	11
9	·6039038	757666	1·319841	·7970572	51	30	·6087614	767327	1·303225	·7933533	30	50	·6133666	776611	10
10	·6041356	758124	1·319044	·7968815	50	31	·6089922	767789	1·302440	·7931762	29	51	·6135964	777078	9
11	·6043674	758582	1·318247	·7967058	49	32	·6092229	768251	1·301656	·7929990	28	52	·6138260	777544	8
12	·6045991	759041	1·317451	·7965299	48	33	·6094535	768714	1·300873	·7928218	27	53	·6140556	778011	7
13	·6048308	759499	1·316655	·7963540	47	34	·6096841	769177	1·300090	·7926445	26	54	·6142852	778478	6
14	·6050624	759958	1·315861	·7961780	46	35	·6099147	769640	1·299308	·7924671	25	55	·6145147	778946	5
15	·6052940	760417	1·315066	·7960020	45	36	·6101452	770103	1·298526	·7922896	24	56	·6147442	779413	4
16	·6055255	760876	1·314273	·7958259	44	37	·6103756	770567	1·297745	·7921121	23	57	·6149736	779881	3
17	·6057570	761336	1·313480	·7956497	43	38	·6106060	771030	1·296964	·7919345	22	58	·6152029	780349	2
18	·6059884	761795	1·312687	·7954735	42	39	·6108363	771494	1·296185	·7917569	21	59	·6154322	780817	1
19	·6062198	762255	1·311895	·7952972	41	40	·6110666	771958	1·295405	·7915792	20	60	·6156615	781285	0
20	·6064511	762715	1·311104	·7951208	40										
'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'	Cosine.	Cotan.	Tang.	Sine.	'

Deg. 52.

Deg. 52.

Deg. 52.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

38 Deg.

38 Deg.

38 Deg.

	Sine.	Tang.	Cotan.	Cotang.	Cosine.		Sine.	Tang.	Cotan.	Cotang.	Cosine.	
0	6156615	781285	1.279941	7880108	60	21	6204636	791170	1.263950	7842352	39	41
1	6158907	781754	1.279174	7878316	59	22	6206917	791643	1.263195	7840547	38	42
2	6161198	782222	1.278407	7876524	58	23	6209198	792116	1.262440	7838741	37	43
3	6163489	782691	1.277641	7874732	57	24	6211478	792590	1.261686	7836935	36	44
4	6165780	783161	1.276876	7872939	56	25	6213757	793064	1.260932	7835127	35	45
5	6168069	783630	1.276111	7871145	55	26	6216036	793537	1.260179	7833320	34	46
6	6170359	784100	1.275347	7869350	54	27	6218314	794012	1.259426	7831511	33	47
7	6172648	784570	1.274583	7867555	53	28	6220592	794486	1.258674	7829702	32	48
8	6174936	785040	1.273820	7865759	52	29	6222870	794961	1.257923	7827892	31	49
9	6177224	785510	1.273057	7863963	51	30	6225146	795435	1.257172	7826082	30	50
10	6179511	785980	1.272295	7862165	50	31	6227423	795911	1.256421	7824270	29	51
11	6181798	786451	1.271534	7860367	49	32	6229698	796386	1.255672	7822459	28	52
12	6184084	786922	1.270773	7858569	48	33	6231974	796861	1.254922	7820646	27	53
13	6186370	787393	1.270013	7856770	47	34	6234248	797337	1.254174	7818833	26	54
14	6188655	787864	1.269253	7854970	46	35	6236522	797813	1.253426	7817019	25	55
15	6190939	788336	1.268494	7853169	45	36	6238796	798289	1.252678	7815205	24	56
16	6193224	788808	1.267735	7851368	44	37	6241069	798765	1.251931	7813390	23	57
17	6195507	789280	1.266977	7849566	43	38	6243342	799242	1.251184	7811574	22	58
18	6197790	789752	1.266219	7847764	42	39	6245614	799719	1.250438	7809757	21	59
19	6200073	790224	1.265462	7845961	41	40	6247885	800196	1.249693	7807940	20	60
20	6202355	790697	1.264706	7844157	40							

Deg. 51.

Deg. 51.

Deg. 51.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

39 Deg.										39 Deg.										39 Deg.											
'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	
0	6293204	809784	1.234897	7771460	60	21	6340559	819948	1.219588	7732872	39	41	6385440	829724	1	205219	7698553	19													
1	6295464	810265	1.234162	7769629	59	22	6342808	820435	1.218865	7731027	38	42	6387678	830216	1	204505	7693996	18													
2	6297724	810747	1.233429	7767797	58	23	6345057	820922	1.218142	7729182	37	43	6389916	830707	1	203793	7692137	17													
3	6299983	811230	1.232696	7765965	57	24	6347305	821409	1.217419	7727336	36	44	6392153	831199	1	203081	7690278	16													
4	6302242	811712	1.231963	7764132	56	25	6349553	821896	1.216698	7725489	35	45	6394390	831691	1	202369	7688418	15													
5	6304500	812195	1.231231	7762298	55	26	6351800	822384	1.215976	7723642	34	46	6396626	832183	1	201658	7686558	14													
6	6306758	812678	1.230499	7760464	54	27	6354046	822871	1.215256	7721794	33	47	6398862	832675	1	200947	7684697	13													
7	6309015	813161	1.229768	7758629	53	28	6356292	823359	1.214535	7719945	32	48	6401097	833168	1	200237	7682835	12													
8	6311272	813644	1.229038	7756794	52	29	6358537	823847	1.213816	7718096	31	49	6403332	833661	1	199527	7680973	11													
9	6313528	814128	1.228308	7754957	51	30	6360782	824336	1.213097	7716246	30	50	6405566	834154	1	198818	7679110	10													
10	6315784	814611	1.227578	7753121	50	31	6363026	824825	1.212378	7714395	29	51	6407799	834648	1	198109	7677246	9													
11	6318039	815093	1.226849	7751283	49	32	6365270	825314	1.211660	7712544	28	52	6410032	835141	1	197401	7675382	8													
12	6320293	815580	1.226121	7749445	48	33	6367513	825803	1.210942	7710692	27	53	6412264	835635	1	196693	7673517	7													
13	6322547	816064	1.225393	7747606	47	34	6369756	826292	1.210225	7708840	26	54	6414496	836129	1	195986	7671652	6													
14	6324800	816549	1.224665	7745767	46	35	6371998	826782	1.209508	7706986	25	55	6416728	836624	1	195279	7669785	5													
15	6327053	817034	1.223938	7743926	45	36	6374240	827271	1.208792	7705132	24	56	6418958	837118	1	194573	7667918	4													
16	6329306	817519	1.223212	7742086	44	37	6376481	827762	1.208076	7703278	23	57	6421189	837613	1	193867	7666051	3													
17	6331557	818004	1.222486	7740244	43	38	6378721	828252	1.207361	7701423	22	58	6423418	838108	1	193162	7664183	2													
18	6333809	818490	1.221761	7738402	42	39	6380961	828742	1.206646	7699567	21	59	6425647	838604	1	192457	7662314	1													
19	6336059	818976	1.221036	7736559	41	40	6383201	829233	1.205932	7697710	20	60	6427876	839099	1	191753	7660444	0													
20	6338310	819462	1.220312	7734716	40																										

Deg. 50.

Deg. 50.

Deg. 50.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

40 Deg.

40 Deg.

40 Deg.

	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	6427876	839099	1191753	7660444	60	21	6474551	849563	1177075	7621036	39	41	6518778	859629	1163291
1	6430104	839595	1191039	7658574	59	22	6476767	850064	1170382	7619152	38	42	6520984	860135	1162607
2	6432332	840091	1190346	7656704	58	23	6478984	850565	1175688	7617268	37	43	6523189	860641	1161923
3	6434559	840587	1189643	7654832	57	24	6481199	851066	1174996	7615383	36	44	6525394	861148	1161240
4	6436785	841084	1188941	7652960	56	25	6483414	851568	1174303	7613497	35	45	6527598	861655	1160557
5	6439011	841581	1188239	7651087	55	26	6485628	852070	1173612	7611611	34	46	6529801	862162	1159874
6	6441236	842078	1187538	7649214	54	27	6487842	852572	1172920	7609724	33	47	6532004	862669	1159192
7	6443461	842575	1186837	7647340	53	28	6490056	853075	1172229	7607837	32	48	6534206	863176	1158511
8	6445685	843073	1186136	7645465	52	29	6492268	853577	1171539	7605949	31	49	6536408	863684	1157830
9	6447909	843570	1185437	7643590	51	30	6494480	854080	1170849	7604060	30	50	6538609	864192	1157149
10	6450132	844068	1184737	7641714	50	31	6496692	854583	1170160	7602170	29	51	6540810	864700	1156469
11	6452355	844567	1184038	7639838	49	32	6498903	855087	1169471	7600280	28	52	6543010	865209	1155789
12	6454577	845065	1183340	7637960	48	33	6501114	855591	1168782	7598389	27	53	6545209	865718	1155110
13	6456798	845564	1182642	7636082	47	34	6503324	856095	1168094	7596498	26	54	6547408	866227	1154431
14	6459019	846063	1181944	7634204	46	35	6505533	856599	1167407	7594606	25	55	6549607	866736	1153753
15	6461240	846562	1181247	7632325	45	36	6507742	857103	1166720	7592713	24	56	6551804	867246	1153075
16	6463460	847062	1180551	7630445	44	37	6509951	857608	1166033	7590820	23	57	6554002	867755	1152397
17	6465679	847561	1179855	7628564	43	38	6512158	858113	1165347	7588926	22	58	6556198	868265	1151721
18	6467898	848061	1179159	7626683	42	39	6514366	858618	1164661	7587031	21	59	6558395	868776	1151044
19	6470116	848561	1178464	7624802	41	40	6516572	859124	1163976	7585136	20	60	6560590	869286	1150368
20	6472334	849062	1177769	7622919	40										

Deg. 49.

Deg. 49.

Deg. 49.

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

41 Deg.

41 Deg.

41 Deg.

	Sine.	Tang.	Cotang.	Cosine.			Sine.	Tang.	Cotang.	Cosine.			Sine.	Tang.	Cotang.	Cosine.	
0	6560390	869286	1.150368	3547096	60	21	6606570	880068	1.136274	7506879	39	41	66550131	890445	1.123032	7468317	19
1	6562785	869797	1.149692	7545187	59	22	6608754	880385	1.135608	7504957	38	42	6652304	890967	1.122375	7466392	18
2	6564980	870308	1.149017	7543278	58	23	6610936	881101	1.134942	7503034	37	43	6654475	891489	1.121718	7464446	17
3	6567174	870820	1.148342	7541368	57	24	6613119	881618	1.134277	7501111	36	44	6656646	892011	1.121061	7462510	16
4	6569367	871331	1.147668	7539457	56	25	6615300	882135	1.133612	7499187	35	45	6658817	892534	1.120405	7460574	15
5	6571560	871843	1.146994	7537546	55	26	6617482	882653	1.132947	7497262	34	46	6660987	893056	1.119749	7458636	14
6	6573752	872355	1.146321	7535634	54	27	6619662	883170	1.132283	7495337	33	47	6663156	893579	1.119094	7456699	13
7	6575944	872868	1.145648	7533721	53	28	6621842	883688	1.131620	7493411	32	48	6665325	894103	1.118439	7454760	12
8	6578135	873380	1.144976	7531808	52	29	6624022	884206	1.130957	7491484	31	49	6667493	894626	1.117784	7452821	11
9	6580326	873893	1.144304	7529894	51	30	6626200	884725	1.130294	7489557	30	50	6669661	895150	1.117130	7450881	10
10	6582516	874406	1.143632	7527980	50	31	6628379	885244	1.129632	7487629	29	51	6671828	895674	1.116476	7448941	9
11	6584706	874920	1.142961	7526065	49	32	6630557	885763	1.128970	7485701	28	52	6673994	896199	1.115823	7446999	8
12	6586895	875433	1.142290	7524149	48	33	6632734	886282	1.128308	7483772	27	53	6676160	896723	1.115170	7445058	7
13	6589083	875947	1.141620	7522233	47	34	6634910	886801	1.127647	7481842	26	54	6678326	897248	1.114518	7443115	6
14	6591271	876462	1.140950	7520316	46	35	6637087	887321	1.126987	7479912	25	55	6680490	897773	1.113866	7441173	5
15	6593458	876976	1.140281	7518398	45	36	6639262	887841	1.126327	7477981	24	56	6682655	898299	1.113214	7439229	4
16	6595645	877491	1.139612	7516480	44	37	6641437	888361	1.125667	7476049	23	57	6684818	898825	1.112563	7437285	3
17	6597831	878006	1.138944	7514561	43	38	6643612	888882	1.125008	7474117	22	58	6686981	899351	1.111912	7435340	2
18	6600017	878521	1.138276	7512641	42	39	6645785	889403	1.124349	7472184	21	59	6689144	899877	1.111262	7433394	1
19	6602202	879037	1.137608	7510721	41	40	6647959	889924	1.123690	7470251	20	60	6691306	900404	1.110612	7431448	0
20	6604386	879552	1.136941	7508800	40												
	Cosine.	Cotan.	Tang.	Sine.			Cosine.	Cotan.	Tang.	Sine.			Cosine.	Cotan.	Tang.	Sine.	

Deg. 48.

Deg. 48.

Deg. 48.



IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

FOR RAILROADS.

213

42 Deg.

42 Deg.

42 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	6691306	900404	1.110612	7431448	60	21	6736577	911526	1.097060	7390435	39	41	6779459	922235	1.084322
1	6693468	900930	1.109963	7429502	59	22	6738727	912059	1.096420	7388475	38	42	6781597	922773	1.083689
2	6695628	901458	1.109314	7427554	58	23	6740876	912592	1.095779	7386515	37	43	6783734	923312	1.083057
3	6697789	901985	1.108665	7425606	57	24	6743024	913125	1.095139	7384553	36	44	6785871	923851	1.082425
4	6699948	902513	1.108017	7423658	56	25	6745172	913659	1.094500	7382592	35	45	6788007	924390	1.081793
5	6702108	903041	1.107369	7421708	55	26	6747319	914192	1.093861	7380629	34	46	6790143	924930	1.081162
6	6704266	903569	1.106721	7419758	54	27	6749466	914727	1.093222	7378666	33	47	6792278	925470	1.080532
7	6706424	904097	1.106075	7417808	53	28	6751612	915261	1.092584	7376703	32	48	6794413	926010	1.079901
8	6708582	904626	1.105428	7415857	52	29	6753757	915796	1.091946	7374738	31	49	6796547	926550	1.079271
9	6710739	905155	1.104782	7413905	51	30	6755902	916331	1.091308	7372773	30	50	6798681	927091	1.078642
10	6712895	905685	1.104136	7411953	50	31	6758046	916866	1.090671	7370808	29	51	6800813	927632	1.078013
11	6715051	906214	1.103491	7410000	49	32	6760190	917402	1.090034	7368842	28	52	6802946	928173	1.077384
12	6717206	906744	1.102846	7408046	48	33	6762333	917937	1.089398	7366875	27	53	6805078	928715	1.076756
13	6719361	907274	1.102201	7406092	47	34	6764476	918474	1.088762	7364908	26	54	6807209	929257	1.076128
14	6721515	907805	1.101557	7404137	46	35	6766618	919010	1.088126	7362940	25	55	6809339	929799	1.075500
15	6723668	908336	1.100914	7402181	45	36	6768760	919547	1.087491	7360971	24	56	6811469	930342	1.074873
16	6725821	908867	1.100270	7400225	44	37	6770901	920084	1.086857	7359002	23	57	6813599	930884	1.074246
17	6727973	909398	1.199628	7398268	43	38	6773041	920621	1.086222	7357032	22	58	6815728	931428	1.073620
18	6730125	909930	1.198985	7396311	42	39	6775181	921159	1.085588	7355061	21	59	6817856	931971	1.072994
19	6732276	910461	1.198343	7394353	41	40	6777320	921696	1.084955	7353090	20	60	6819984	932515	1.072368
20	6734427	910994	1.197702	7392394	40										1.071742

Deg. 47.

Deg. 47.

Deg. 47.

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## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

43 Deg

43 Deg.

43 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	.6819984	.932515	1.072368	.7313537	60	.21	.6864532	.944001	1.059320	.7271740	39	41	.6906721	.935064	1.047049
1	.6822211	.933059	1.071743	.7311553	59	22	.6866667	.944651	1.058703	.7269743	38	42	.6908824	.935620	1.046440
2	.6824237	.933603	1.071118	.7309568	58	23	.6868761	.945102	1.058086	.7267745	37	43	.6910927	.936177	1.045831
3	.6826263	.934147	1.070494	.7307583	57	24	.6870875	.945653	1.057470	.7265747	36	44	.6913029	.936734	1.045222
4	.6828289	.934692	1.069870	.7305597	56	25	.6872988	.946204	1.056854	.7263748	35	45	.6915131	.937291	1.044613
5	.6830313	.935238	1.069246	.7303610	55	26	.6875101	.946755	1.056238	.7261748	34	46	.6917232	.937849	1.044005
6	.68322738	.935783	1.068623	.7301623	54	27	.6877213	.947307	1.055623	.7259748	33	47	.6919332	.938407	1.043397
7	.6834261	.936329	1.068000	.7299635	53	28	.6879325	.947859	1.055008	.7257747	32	48	.6921432	.938965	1.042790
8	.68362984	.936875	1.067377	.7297646	52	29	.6881435	.948411	1.054394	.7255746	31	49	.6923531	.939524	1.042183
9	.68383107	.937421	1.066755	.7295657	51	30	.6883546	.948964	1.053780	.7253744	30	50	.6925630	.960082	1.041576
10	.68403229	.937968	1.066134	.7293668	50	31	.6885655	.949517	1.053166	.7251741	29	51	.6927728	.960642	1.040970
11	.68423350	.938515	1.065512	.7291677	49	32	.6887765	.950070	1.052553	.7249738	28	52	.6929825	.961201	1.040364
12	.68443471	.939062	1.064891	.7289686	48	33	.6889873	.950624	1.051940	.7247734	27	53	.6931922	.961761	1.039758
13	.68463591	.939610	1.064271	.7287695	47	34	.6891981	.951178	1.051327	.7245729	26	54	.6934018	.962321	1.039153
14	.68483711	.940157	1.063651	.7285703	46	35	.6894089	.951732	1.050715	.7243724	25	55	.6936114	.962881	1.038548
15	.68503830	.940706	1.063031	.7283710	45	36	.6896195	.952287	1.050103	.7241719	24	56	.6938209	.963442	1.037944
16	.68523948	.941254	1.062411	.7281716	44	37	.6898302	.952842	1.049492	.7239712	23	57	.6940304	.964003	1.037340
17	.68544066	.941803	1.061792	.7279722	43	38	.6900407	.953397	1.048880	.7237705	22	58	.6942398	.964565	1.036736
18	.68564184	.942352	1.061174	.7277728	42	39	.6902512	.953952	1.048270	.7235698	21	59	.6944491	.965126	1.036133
19	.68584300	.942901	1.060556	.7275732	41	40	.6904617	.954508	1.047659	.7233690	20	60	.6946584	.965688	1.035530
20	.68604416	.943451	1.059938	.7273736	40										

Deg. 46.

Deg. 46.

Deg. 46.

'	Cosine.	Cotan.	Tang.	Cosine.	'	Sine.	Cotan.	Tang.	Cosine.	'
0	.7193398			.7193398	0				.7193398	0
1	.7195418			.7195418	1				.7195418	1
2	.7197438			.7197438	2				.7197438	2
3	.7199457			.7199457	3				.7199457	3
4	.7201476			.7201476	4				.7201476	4
5	.7203494			.7203494	5				.7203494	5
6	.7205511			.7205511	6				.7205511	6
7	.7207528			.7207528	7				.7207528	7
8	.7209544			.7209544	8				.7209544	8
9	.7211559			.7211559	9				.7211559	9
10	.7213574			.7213574	10				.7213574	10
11	.7215589			.7215589	11				.7215589	11
12	.7217602			.7217602	12				.7217602	12
13	.7219615			.7219615	13				.7219615	13
14	.7221628			.7221628	14				.7221628	14
15	.7223640			.7223640	15				.7223640	15
16	.7225651			.7225651	16				.7225651	16
17	.7227661			.7227661	17				.7227661	17
18	.7229671			.7229671	18				.7229671	18
19	.7231681			.7231681	19				.7231681	19

## IV.—NATURAL SINES AND TANGENTS TO A RADIUS 1—continued.

44 Deg.

44 Deg.

44 Deg.

'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'	Sine.	Tang.	Cotang.	Cosine.	'
0	6946584	965688	1.035530	7193398	60	21	6990396	977564	1.022550	39	41	7031879	989006	1.011115	7110041
1	6948676	966251	1.034927	7191377	59	22	6992476	978133	1.022355	38	42	7033947	989582	1.010527	7107995
2	6950767	966813	1.034325	7189355	58	23	6994555	978702	1.022160	37	43	7036014	990158	1.009939	7105948
3	6952858	967376	1.033723	7187333	57	24	6996633	979272	1.021965	36	44	7038081	990734	1.009352	7103901
4	6954949	967939	1.033122	7185310	56	25	6998711	979842	1.021770	35	45	7040147	991311	1.008764	7101854
5	6957039	968503	1.032520	7183287	55	26	7000789	980412	1.021575	34	46	7042213	991888	1.008178	7099806
6	6959128	969067	1.031919	7181263	54	27	7002866	980983	1.021380	33	47	7044278	992465	1.007591	7097757
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8	6963305	970196	1.030719	7177213	52	29	7007018	982125	1.020990	31	49	7048406	993620	1.006420	7093657
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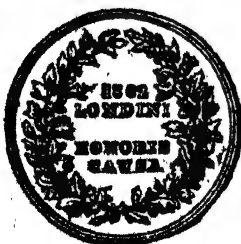
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